

Appendix 11.1: Sloy Pumped Hydro Storage Scheme: 2010 Ground Investigation Records



FUGRO ENGINEERING SERVICES LIMITED

SCOTTISH & SOUTHERN ENERGY

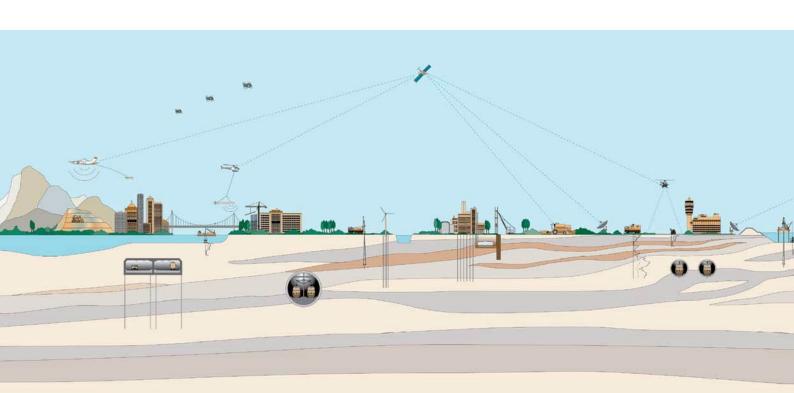
SLOY PUMPING STATION

FACTUAL REPORT ON GROUND INVESTIGATION

CONTRACT NO : CON103001

DATE : MAY 2010







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SCOTTISH & SOUTHERN ENERGY SLOY PUMPING STATION



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1. INTRODUCTION

On the instructions and under the supervision of Jacobs Engineering (UK) Limited (the Engineer), acting on behalf of Scottish & Southern Energy (the Employer), a site investigation has been carried out by Fugro Engineering Services Limited (FES) at Sloy Power Station, Argyll and Bute.

The objective of the investigation was to determine the ground conditions at the site and to provide information that would assist the Engineer in the geotechnical appraisal of the site. The scope of the investigation was determined by the Engineer.

A factual report was requested including exploratory hole and field testing records, laboratory test results and a site plan. The exploratory hole and laboratory test data have also been provided as digital data to AGS format. Photographs of the rock core and trial pits are presented separately in the Addendum.

The site work, which comprised eleven light cable tool percussion boreholes with rotary cored follow-on and three rotary open hole boreholes with rotary cored follow-on, to a maximum depth of 35.00 m, seven machine dug trial pits, twenty seven hand dug trial pits with Mackintosh probes, concrete coring and geophysical surveying (acoustic/optical televiewer and crosshole geophysics), was carried out between the 18th January and the 17th March 2010.

2. THE SITE AND GEOLOGY

2.1 SITE LOCATION AND DESCRIPTION

The site is located within the existing Sloy Power Station site boundary in the village of Inveruglas, approximately four miles north of Tarbet and adjacent to Loch Lomond, Argyll and Bute. The approximate National Grid reference of the site is NN 321 098.

At the time of the investigation the site comprised a grassed area and tarmac access road immediately north east of the existing power station building. The site is bound to the north west and north east by woodland, to the immediate east by the A82 and Loch Lomond beyond and to the south and west by the existing turbine hall and substation.

2.2 GEOLOGY

The records of the British Geological Survey (Sheet 38W-Ben Lomond - of the 1:50,000 Series Geological Map, Solid (1987) Edition) indicate that the site is underlain by Quartz Mica Schist of the Beinn Bheula Formation. Drift deposits are not shown to be present at the site.



Further background research such as a desk study was not required within the terms of reference for the work.

3. METHOD OF INVESTIGATION

3.1 GENERAL

A Cable Avoidance Tool (CAT) survey was undertaken at each of the exploratory hole locations. Prior to the sinking of the boreholes and Mackintosh probeholes, inspection pits were dug by hand at each location in order to identify the presence of any services. Services were not encountered.

Details of the in-situ sampling and testing carried out, together with the descriptions of the strata encountered, are given on the various exploratory hole records. An explanation of the symbols and abbreviations used on all the exploratory hole records, together with the method of strata description utilised, is given in the Notes on Exploratory Hole Records (Figures KS/01 to KS/06) in Appendix A. The investigation was generally carried out in accordance with BS 5930:1999ⁱ, BS EN ISO 14688-1:2002ⁱⁱ and BS EN ISO 14689-1:2003ⁱⁱⁱ as appropriate. The borehole records are given in Figures FR1 to FR14, the concrete corehole record is given in Figure FR15 and the trial pit records are given in Figures FR16 to FR49 in Appendix A.

All geotechnical samples were transported to the laboratories and offices of FES in Consett for examination and testing as scheduled by the Engineer. Contamination samples taken during the investigation were sent directly to the contamination testing laboratory.

During the course of the investigation three Vibrock 2000 vibration monitors were placed inside the turbine hall and within the high voltage compound in order to monitor peak vibration. The digital data has been reported separately.

3.2 CABLE PERCUSSION BORING

Eleven, 150 mm minimum diameter, boreholes (BH1 to BH11) were sunk to depths below ground level (bgl) of between 1.20 m (BH5) and 6.60 m (BH3) using light cable tool percussion boring techniques. The borehole records are given in Figures FR1 to FR11 in Appendix A.

Disturbed samples were taken at each change in soil type and at regular vertical intervals during boring in order to identify and give a record of the strata encountered.

Standard penetration tests (SPT) using a split spoon (S) or a solid 60° cone (C) were carried out in the Made Ground and granular deposits. The results are shown as S(N) and C(N) values on the borehole records at the relevant depths in Appendix A.



During the course of boring attention was given to recording any evidence of water inflow in order that the groundwater level beneath the site could be established. Water levels at breaks in boring were recorded where appropriate. Where water was added to facilitate penetration of the soil strata, this is noted on the borehole records.

3.3 ROTARY DRILLING

Eleven boreholes (BH1 to BH11) were extended by rotary core drilling techniques and three boreholes were sunk by rotary open hole and core drilling techniques (BH5A, BH8A and BH12) to depths below ground level of between 3.40 m (BH5) and 35.00 m (BH2, BH3, BH4, BH6 and BH12) using a truck mounted Massenza drill. The coring was carried out using a Geobore S size wireline core barrel and rigid coreliner with impregnated diamond set bits and water flush. Open hole drilling was carried out using 150 mm tricone roller bits and the Symmetrix drill and case system.

During the course of drilling attention was given to recording any evidence of water inflow in order that the groundwater level beneath the site could be established. Water levels at breaks in boring were recorded where appropriate.

The cores were logged by a geotechnical engineer from FES and photographed on site. The Solid Core Recovery (SCR) and Rock Quality Designation (RQD) have been determined using the modified proposal as given in Norbury et al iv, that a "solid cylinder" should be defined as having a full diameter (but not necessarily a full circumference) without discontinuities and should be measured axially along the core. In a number of instances the logging geologist assessed that some core from one run was recovered with the core from the next run. In these cases the TCR, SCR and RQD have been determined assuming that the core had been recovered from the core run in which it had first been drilled.

The borehole records are given in Figures FR1 to FR14 in Appendix A. The core photographs are given in the Addendum.

3.4 CONCRETE COREHOLE

A single concrete corehole was drilled in the tailrace retaining wall as instructed by the Engineer. The record is given in Figure FR15 in Appendix A and photographs are given in the Addendum.

3.5 TRIAL PITS

Twenty seven trial pits were excavated by hand (MP1 to MP26 and TP1) and seven trial pits were excavated by machine (TP2 to TP7 and TP2A) using a JCB 3CX to depths of between 0.50 m (TP2) and 3.70 m (TP6) below ground level. The trial pits were logged by a geotechnical engineer from FES who took samples and carried out in-situ testing as shown on the trial pit records (Figures FR16 to FR49 in Appendix A). Notes on



excavation stability and any groundwater encountered are also given on the records. Photographs of the trial pits were also taken by the engineer from FES and these are reproduced in the Addendum.

During the course of excavation attention was given to recording any evidence of water inflow in order that the groundwater level beneath the site could be established. The depth at which water seepage or ingress was encountered has been noted on the trial pit records.

On completion of excavating the hand dug trial pits hand held Mackintosh probes were carried out from the base of each excavation to refusal. The results of the probing are given in Figure MP1 in Appendix B.

3.6 INSTRUMENTATION AND MONITORING

On completion of boring, a total of three 19 mm Casagrande type piezometers and two 50 mm slotted standpipes were installed in the exploratory holes. Details of the installations are given on the relevant borehole records.

Observations of the water level in the standpipes were made both during and after the fieldwork period. Water level observations are given in Figures GM1 to GM5 in Appendix B.

3.7 DOWNHOLE TELEVIEWER SURVEY

On completion of boring, downhole optical and acoustic televiewer surveys were carried out and the results are given in Appendix C.

3.8 CROSS HOLE TOMOGRAPHY SURVEY

On completion of boring a shallow seismic cross hole tomography survey was carried out between five boreholes (BH2, BH3, BH4, BH6 and BH12). The survey was carried out for FES by Fugro Aperio Limited and the results are given in their report reference 3525 given in Appendix D.

3.9 FIELD TESTING

Eleven variable head permeability tests were carried out in accordance with BS5930:1999 in seven boreholes (BH1 to BH4, BH6, BH11 and BH12) during the course of boring. The results are given in Figures FT1 to FT11 in Appendix B. Caution should be exercised when using these test results to derive parameters for design purposes. Such determinations in boreholes test only a small volume of soil which may have been disturbed by the boring process. Falling head tests are also very prone to silting up. The permeability value of the soil mass as a whole may be significantly different from the values derived from these tests.



Eighteen packer permeability tests were undertaken in accordance with BS5930:1999 in seven boreholes (BH1 to BH4, BH6, BH11 and BH12) during the course of boring. The results are given in Figures FT12 to FT29 in Appendix B. Very low flow was recorded at each pressure stage during the tests and therefore Lugeon values have not been calculated.

3.10 SURVEY

The positions of the exploratory holes were set out by reference to features shown on the site plan under the supervision of the Engineers site representative.

The ground levels and grid co-ordinates at the exploratory hole positions were related to survey station information presented on the site survey drawing supplied by the Engineer. A Trimble GPS system was used to determine the investigation locations relative to this survey control. The ground levels have been quoted to the nearest 0.05 m on the records and the grid co-ordinates are given to the nearest 1.00 m.

4. RESULTS OF EXPLORATORY HOLES

4.1 GENERAL

Borehole records (Figures FR1 to FR14), concrete corehole record (Figure FR15) and trial pit records (Figures FR16 to FR49) giving details of the strata encountered are provided in Appendix A. A site plan showing the approximate positions of the exploratory holes is presented in Figure LP1 in Appendix G.

The strata descriptions given in the borehole records, unless otherwise noted, are compiled from an examination of the disturbed and core samples only, together with the results of any field and laboratory testing. Relative density descriptions are based on the results of the SPT and have not been amended to take into account any overburden effects. The consistency of cohesive strata is based on visual assessment together with any available laboratory test results. Where there is a degree of uncertainty regarding the relative density or consistency of the soil, the terms "probably" or "possibly" have been used and the descriptions should be treated with caution.

4.2 LIMITATIONS AND USE OF DATA

The scope of the investigation was determined by the Engineer for the particular project requirements set out in the Specification for the Contract. A factual report only was required, without interpretation of the data from the present investigation or consideration of data from other sources, except where noted. The data presented in this report reflects the site conditions encountered at the time the investigation was performed. The investigation has disclosed evidence of conditions at point locations across the site which provides information about discrete volumes of soil or rock. Accordingly, there may be ground conditions at the site which may not have been



revealed by the investigation, and the passage of time may give rise to changes in the conditions encountered. Any interpolation or extrapolation of strata from the exploratory holes is subject to the interpretation of the reader. Any cross - sections or plots are generalised by necessity and have been based on information found at the exploratory holes and depths sampled and tested. The records should be read in conjunction with the Notes on Exploratory Hole Records in Appendix A. Particular attention is drawn to the comments made on groundwater and interpretation which are given in these Notes.

The investigation has been carried out by Fugro Engineering Services Limited and the report has been prepared for the sole internal use of Scottish & Southern Energy. This report shall not be relied upon or transferred to any other parties without the express written authorisation of Fugro Engineering Services Limited. If an unauthorised third party comes into possession of this report they rely upon it at their peril and the authors owe them no duty of care and skill.

It is Fugro Engineering Services Limited's understanding that this report is to be used for the purposes as described in the Specification for the investigation and as summarised in the text of the report. Should the purpose for which the report is used or the proposed use of the site change, this report may no longer be valid. Any further use or reliance upon the report in these circumstances by Scottish & Southern Energy without further review by and advice from Fugro Engineering Services Limited shall be at their sole and own risk.

4.3 STRATA ENCOUNTERED

The exploratory holes encountered the following general succession of strata which, apart from the Made Ground, concurs with that anticipated from published geological records.

TOPSOIL / MADE GROUND Sandy GRAVEL and COBBLES Quartz mica SCHIST

The hand dug trial pits encountered the following general succession of strata, which concurs with that anticipated from published geological records.

Organic TOPSOIL / PEAT Sandy GRAVEL and COBBLES

4.4 GROUNDWATER

Groundwater was encountered in TP07 at 0.50 m, MP02 at 0.40 m, MP05 at 0.35 m, MP06 at 0.55 m, MP08 at 0.50 m, MP09 at 0.20 m, MP10 at 0.40 m, MP11 at 0.40 m,



MP12 at 0.35 m, MP14 at 0.40 m, MP15 at 0.35 m, MP16 at 0.40 m, MP21 at 0.20 m, MP22 at 0.50 m and in MP25 at 0.55 m.

It should be noted that the method of drilling may obscure groundwater strikes. Readings of groundwater levels in the standpipes are given in Figures GM1 to GM5 in Appendix B.

5. GEOTECHNICAL LABORATORY TESTING

5.1 INTRODUCTION

The following laboratory tests were scheduled by the Engineer and carried out by FES in accordance with BS1377:1990^v where applicable. The results are given in Appendix E. *Attention is drawn to the comments on interpretation of the results of the investigation on page KS/01 of the Notes on Exploratory Hole Records.* General Notes on Laboratory Test Results (Figure LKS/01) also precede the laboratory test results in Appendix E.

All tests with the exception of the chemical analyses, seismic velocity tests and aggregate tests were carried out in the Fugro in-house laboratory at Consett and the tests for which the laboratory have UKAS accreditation are detailed on the Schedules preceding the laboratory test results in Appendix E.

The chemical analyses were undertaken by Derwentside Environmental Testing Services Limited (DETS), the seismic velocity tests were carried out by Reading University and the aggregate tests were carried out by Environmental Services Group Limited, whose laboratories are accredited for the tests undertaken.

5.2 INDEX PROPERTIES

Natural moisture content determinations were made on four samples of the granular soils and liquid and plastic limit determinations were made on a single sample of the cohesive soils in order to classify the plasticity of the materials and the results are given on the Summary of Classification Tests (Figure LT1/1 in Appendix E).

5.3 PARTICLE SIZE ANALYSES

Particle size analyses were undertaken on a total of twenty samples in order to classify the materials in respect to their grain size. The particle size analyses were carried out by sieving and continued by sedimentation. The results are given as particle size distribution curves (Figures LT2/1 to LT2/20 in Appendix E).



5.4 ROCK TESTS

The slake durability of three samples of rock core was determined and the results are given on the Summary of Rock Classification Tests in Figure LT8/1 in Appendix E.

The point load index using the methods outlined by the ISRM Commission on Testing Methods, 1985, was determined for one hundred and thirty specimens taken from samples of rock core. The results are given in Figures LT8/2 to LT8/16 in Appendix E.

The Unconfined Compressive Strength (UCS) of thirty two samples of rock core was determined using the method outlined in ISRM Suggested Methods 1981 vi. The results are given in Figures LT8/17 to LT8/23 in Appendix E.

The Brazilian tensile strength of eight samples of rock core was determined using the method outlined in ISRM Suggested Methods 1981 ^{vii}. The results are given in Figures LT8/17 to LT8/23 in Appendix E.

Six hoek shear tests were carried out on specimens taken from samples of rock core. The results are given in Figures LT8/24 to LT8/41 in Appendix E.

The Aggregate Crushing Value, Aggregate Impact Value, LA Abrasion and Magnesium Sulphate Soundness was determined for ten specimens taken from samples of rock core in accordance with BS EN 1097:1998 viii and BS812:1990 ix. The tests were carried out for FES by Environmental Services Group Limited and the results are presented as their test certificates in Appendix E.

The seismic velocity of six samples of rock core were determined. The tests were carried out for FES by Reading University and the results are presented in their test certificate at the end of Appendix E.

Chemical analyses were made on eleven samples of rock in order to determine their pH value and total sulphate content. The tests were carried our for FES by DETS and the results are given in their Certificate of Analysis number 10-36994 at the end of Appendix E.

6. CONTAMINATION TESTING

6.1 INTRODUCTION

The following laboratory tests were scheduled by the Engineer and carried out for FES by Derwentside Environmental Testing Services (DETS) whose laboratory is accredited by UKAS, and details of their current accreditation may be obtained from them.



6.2 CHEMICAL ANALYSES ON SOIL SAMPLES

A total of thirteen soil samples from the exploratory holes were analysed for some or all of the following suite of determinands:

Arsenic (total)

Cadmium (total)

Chromium (total)

Lead (total)

Mercury (total)

Selenium (total)

Copper (total)

Nickel (total)

Zinc (total)

Boron (water soluble)

pН

Sulphate (total)

Sulphide

Cyanide (total / complex / free)

Phenols (speciated)

Total Petroleum Hydrocarbons (TPH – by gas chromatography)

Polychlorinated Biphenyls (PCB - as Aroclors, screen)

Semi-volatile organic compounds (SVOCs)

Asbestos screen

In addition, the pH value and water soluble sulphate content was determined for eight samples of soil and the organic content of two samples of soil were determined.

The results are given in DETS Certificate of Analysis reference numbers 10-35469, 10-36315, 10-36951 and 10-37631 in Appendix F.



REFERENCES

- Norbury, D.R., Child, G.H., and Spink, T.W., 'A critical review of Sections 8 (BS 5930:1981), Soil and rock descriptions, Geological Society Engineering Geology Special Publication No 2, Proceedings of 20th Regional Meeting of the Engineering Group, Guildford, 1986.
- ^v BS 1377:1990, Methods of tests for soils for civil engineering purposes. British Standards Institution
- vi ISRM Suggested Methods Rock Characterization, Testing and Monitoring Editor E. T. Brown, International Society of Rock Mechanics, 1981.
- vii ISRM Suggested Methods Rock Characterization, Testing and Monitoring Editor E. T. Brown, International Society of Rock Mechanics, 1981.
- viii BS EN 1097:1998 Tests for Mechanical and Physical Properties of Aggregates. British Standards Institution.

ⁱ BS 5930:1999, Code of practice for site investigations. British Standards Institution.

ⁱⁱ BS EN ISO 14688-1:2002 Geotechnical investigation and testing – Identification and classification of soil – Part 1 Identification and description. British Standards Institution.

ⁱⁱⁱ BS EN ISO 14689-1:2003 Geotechnical investigation and testing – Identification and classification of rock – Part 1 Identification and description. British Standards Institution.

ix BS 812:1990 Testing Aggregates. British Standards Institution.



APPENDIX A Exploratory Hole Records

Notes and Key Sheets on Exploratory Hole Records Borehole Records Concrete Corehole Records Trial Pit Records Figures KS/01 to KS/06 Figures FR1 to FR14 Figure FR15 Figures FR16 to FR49

GENERAL NOTES

1 OPERATING PROCEDURES

The procedure used for cable percussion boring, rotary drilling, trial pitting, sampling, in situ and laboratory testing and sample descriptions are generally in accordance with BS5930:1999 'Code of practice for site investigations', BS EN ISO 14688-1:2002 'Geotechnical investigation and testing – Identification and classification of soil – Part 1 Identification and description', BS EN ISO 14689-1:2003 'Geotechnical investigation and testing – Identification and classification of rock – Part 1 Identification and description' as appropriate, and BS1377:1990 'Methods of test for soils for civil engineering purposes', unless stated otherwise.

2 GROUNDWATER

Exploratory hole water levels are recorded together with the depths at which seepages or inflows of water are detected. These observations are noted on the Records, but may be misleading for the following reasons:

- a) The exploratory hole is rarely left open at the relevant depth for a sufficient time for the water level to reach equilibrium.
- b) A permeable stratum may have been sealed off by the borehole casing.
- c) Water may have been added to the borehole to facilitate progress.
- d) The permeability may have been altered by the excavation/boring/drilling process.

Standpipes or piezometers should be installed when an accurate record of groundwater level is required, however, it should be noted that groundwater levels may vary significantly due to seasonal, climatic or man made effects. Water levels recorded during the investigation and any advice or comment made accordingly may, therefore, not be appropriate to particular foundation, geotechnical design, or temporary works solutions. Long term monitoring of standpipes or piezometers is always recommended when water levels are likely to have a significant effect on design.

3 CHISELLING

The remarks in the Borehole Records contain information on the time spent advancing the borehole by 'Chiselling Techniques', and the depth of borehole over which it was required. Such information may be affected by a wide range of variable factors, unrelated to the geotechnical properties of the strata. Such factors include, but are not restricted to: plant, equipment and operator. The data should, therefore, only be used subjectively and with extreme caution.

4 IDENTIFICATION AND DESCRIPTION OF SOILS - SEE SEPARATE SHEET

The identification system follows the Company's Engineering: Geotechnical Procedures Manual which is based on BS EN ISO 14688-1:2002 and appropriate clarifications in the National Foreword, BS 5930:1999 and BS EN ISO 14689-1:2003

Relative density terms are given where supported by SPT N values, with the exception of Made Ground. The field assessment of compactness or relative density for coarse grained soils is only given on trial pit records where appropriate assessment of the soils has been undertaken.

Where the terms 'soft to firm', 'firm to stiff' etc. are used they indicate a strength which is close to the borderline between the two terms and cannot be precisely defined by inspection only, and/or which is indicated as borderline or ranging between the two terms after consideration also of in situ and laboratory test results. Consistencies may have been amended in the light of test results

Where 'to' links two terms, as in 'slightly sandy to sandy' this again represents a borderline case or a range, where the precise proportions cannot be determined as outlined previously.

The name of the geological formation is only given where this has been requested and can be determined with confidence (see Clause 41.5 of BS 5930:1999).

5 INTERPRETATION OF THE RESULTS OF THE INVESTIGATION

The description of ground conditions encountered and any engineering interpretation included in the report are based on the results of the boreholes and trial pits and the field and laboratory testing carried out. There may be ground conditions at the site which have not been revealed by the investigation and consequently have not been taken into account.

Any interpolation or extrapolation of strata between exploratory holes shown on any cross sections or site plans is an estimate only of the likely stratification based on general experience of the ground conditions and is subject to the interpretation of the reader.

The term "TOPSOIL" is used in this report to describe the surface, usually organic rich, layer including turf, subsoil and weathered material with roots. The use of this term may not imply that the soil satisfies the requirements of Clause 3 of BS 3882:1994, 'Specification for topsoil', or is suitable for general horticultural and agricultural purposes.

Laboratory test results in this report give the soil properties of individual specimens tested under specified conditions. Individual results or groups of results may not be appropriate for use as design parameters for some geotechnical analyses. The samples may be non-representative, disturbed internally, or prepared and tested under conditions suited for different geotechnical applications. Unless the selection of design parameters is discussed in this report, it is recommended that the advice of a Geotechnical Specialist is sought.

IN SITU TESTING AND SAMPLING

STANDARD PENETRATION TESTS

- S() Standard Penetration Test (SPT). A 50mm diameter split barrel sampler is driven 450mm into the soil using a 63.6kg hammer with a 760mm drop. The penetration resistance (also known as the 'N' value) is expressed as the number of blows required to obtain 300mm penetration below an initial seating drive of 150mm which is taken through any ground which may be disturbed at the base of the borehole. The test is usually completed when the number of blows recorded during the test drive only reaches 50 in soils or 100 in weak rock. If a sample is not recovered in the sampler, a disturbed sample is taken on completion of the test and given the same depth as the top of the Standard Penetration Test drive.
- C() Standard Penetration Test carried out with a 60 degree cone. The test is usually conducted in coarse granular soils or weak rock using the same procedure as for the SPT, but with a 50mm diameter, 60 degree apex, solid cone fitted to the split barrel. A bulk disturbed sample is taken and given the same depth as the top of the test drive.

The depth on the borehole record at the left hand side of the 'depth' column is that at the start of the normal 450mm penetration. Where the full penetration of 300mm for the test drive is obtained, the penetration resistance ('N' value) is reported in the 'SPT Blows/N' column. If the full penetration of 300mm in the test drive is not obtained, then the length of drive (test length in mm) and the penetration resistance (number of blows) are both reported. Blows through the initial seating drive (normally 150mm) are not reported.

* in the 'Test Length' column denotes that the blows and penetration were all in the initial Seating Drive section.

OTHER IN SITU TESTS

The following in situ tests are reported on the **Exploratory Hole Records**, in the 'Test' or 'Type' and 'Results' columns where appropriate.

- k In situ Permeability Test refer to detailed test results for permeability values
- PMT Pressuremeter Test refer to detailed test results for modulus values, etc.
- VN/R() Borehole Shear Vane Test (undrained shear strength cu in kPa) refer also to detailed test results, N 'Natural' or peak shear strength, R Remoulded shear strength
- VN/R() Hand Shear Vane Test (Direct reading of undrained shear strength in kPa). 'N' and 'R' as above. The values are indicative and should not be taken as being equivalent to laboratory test results. The Pilcon vane results have a factor varying from about a sixth for the 33mm vane to a third for the 19mm vane which reduces the BS1377 shear vane value. The values presented are therefore approximate and should be treated with great caution if used for design purposes
- PP() Pocket Penetrometer. Unconfined Strength (UCS) reported in kg/cm² to the nearest 0.25 kg/cm² or kPa with the same accuracy. Equivalent c_u in kPa is very approximately UCS x 50. Pocket Penetrometers are an aid to logging of cohesive soils, the results are indicative and should not be relied upon. The equipment used is not calibrated
- CBR() California Bearing Ratio Test (CBR%) refer also to detailed test results
- PID() Photo-Ionisation Detector Readings in headspace of small disturbed chemical samples. Result given in ppm by volume

SAMPLES

- U General purpose open tube sample. Sample normally taken with open tube sampler approximately 0.1m diameter and 0.45m long and driven with 80kg sinker bar and 56kg sliding hammer, unless noted otherwise. "XX" in U100 blows column denotes the number of hammer blows. The height of hammer drop can be variable depending on operator technique. Depths are given to the top of the sample if full penetration and recovery are achieved, otherwise actual lengths of penetration and recovery are given in the appropriate columns.
- U(X) General purpose open tube sample (X) mm diameter

TW(X) Thin wall (push) sample (X) mm diameter

P(X) Piston sample (X) mm diameter CBR Sample taken in CBR Mould

D Small disturbed sample (plastic tub or jar with air tight lid)

B Bulk disturbed sample (polythene bag, tied at neck - size dependent on purpose)

W Water sample

Sample not recovered

C Core sample (CS – short core, generally about 100mm; CL – long core, generally 200mm to 300mm)

CD Sample for chemical analysis in a plastic tub K Sample for chemical analysis in an amber

glass jar

V Sample for chemical analysis in a glass vial CDKV Set of samples for chemical analysis as above

WAC Sample for Waste Acceptance Criteria

ES Environmental Sample EW Environmental Water Sample

KEY TO BOREHOLE AND TRIAL PIT RECORDS

Soil T	ypes				
Coars	se grained, Non cohesive	Fine	grained, Cohesive	Other	Soil Types
	Boulders	:	Silt		Topsoil
- 1 - 1 - 1 - 1 - 1 - 1 - 1	Cobbles		Clay		Peat
	Gravel				Made Ground
	Sand	Note:	Composite soil types may be signified by combined symbols.		
Rock	Types				
	nentary Sandstone		Chalk		Coal
	Siltstone		Limestone		Mudstone/Claystone/Shale
	Conglomerate		Breccia		
	Matamanakia				
	Metamorphic	+ + +	Igneous		
****	Coarse/Medium grained	+++	Coarse grained		
******* ******	Fine grained	++++ ++++ ++++	Medium grained		
		www.	Fine grained		
	KEYT	O SITE	PLANS AND CROSS SECTIONS	•	Borehole Position
	Highest recorded water	er level in	piezometer or standpipe		Trial Pit Position
	Length of piezometer/s	standpipe	e response zone		
0	(● Tip Depth)			A	A
gen	▼ Highest recorded wate	er level in	hole		Line of Section
Pe	✓ Water strike				
Borehole Legend		test (SPT)) "N" value using split spoon		
ore) "N" value using solid 60° cone		
Ď	cu() Undrained cohesion in		, it take using solid out colle		

DESCRIPTION OF ROCK CORES

DESCRIPTIVE ORDER

Strength, Structure, Colour, Texture, Grain Size, ROCK NAME. Minor constituents and additional information. (Geological formation - see comments under identification and description of soils). Mass characteristics - factual description of weathering state (if appropriate) and description of discontinuities and fracture state (if appropriate).

Term	Field identification	Strength (MPa)
Extremely weak	Can be indented by thumbnail. Gravel sized lumps crush between finger and thumb.	<1.0
Very weak	Crumbles under firm blows with point of geological hammer. Can be peeled by a pocket knife.	1 – 5
Weak	Peeled by a pocket knife with difficulty. Shallow indentations made by firm blow with point of geological hammer.	5 – 25
Medium strong	Cannot be scraped with pocket knife. Can be fractured with a single firm blow of geological hammer.	25 – 50
Strong	Requires more than one blow of geological hammer to fracture.	50 – 100
Very strong	Requires many blows of geological hammer to fracture.	100 – 250
Extremely strong	Can only be chipped with geological hammer.	> 250

DISCONTINUITIES

Bedding Spacing & Planar Structures *	Spacing (mm)	Discontinuity Spacing
	>6000	Extremely widely spaced
Very thickly bedded	>2000 2000-6000	Very widely spaced
Thickly bedded	600 - 2000	Widely spaced
Medium bedded	200 - 600	Medium spaced
Thinly bedded	60 - 200	Closely spaced
Very thinly bedded	20 - 60	Very closely spaced
Thickly laminated (Sedimentary) narrow (Metamorphic & Igneous)	6 – 20 <20	Extremely closely spaced
Thinly laminated (Sedimentary) Very narrow (Metamorphic & Igneous)	<6	

For igneous and metamorphic rocks the appropriate descriptive term for planar structure should be used e.g. medium foliated gneiss, very narrowly cleaved slate, very thickly flow banded diorite.

WEATHERING

Standard descriptions of weathered rocks for engineering purposes should always include comments on the degree, extent and nature of any weathering effects at material or mass scales. This may allow subsequent classification and provide information for separating rock into zones of like character. Indications of weathering include

changes in colour changes in fracture state

reduction in strength presence, character and extent of weathering products

If a systematic classification following the guidelines given in the Standard can be applied unambiguously, this is described in the text of the report. Otherwise, the rocks are not classified in terms of weathering beyond the approach described above.

Weathering terms that may be used for description of rock material and these terms may be qualified or combined.

Discoloured The degree and type of colour change from original is described, and if for mass or particular mineral

constituents

Disintegrated Fragmentation by physical weathering, bonding lost but material fabric is intact. Material friable, not

decomposed

Decomposed Chemical alteration of mineral grains so material fabric is intact but some or all grains are decomposed

For rock mass weathering the following terms may be used

Slightly weathered Discolouration on surfaces and / or of material

Moderately Less than half of mass decomposed/disintegrated. Fresh/discoloured rock as continuous material or

corestones

Highly More than half decomposed/disintegrated. Fresh/discoloured rock as discontinuous framework or

corestones

Completely All rock material decomposed and/or disintegrated. Original mass structure largely intact

Residual Soil All material converted to soil, structure and fabric destroyed, may be volume change but material not moved The term 'Fresh' is used to indicate that there is no visible weathering or alteration, except possibly slight discolouration on major surfaces.

ROCK CORES

ROCK CORE SIZES

The core barrels commonly used by the Company in site investigations are as follows:

			site investigations a		0 : 0 0	0 : 10
Core Barrel	Borehole	Standard Core	Core Size using	Casing Size	Casing O.D	Casing I.D
Type	Diameter	Size	Rigid Plastic Liner	or Type	(mm)	(mm)
	(mm)	(mm)	(mm)			
	NDARD BRITISH	-				
NWM	75.7	54.7	51	NX	88.9	76.2
HWF	98.8	76.2	72	HX	114.3	100.0
HWAF	99.5	70.9	-	HX	114.3	100.0
PWF	120.0	92.1	87	PX	139.7	122.3
SWF	145.4	112.8	107	SX	168.3	147.7
UWF	173.7	139.8	132	UX	193.7	176.2
	WIRELINE SIZE		_			
BQ	59.9	36.4	35			
NQ	75.7	47.6	45			
HQ	96.1	63.5	61			
PQ	122.7	85.0	82			
GEOBORE	146.0	102.0	102	SX	168.3	147.7
S						
	THINWALL SIZE					
TNX	75.7	60.8	-	NX	88.9	76.2
T2 66	66.1	51.9	-	74	74.3	67.3
T2 76	76.1	61.9	-	84	84.3	77.3
T2 86	86.1	71.9	68	98	98.0	89.0
T2 101	101.1	83.9	80	113	113.0	104.0
T6 116	116.1	92.9	89	128	128.0	118.0
T6 131	131.1	107.9	104	143	143.0	133.3
	N STANDARD BAI					
4.12F	105.2	74.7	72	PX	139.7	122.3
TRIEFUS						
5.5x4C	139.7	101.6	-	SX	168.3	147.7
0,1,0,5						
SINGLE						
TUBE				5.4	400 =	400.0
B116	116	102	-	PX	139.7	122.3
B146	146	132	-	SX	168.3	147.7

Note: Core diameters may vary when different lining systems are in use.

ROCK CORE CHARACTERISTICS

- TCR **Total Core Recovery.** The length of the total amount of core sample recovered, expressed as a percentage of the length of the core run.
- SCR Solid Core Recovery. The length of solid core recovered, expressed as a percentage of the length of the core run.

Solid core is defined as that length of core which has a full diameter, but not necessarily a full circumference. Only natural fractures are considered. Drilling or handling induced fractures are ignored.

- RQD Rock Quality Designation. The length of solid core recovered in pieces each more than 100mm long as a percentage of the core run length.
- I_f Fracture Index. The number of discontinuities expressed as 'fractures per metre', measured over any convenient length of consistent fracture characteristics.

Zones of atypical fracturing of restricted extent which occur within a rock unit of uniform fracture characteristics are identified within the Description of Strata.

NI - Not Intact NR - No Recovery NA - Not Applicable

 I_{s} Corrected **Point Load Strength Index** $I_{s(50)}$ which is given in MPa

IDENTIFICATION AND DESCRIPTION OF SOILS

	Basic Soil Typ		Particle Siz mm)	е	Visual Identification	Composite Soil Ty (Mixtures of basic s	es)		Density / Consistency / Peat Condition					
VERY COARSE SOILS	BOULDE	RS		200	Large Boulders >630mm. These soils only seen complete in pits or exposures. Often difficult to recover from boreholes.	Scale of secondary coarse soils. Term Term before (term in '[]' may	before	, descript	ion after pr				ative description and particle	
		C	coarse	63	Easily visible to naked eye; particle shape can be described, grading can	be used for 2 nd ry in the second parts, matrix etc) in the second parts, matrix etc) in the second parts, matrix etc) in the second parts in th					Standard Per for Coarse So	netration Test	in Boreholes	
/el Sizes)		n	nedium	6.3	be described. Well graded: wide range of grain	Slightly (sandy*) [occasional / little]	SLES F	compone		<5	No of blows	Relative Dens Very Loose	sity	
d Gravel	GRAVE	L		0.0	sizes, well distributed. Poorly graded: not well graded. (May be uniform: size of most particles lies between		(COBBLES e Notes)	seconda constitue e.g. Gra	ents.		4-10	Loose		
Sand and		fi	ine		narrow limits; or gap graded; an intermediate size of particle is	[some] , $\overset{\circ}{\wp}$ fin			medium lar fine	5 – 20	10-30	Medium Dens	se	
%		c	coarse	2	markedly under represented). Visible to naked eye; no cohesion when dry; grading can be described.	Very (sandy*) [much / many]	SAND, GRAVEL or BOULDERS	sandstor mudston		20 to 40†	30-50	Dense		
E SOILS y over 65%	SAND	n	nedium	0.63	Well graded and poorly graded: as above		SAND, or BOL	and (san		50†	>50	Very Dense Visual Examir	nation: nick	
COARSE SOILS (Typically over 65'		fi	ine	0.2		* Fine or coarse + Very coarse so	soil typ oil type	e as appr – see Not	opriate tes		Slightly cemented		in lumps which	
ŏ E		c	coarse	0.063	Only coarse silt visible with hand lens;	† described as fi Scale of secondary	consti	tuents wit	h fine soils			clayey SILT – ι		
	SILT	n	medium	0.02	exhibits little plasticity and marked dilatancy; slightly granular or silky to touch. Disintegrates in water; lumps	before, description		rincipai co	instituent.	Approx	affect on mate	ary constituent le rial characteris ry' not applicab	tics. Terms	
		fi	ine	0.0063	dry quickly; possesses cohesion but powders easily between fingers.	Term before	Principal Soil Type	Descript	ion after	% 2 nd ry soil type	Consistency	· · ·		
Silt and Clay Sizes)				0.002	Term "SILT" or "CLAY" must be used, "SILT/CLAY" not allowed.	Slightly (sandy*)		Used to compone seconda		<35	Very soft		pushed in up to udes between	
d Clay					Dry lumps can be broken but not powdered between the fingers; they	(sandy*)	or SILT	constituents e.g. gravelly		35 to 65†	Soft	Finger push 10mm. Mould		
	CLAY				also disintegrate under water but more slowly than silt; smooth to the	Very (sandy*)	CLAY o	sandy C Gravel is rounded		>65†	Firm	Thumb mak easily. Rolls to	es impression thread	
-INE SOILS Typically over 35%					touch; exhibits plasticity but no dilatancy; sticks to the fingers and dries slowly; shrinks appreciably on drying usually showing cracks.	* Coarse soil type a † or described as o behaviour	as appi	opriate	•	ass	Stiff	Can be inde thumb. Crumb	nted slightly by oles if rolled	
FINE SOILS (Typically ov					Intermediate and high plasticity clays show these properties to a moderate	EXAMPLES OF CO					Very Stiff	Cannot be mo		
FINE (Typ					and high degree, respectively.	Loose brown very s	sandy s	subangula	r coarse G		Hard	Can be scrato nail	ched by thumb	
S	ORGAN CLAY,	\	/aries		Contains varying amounts of organic vegetable matter - defined by colour: grey - slightly organic;	with many pockets Firm thinly interlam		•	Firm Peat Spongy Peat		ressed together ssible, open			
ORGANIC SOILS	SILT SAND	or			dark grey – organic; black – very organic.	Dense light brown clayey fine and medium SAND.					Plastic Peat		ded in hand, smears	
Structur	е												Particle Nature	
Term	F	ield Ide	entification			Interval Scales							Particle Shape &	
Homo- geneous			consists es			Scale of Bedding S	pacing		Mean Sp (mm)	acing	Scale of Spac Discontinuities		Form Very angular	
Interbedo interlamin	nated in	n equal		s. Otherw	types. Pre-qualified by thickness term if ise thickness of, and spacing between,	Very thickly bedded	b		over 200	0	Very widely sp large]	oaced / [Very	(Sub) angular (Sub) rounded Well rounded	
Hetero- geneous		mixtu	re of types			Thickly bedded			2000-600)	Widely spaced	d / [Large]	Low Sphericity	
Weathers (granular)	Particle	s may be w	eakened a	and may show concentric layering	Medium bedded			600-200		Medium space	ed / [Medium]	Flat or Elongate	
(cohesive	e) U				ar structure	Thinly bedded			200-60		Closely space		High Sphericity	
Fissured Sheared					olished discontinuities shed discontinuities	Very thinly bedded Thickly laminated			60-20 20-6		Very closely /	[Very small]	Cubic	
Intact	N	lo fissu	ures			Thinly laminated			under 6		Extremely clos	sely spaced	Particle	
Fibrous F	s	queeze	ed only wat	er, no soli	and retain some strength. When ds strength lost. Partial decomposition.	Spacing terms may also be used for distance between partings, isolated beds or laminae, desiccation cracks, rootlets etc. Terms such as partings or dustings may be						Surface Texture		
fibrous P	eat T	urbid v	water when	squeezed	, <50% solids	used for laminae less than 2mm and less than 0.6mm r					spectively.	Rough		
Amorpho Peat	S	queeze	ed only pas	te with >5		Discontinuity Shape (See Standard for	е		scale (mm' n scale (cr		smooth r, stepped, undu	ılating	Smooth Polished	
Gyttja Humus	Р	lant re	mains, livin	g organisr	ns & inorganic constituents in topsoil	Persistence/Openn	,	Large	scale (m ³ s)	wavy, cur	ved, straight	rolished		

NOTES Identification and descriptive method, and descriptions, generally in accordance with BS5930:1999 Section 6 clauses 41 and 43 and BS EN ISO 14688-1:2002

NOTES Identification and descriptive method, and descriptions, generally in accordance with BS5930:1999 Section 6 clauses 41 and 43 and BS EN ISO 14688-1:2002
Additional notes relating to BS EN ISO 14688-2:2004 – modified terms for content of secondary fraction given in Annex B Table B1 are not comparable to 5930 and are not be used.
Organic Content: - Low – 2 to 6%; Medium - 6 to 20%; High - >20%. Terms not used on borehole records
Carbonate content: - Only noted if field test with dilute HCI undertaken – Carbonate free if no effervescence; Calcareous if slight effervescence; Highly calcareous if strong reaction
Undrained shear strength: - terms from laboratory or in situ tests not given on borehole records.
Very Coarse Soils – described by initially removing very coarse materials and describing residue before adding back the very coarse soils. If residue is cohesive then described as
'......(COBBLES / BOULDERS) with low (cobble / boulder) content with (some / much etc) matrix of' If residue is granular then described as 'with matrix of ' or as a coarse soil.
Cobbles: - <10% - low cobble content; 10 to 20% - medium content; >20% - high content;
Boulders <5% - low boulder content; 5 to 20% - medium content; >20% - high content

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No.** BH₁ 200mm to 5.60m 200mm to 5.60m Coordinates (Local Grid) Ground Level Equipment 140mm to 20.00m 232110 E 140mm to 20.00m Dando 2000 Knebel 709828 N 11.74 m OD **Drill Crew** Logged by Compiled by Approved by

Dates	Drilled	End	26/01/2010 03/02/2010			02/02/	2010 2	9/03/2010 12/04/2010			i .
Date & Time	Casing Depth (m)	Depth to Water	Sample Depth (m)	Details Type	1	SPT Blows/N Drive mm	U100 Blows/ Recovery mm	Description of Strata	Depth (Thick- ness)	Level	Leger
	(,	(m)	From To	туре	NO.	Test	Result		(m)		/ / × / /
26/01			- - - - - - - - - - - - - - - - - - -	В	1			Grass over TOPSOIL. MADE GROUND: Composed of brown grey sandy angular to subangular fine to coarse gravel of mica schist with many subangular cobbles. Sand is medium to coarse. MADE GROUND: Mica schist boulder.	(0.10) 0.10 (0.20) 0.30 (0.20)	11.64 11.44 11.24	
								MADE GROUND: Composed of medium dense grey brown angular and subangular fine to coarse gravel of mica schist with occasional subangular boulders. Between 0.50m and 1.20m: Frequent roots. Below 0.90m: Decreasing organic content.	(0.70)		
6/01		DRY	-						Ē		
27/01			1.20-1.65 1.20-1.70		2	C32		Possible MADE GROUND: Composed of dense dark brown organic sandy angular to subangular fine to coarse gravel of mica schist and quartz with many angular cobbles and occasional subangular boulders >150mm. Sand is fine to coarse.	- 1.20 	10.54	
			_ 2.00	С		C50/		At 2.00m: Becoming very dense.	- - - - -		
	1.90	DRY	2.00 2.00-2.50	D B	3 4	95			- - - -		
									-		
			2.70	D	5				- - - -		
	2.90	DRY	3.00-3.45 3.00-3.50		6	C50/ 180			- - - - -		
									(4.40)		
			3.70	D	7				- - - -		
	3.90	DRY	4.00-4.45 - 4.00-4.50		8 9	C50/ 105			- - - -		
			- - - - -								
			4.70	D					- - - -		
	4.90	DRY	5.00-5.45	С		C50/			<u> </u>		

Remarks 1 (See notes & keysheets) 2

Scale 1:25

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.

2 Rotary coring was carried out using Geobore drilling techniques.

3 The borehole was advanced by cable percussive drilling to 5.60m and then progressed by rotary coring means to 20.00m.

4 An amount of water was added to facilitate boring in granular strata.

5 Borehole advanced by chiselling from 2.30m to 2.60m (60 mins); from 4.30m to 4.70m (45 mins); from 5.50m to 5.60m (45 mins).



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR1 (1 of 4)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No.** BH1 200mm to 5.60m 140mm to 20.00m 200mm to 5.60m 140mm to 20.00m Equipment Dando 2000 Knebel Coordinates (Local Grid) Ground Level 232110 E 709828 **N Drill Fluid** 11.74 m OD Water **Drill Crew** Logged by Compiled by Approved by Start End **Dates Drilled** 26/01/2010 29/03/2010 02/02/2010 12/04/2010 03/02/2010

		End	03/02/2010			02/		.0 29/0	3/2010 12/04/2010			
Date	Casing	Water Depth	Sample/Co	_	r -		SPT Blows	Result or U100		Depth		
&	Depth	(m)	Depth (m)	Туре	No.	İ	Blows /N	Blows/ Rec. mm	Description of Strata	(Thick-	Level	Legend
Time	(m)	(Flush Return) %		TCR %	SCR %	RQD %	Core Size (mm)	or Fracture Index		ness) (m)		
		70	-		,.		255	muex		(,		
			5.00-5.50	В	10					‡		
			-							‡		
										ļ.		
			-							Ŧ		
27/01		DRY	E							F		
28/01		1.90	5.60	С		/	C75/ 85*		Very strong fractured foliated dark grey	5.60	6.14	~~~
		(0)	5.60-5.60	NA	NA	NA			micaceous SCHIST interbedded with quartz. Fresh.	ţ		~~~
		(100)	5.60-5.85	100	80	48/				‡		$\sim\sim$
			_							<u>L</u>		~~~
			-							‡		~~~
			-					0		‡		$\sim\sim$
			- -							F		~~~
			-						Between 6.45m and 7.35m: Induced	Ŧ		~~~
			F						fractures, closely spaced inclined	F		~~~
		(100)	5.85-7.35	100	97	39			horizontal undulating rough moderately open clean.	Ē		~~~~
			[E		~~~~
										ŧ		$\sim\sim$
			_							<u> </u>		$\sim\sim$
			<u> </u>							┢		~~~
			-							‡		~~~
			-							<u> </u>		~~~
			- -				ł	3	Between 7.32m and 7.54m: Fine to medium sand.	‡		~~~
			-						Between 7.35m and 8.35m: Closely spaced	‡		~~~
			-						inclined horizontal fractures planar smooth and rough. Locally infilled <5mm.	F		~~~
			-							Ŧ		~~~
			-							ŧ.		~~~~
			-							Ŧ		$\sim\sim$
			_							L		~~~
		(100)		100	100					F		~~~
		(100)	7.35-8.85	100	100	57				ŧ		~~~
			_					10		(5.25)		~~~
			-							ţ		~~~
			_							<u>L</u>		~~~
			-							‡		~~~
			ļ						Between 8.70m and 10.35m: Frequent quartzite bands.	‡		~~~~
28/01	8.85	DRY	<u>-</u>						Between 8.85m and 9.50m: Closely spaced	F		~~~~
29/01	8.85	4.90						AZCL	horizontal and subhorizontal stepped smooth and occasional rough locally	Ŧ		$\sim\sim$
			F						stained yellow and local quartzite veining on surfaces.	F		~~~
			E						verning on surfaces.	F		~~~
			E							E		~~~
		(100)	- 8.85-9.90	97	4	3		5		t		~~~
		(100)	5.63-9.90	,	*					ţ		~~~
			F							 		~~~
			‡							ţ		
			<u> </u>							‡		
			ļ.					NI	Between 9.90m and 10.35m: Closely spaced	‡		~~~~
1			ļ .	1			1		horizontal stepped smooth and planar rough open clean.	‡		~~~
									10491 Open Clean.	<u> </u>		

Remarks 6 (See notes & keysheets) Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No. FR1 (2 of 4)

f 4)

26/01/2010

03/02/2010

Equipment Dando 2000 Knebel

Drill Crew

Dates Drilled

Drill Fluid Water

Start End

Borehole Diameter 200mm to 5.60m 140mm to 20.00m **Casing Diameter** 200mm to 5.60m 140mm to 20.00m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232110 E 709828 **N** 11.74 m OD

BH1

Logged by Compiled by Approved by 02/02/2010 29/03/2010 12/04/2010

	ı———	Ena	03/02/2				02/		.0 29/0	3/2010 12/04/2010			
Dete	Cocina	Water Depth	San	nple/Co	re Rec	overy		SPT Blows	Result		Depth		
	Casing	(m)		, ,	Туре	No.		/N	or	December of Conta	(Thick-	Level	Legend
& -:	Depth	(Flush Return)	Depth	(m)			DC -	Core Size (mm)	Fracture	Description of Strata	ness)		
Time	(m)	Return)	From	To	TCR	SCR %	RQD %	Size	Index		/m)		
<u> </u>		% ′		. 0	/0	/0	/0	()			(m)		
		(100)	0.00	-10.35	100	89	22				İ		~~~
		(100)	9.90-	-10.35	100	89	22				ŀ		$\sim\sim$
			Ī								Į.		~~~
										Between 10.35m and 10.85m: Very closely	ţ		$\sim\sim$
			_							spaced horizontal and subhorizontal planar smooth moderately open clean.	-		~~~
			_						7	Probably drilling induced	<u>L</u>		$\sim\sim$
											t		~~~
			-								ŀ		$\sim\sim$
											<u> </u>		~~~
											t		$\sim\sim$
			-							Strong fractured foliated dark and light	10.85	0.89	~~~
										grey quartz mica SCHIST. Slightly	Ĺ		$\sim\sim$
										weathered. Fractures are subhorizontal to	ŀ		~~~
		(100)	10.35-	-11.85	100	100	39			subvertical very closely to medium spaced planar smooth and undulating smooth and	F		$\sim\sim$
			Ĺ							frequently cemented with quartz.	<u>t</u>		~~~
			<u> </u>							Between 10.85m and 11.85m: Closely spaced	ţ		~~~~
I	I		<u> </u>							fractures subhorizontal to inclined 15 degrees planar rough moderately open to	ļ.		~~~
			_							open, locally infilled grey fine silty	<u>L</u>		~~~~
			<u> </u>							sand <2mm.	t		~~~
			Ī								Ţ.		~~~
											<u> </u>		$\sim\sim\sim$
											ŀ		~~~
									2		Ī		~~~
			_								<u></u>		~~~
			_								ŀ		~~~
											Į		~~~
			L							Between 12.30m and 13.35m: Induced	t		~~~
			-							fractures closely spaced subhoriontal to	ŀ		$\sim\sim$
										inclined 15 degrees planar smooth and	ļ.		~~~
			_							striated moderately open clean.	Ŀ		~~~
			-								ŀ		~~~
		(100)	11.85	-13.35	100	100	61				‡		~~~
			L								Ł		~~~
			[Ę		$\sim\sim$
											ţ		~~~
			_								-		$\sim\sim$
											F		~~~
											ţ		$\sim\sim$
I	I		-								ł		~~~
			F							Between 13.35m and 14.85m: Possibly	F		~~~~
					1					induced fractures closely spaced	ţ		~~~
I	I		<u>L</u>							undulating striated and planar smooth moderately open to open clean.	Ł		~~~~
			F								F		~~~
			-								ţ		~~~~~
			_								ł		~~~
I			F						6		F		~~~~~
I	I		<u> </u>								ţ		~~~
			1								}		~~~~
											F		~~~
		(100)	13.35-	14.85	100	97	37				t		~~~~
			ļ								ł		~~~
			-								†		~~~~
			<u> </u>								}		~~~
			-								Ī		$\sim\sim$
			<u> </u>								F		~~~
			1								}		$\sim\sim$
			ļ								ţ		·~~
29/01	14 85	DRY	ŀ							Between 14.85m and 15.15m: Closely spaced	ł		$\sim\sim$
25,01	24.03	DKI	Ī		-				<u> </u>	fracture subhorizontal to inclined 25	Į		~~~
03/02	14.85	4.10	ļ						AZCL	degrees planar smooth open to very open	ţ		$\sim\sim$
			_							slightly infilled grey silt on surfaces	H		
Domari		Щ				Щ				<2mm thick.			

Remarks

(See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR1 (3 of 4)

Equipment Knebel Water

Drill Fluid Drill Crew Dates Drilled

Dando 2000

Borehole Diameter 200mm to 5.60m

Logged by

200mm to 5.60m 140mm to 20.00m 140mm to 20.00m

Compiled by

Casing Diameter

Approved by

BOREHOLE No.

Coordinates (Local Grid) Ground Level 232110 E 709828 N 11.74 m OD

BH1

		End	03/02/2010			02/	02/201	.0 29/0	3/2010 12/04/2010			
	Casing	Water Depth (m)	Sample/Co	re Rec	, i	1	SPT Blows /N	Result or		Depth (Thick-	Level	Legen
& Time	Depth (m)	(Flush Return)	Depth (m) From To			RQD %	Core Size	or Fracture Index	Description of Strata	`ness)		
		%	From 10	70	70	70	(mm)			(m)		
										E		~~~
			<u> </u>							Ł		~~~
			_					3		ţ		$\sim\sim$
			<u>-</u> -							(9.15)		~~~
			_							<u> </u>		~~~
		(100)	14.85-16.35	98	94	29			At 15.66m: Fracture inclined 20 degrees	ţ		~~~
			-						stepped rough very open infilled light grey silt <3mm thick.	‡		$\sim\sim$
			ļ						Between 15.75m and 16.35m: Closely spaced fractures subhorizontal to inclined 85	‡		$\sim\sim$
									degrees.	ţ		$\sim\sim$
			-						Between 15.75m and 15.93m: Planar smooth moderately open locally infilled with	-		$\sim\sim$
			<u> </u>					8	light grey silt <2mm thick.	ţ		~~~
			_							ţ		~~~
										ţ		$\sim\sim$
			ţ					-		ţ		$\sim\sim$
			-							-		$\sim\sim$
			ļ						At 16.67m: Fracture subhorizontal	‡		$\sim\sim$
			-						slightly undulating smooth moderately open slight yellow staining penetrating	‡		~~
			ļ						<pre><2mm some quartzite veining. At 16.78m: Induced fracture horizontal</pre>	†		~~~
			ļ						planar smooth tight clean.	‡		$\sim\sim$
			-						At 16.87m: Fracture inclined 10 degrees undulating smooth open clean.	†		$\sim\sim$
		(100)	16.35-17.85	100	94	71				‡		$\sim\sim$
			<u>-</u>					2		‡		$\sim\sim$
										‡		~~~
			-							‡		~~~
			_ -						At 17.55m: Induced fracture inclined horizontal undulating striated tight to	F		$\sim\sim$
			- -						moderately open clean.	‡		$\sim\sim$
			-							‡		$\sim\sim$
			- -				-		Between 17.85m and 18.85m: Closely spaced fractures subhorizontal to inclined 10	‡		$\sim\sim$
			- -						degrees undulating smooth and planar	‡		~~~
			-						smooth moderately open to open locally with infilling <2mm.	F		~~~
			ļ							‡		~~~
			-							‡		$\sim\sim$
			-							‡		$\sim\sim$
			-							‡		$\sim\sim$
		l	F	l.				6		F		~~
		(100)	17.85-19.35	100	92	21				Ŧ		~~~
			Ė						Data 10 05 and 10 05 53 3	‡		\sim
			ļ						Between 18.85m and 19.35m: Closely spaced fractures horizontal and vertical stepped	‡		$\sim\sim$
			ļ						<pre>smooth open and very open infilled light grey silt <2mm.</pre>	Ŧ		~~~
			-						grey siic \zmm.	F		~~~
			ļ							‡		~~~
			Ė					>20	Detroop 10 25m 200 00m 2	‡		~~~
			-						Between 19.35m and 20.00m: Closely spaced fractures subhorizontal to inclined 30	‡		$\sim\sim$
			ļ						degrees planar rough moderately open and open locally infilled with light grey	‡		~~~
			-						silt <2mm.	F		~~~
	I	I	r e	1	I		1	1	l	t		000

Remarks

(See notes & keysheets)

03/02 20.00

Scale 1:25

(100)

DRY

19.35-20.00 100

82

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

14

Contract No.

End of Borehole

CON103001

-8.26

Figure No.

FR1 (4 of 4)

20.00

Equipment Dando 2000

Drill Fluid Drill Crew Dates Drilled Knebel Water/Polymer

25/01/2010 Start

Borehole Diameter 200mm to 12.00m 140mm to 35.00m Casing Diameter 200mm to 12.00m 140mm to 35.00m

BOREHOLE No.

232144 F 709839 N

BH₂

Coordinates (Local Grid) Ground Level 11.97 m OD

Logged by Compiled by Approved by

		End	10/02/2010			27/		29/0	1/2010 12/04/2010			
Date	Casing	Water Depth	Sample/Co	_	— –	,	SPT Blows /N	Result or U100		Depth		
& Time	Depth (m)	(ṁ) (Flush Return)	Depth (m)		SCR	RQD	Core Size	Blows/ Rec. mm or Fracture	Description of Strata	(Thick- ness)	Level	Legen
25/01		<u></u> % *	From To	%	%	%	(mm)	Index	TOPSOIL: Composed of dark brown organic sandy subrounded fine to medium gravel of mica schist and quartz with frequent roots. Sand is fine to coarse.	(m) - (0.50)		
			0.60-1.20	В	1				MADE GROUND: Composed of dark brown fine to coarse sand and angular to subangular fine to coarse gravel of mica schist with occasional quartz and rare angular to subangular cobbles <100mm of mica schist. Rare angular fine to medium coal fragments. Rare organic / leaf material noted.	0.50 (0.10) 0.60	11.47 11.37	
25/01	1.20	DRY	-						MADE GROUND: Composed of medium dense dark brown sandy angular to subangular	(1.10)		
26/01	1.20 1.90	DRY	1.20-1.70 1.20-1.65	ВС	2		C42		cobbles and boulders of mica schist and occasional quartz. Sand is fine to coarse.			
			2.00-2.45	C D B	3 4		C50/ 20		Probably MADE GROUND: Composed of dense dark brown fine to coarse SAND and angular to subangular fine to coarse gravel of mica schist with occasional subangular cobbles <120mm.	1.70	10.27	
	2.90	DRY	2.80 - 3.00 - 3.00-3.50	D C B	5		C75/ 20*			(2.30)		
			3.80	D	7							
26/01	3.90 4.30	DRY	4.00 - 4.00-4.03	С			C75/ 30*		Probably medium strong fractured dark grey mica SCHIST recovered as angular fragments.	4.00	7.97	
04/02	4.30	(0)	4.30	С	8		C0/ 0	AZCL	Strong horizontally and vertically foliated interbedded quartz mica SCHIST. Slightly weathered. Between 4.50m and 6.43m: Orange staining on fracture surfaces penetrating <5mm.	4.30	7.67	
Remar	_	(70)	4.30-5.55 —	98	48	0						

Remarks 1 (See notes & keysheets)

Scale 1:25

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.

Rotary coring was carried out using Geobore drilling techniques.

The borehole was advanced by cable percussion means to 4.30m and then progressed by rotary coring means to 3 35.00m.

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 1.80m to 2.00m (60 mins); from 2.30m to 2.70m (45 mins); from 2.80m to 3.00m (30 mins); from 3.00m to 3.30m (15 mins); from 3.50m to 3.70m (30 mins); from 4.10m to 4.30m (45 mins).



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR2 (1 of 8)

Equipment Dando 2000 Knebel

End

Drill Fluid Drill Crew Dates Drilled

10/02/2010

Borehole Diameter 200mm to 12.00m 140mm to 35.00m **Casing Diameter** 200mm to 12.00m 140mm to 35.00m

BOREHOLE No. Coordinates (Local Grid) Ground Level

BH₂ 232144 E 709839 N 11.97 m OD

Logged by 27/01/2010 29/01/2010

Compiled by	Approved by
29/01/2010	12/04/2010
29/01/2010	12/04/2010

-4.	Casing	Water Depth	Sample/Core Recovery				SPT	Result	ult	Depth			
ate &	Casing Depth	(m)	Depth	(m)	Туре	No.		SPT Blows /N		Description of Strata	(Thick-	Level	Lege
me	(m)	(Flush Return) %	From	To	TCR %	SCR %	RQD	Core Size (mm)	Fracture Index		ness) (m)		
		70	-		<u> </u>	<i>-</i> -		()			1 (,		~~
											ţ		~~
			L						>20		Ł		\sim
			Ī						/20		F		~~
											‡		\sim
			<u>L</u>								L		\sim
		-						i			ţ		~~
			_								t		\sim
		(60)	5.55-	-6.00	100	53	22				F		\sim
			-								Ī		~
			-								ţ		\sim
								1			F		~
			[F		~
			Ī						NI		Ī		\sim
		(60)	6.00-	-6.60	100	63	17				‡		~
		(11)									†		\sim
			L								L		\sim
										Between 6.60m and 7.50m: Very closely	ł		~
			_							spaced horizontal and undulating smooth subvertical open with quartz bands and	Ī		\sim
										mica crystals <3mm clean.	Į.		\sim
			-								†		~
									9		ł		\sim
		(100)			100						F		\sim
		(100)	6.60-	-7.50	100	78	0				Ī		\sim
			-								‡		\sim
			-								†		\sim
											(6.20)		\sim
			Ī.,							Between 7.50m and 10.50m: Fractures are	F		\sim
									AZCL	closely spaced horizontal and subvertical	F		~
			-							undulating stepped rough open stained yellow on surfaces with fine to medium	Ī		\sim
			-							sand infilling locally <3mm.	‡		\sim
			-								†		~
			_								t		\sim
			_								F		\sim
									6		Ŧ		~
		(100)	·	0 00	97		٠,				Į.		\sim
		(100)	/.50-	-9.00	97	79	34				ţ		\sim
											†		\sim
			_								L		~
											ł		\sim
			<u> </u>								ł		\sim
			Ē								F		~
			-						15		Ţ		\sim
			_								t		\sim
			_					1			<u> </u>		\sim
			<u> </u>								ł		\sim
											F		\sim
			Ī						5		Ţ		\sim
			<u> </u>								ţ		\sim
			L								L		\sim
			ŀ								ł		\sim
											F		\sim
		(100)	9.00-	-10.50	100	92	23				ļ		~
			<u> </u>								ţ		\sim
			_								t		\sim
			_		I			I					

(See notes 7 & keysheets)

See installation details on final sheet. Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR2 (2 of 8)

Equipment Dando 2000 Knebel

Drill Fluid **Drill Crew Dates Drilled** Water/Polymer

Borehole Diameter 200mm to 12.00m 140mm to 35.00m **Casing Diameter** 200mm to 12.00m 140mm to 35.00m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232144 **E** 709839 N 11.97 m OD

BH2

ged by	Compiled by	Approved by
/01/2010	29/01/2010	12/04/2010

Drill Crew Dates Drilled Start 25/01/2010 End 10/02/2010							ged by	Com .0 29/0	piled by Approved by 1/2010 12/04/2010			
Date &	Casing Depth	Water	Sample/Co	Г	No.		SPT Blows /N		Description of Strata	Depth (Thick-	Level	Legend
Time		(Flush Return) %	Depth (m) From To	TCR	SCR %	RQD %	Core Size (mm)	Fracture Index	Description of Strata	ness)		
		76		,,,	70	, , , , , , , , , , , , , , , , , , ,	()			()		~~~
			<u>.</u> -							‡		~~~ ~~~
								8		-		~~~
			_						Very strong fractured highly foliated	_10.50	1.47	~~~
			<u>.</u>						dark and light grey mica SCHIST interbedded with quartz. Fresh. Fractures widely spaced horizontal and	ŧ		~~~
			<u>.</u> -						subhorizontal planar rough moderately open clean.	‡		~~~
			[-						Between 10.50m and 10.82m: Closely spaced fractures inclined 10 degrees planar	-		~~~
			[-						stepped smooth open clean. Between 10.95m and 11.22m: Closely spaced fractures inclined undulating rough open	-		~~~
		(100)	10.50-12.00	100	93	57			clean.	‡		~~~
			<u> </u>							‡		
			- -					4		<u> </u>		~~~
			<u>-</u>						At 11.72m: Fracture horizontal stepped to undulating smooth to rough open clean.	į		~~~
			- -						At 11.78m: Induced fracture stepped smooth moderately open clean.	ŧ		~~~
04/02	12.00	DRY	<u>-</u>							-		~~~
05/02	12.00		[Between 12.19m and 12.26m: Very closely	-		~~~
			<u>-</u> -					0	spaced fractures inclined 10 to 45 degrees undulating smooth open clean.	‡		~~~
			<u>-</u>						Between 12.34m and 12.85m: Closely spaced fractures inclined 70 degrees and 30	‡		~~~
								6	degrees planar undulating rough open clean.	<u> </u>		~~~
		(90)	12.00-13.35	100	91	38		6		ŧ		~~~
			<u>.</u> -							‡		~~~
			[_					>20		Į.		~~~
			<u>-</u>							‡		~~~
			<u>-</u> -							‡		~~~
										ŧ		~~~
			 -					0		+		~~~
			<u>-</u> -							ŧ		~~~
			- -							Ī		~~~
			<u>-</u>						Between 14.00m and 14.35m: Closely spaced	Ł		~~~
			- -						fractures inclined 45 degrees stepped smooth moderately open clean.	+		~~~
		(70)	13.35-15.00	100	98	39				Į.		~~~
			-							‡		
			<u> </u>					5	Between 14.50m and 15.00m: Closely spaced subhorizontal fractures undulating rough	<u> </u>		~~~
			<u>-</u>						open clean.	‡		
			F							F		~~~
			<u> </u>							‡		
	<u> </u>						Щ.			<u> </u>		
Remar (See note												

(See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR2 (3 of 8)

25/01/2010

Equipment Dando 2000 Knebel

Drill Fluid Water/Polymer **Drill Crew** Start End **Dates Drilled**

Borehole Diameter 200mm to 12.00m 140mm to 35.00m **Casing Diameter** 200mm to 12.00m 140mm to 35.00m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232144 E 709839 N 11.97 m OD

BH2

Logged by Compiled by 27/01/2010

Approved by 12/04/2010

Dates	Drilled	Start End	25/01/2010 10/02/2010			27/	01/201	0 29/0	1/2010 12/04/2010			
	<u> </u>	Water	Sample/Co	re Rec	covery		SPT			Depth		
&	Casing Depth	(111)	Depth (m)	Туре	No.		Blows /N	Result or Fracture	Description of Strata	(Thick- ness)	Level	Legend
Time		(Flush Return) %	From To	TCR %	SCR %	RQD %	Core Size (mm)	Index	·	(m)		
		,,	-							(,		~~~
										Ŧ		~~~
			<u>-</u> -						Between 15.29m and 15.32m: Open fracture zone.	†		~~~
			-					NI	2011.	ţ		~~~
			<u>-</u>						Between 15.55m and 16.28m: Closely spaced	<u>t</u>		~~~
			-					2	horizontal undulating rough open locally sand infiled <2mm.	‡		~~~
		(100)	- 15.00-16.50	100	85	55				Ŧ		~~~
		(100)	_ 13.00-16.50 [100	05	33				Ŧ		~~~
			[Ŧ		~~~
			<u> </u>					16		F		~~~
			-					10		ł		~~~
			<u>-</u> -							Ł		~~~
										ţ		$\sim\sim$
05/02	16.50	DRY	‡							‡		~~~
08/02	16.50		-							(12.00)		~~~
			-					0		‡		~~~
			-							‡		~~~
			-							Ŧ		~~~
			- -							Ŧ		~~~
			F					>20		F		~~~
			[At 17.15m: Fracture inclined 15 degrees stepped undulating rough open clean	Ŧ		~~~
		(100)	16.50-18.00	100	97	79			At 17.15m: Fracture inclined 15 degrees stepped undulating rough open clean.	Ŧ		~~~
			<u> </u>						stepped undurating rough open cream.	ł		$\sim\sim$
			<u>-</u>							Ł		~~~
			‡							‡		~~~
			-					2		‡		~~~
			- -							ţ		~~~
									Between 17.90m and 17.93m: Horizontal fracture planar rough open clean.	‡		~~~
			<u>-</u>							‡		~~~
			[Ŧ		~~~
			<u> </u>							Ł		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			<u> </u>					-	Between 18.34m and 18.41m: Subvertical to inclined open clean.	ţ		~~~
			<u>-</u>					NI	Inclined Open Clean.	‡		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			-					[`		ļ		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			ļ							‡		~~~
		(100)	18.00-19.50	100	95	59				‡		~~~
			Ī							Ŧ		~~~
			L					1		Ł		~~~
			E							F		~~~
			-							£		~~~
			<u> </u>							t		~~~
			<u> </u>							ţ		~~~
			<u>-</u>							<u></u>		~~~
			<u>-</u> -					AZCL	At 19.60m: Induced fracture inclined 10 degrees stepped smooth moderately open	‡		
			<u> </u>						clean.	‡		
			 - -						At 19.80m: Induced fracture inclined 10 degrees undulating smooth moderately open	‡		~~~
			ļ.						clean.	‡		~~~
			<u>-</u>							†		~~~

Remarks

(See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR2 (4 of 8)

Equipment Dando 2000 Knebel

Drill Fluid Water/Polymer **Drill Crew**

Borehole Diameter 200mm to 12.00m 140mm to 35.00m **Casing Diameter** 200mm to 12.00m 140mm to 35.00m **BOREHOLE No.**

Coordinates (Local Grid) Ground Level 232144 E 709839 **N** 11.97 m OD

BH2

Drill C	rew Drilled	Start	25/01/20	010			Log	ged by	Com	piled by Approved by				
Dates	Dillea	End	10/02/20				27/	01/201	.0 29/0	1/2010 12/04/2010				
Date	Casing	Water Depth	Sam	ple/Co	_	. <u> </u>	· · ·	SPT Blows /N	Result			Depth		
&	Depth	(ṁ) _(Flush	Depth ((m)	Туре		-		or Fracture	Description of	of Strata	(Thick- ness)	Level	Legen
Time	(m)	Return)	From	То	TCR %	SCR %	RQD %	Core Size (mm)	Index			(m)		
												1		~~~
			_									Ŧ		~~~
		(100)	- 19.50-2	21.00	98	98	89		0	At 20.36m: Induced frac	ture inclined 15	ļ.		~~~
										degrees stepped smooth clean.	moderately open	Ē		~~~
			-									F		~~~
			-									ļ.		~~~
			-									F		~~~
												f		~~~
			<u>-</u>									L		~~~
			-						AZCL	At 21.15m: Induced frac	ture inclined 15	ţ		~~~
			_							degrees planar smooth t	ight clean.	Ł		~~~
		(100)	21.00-2	21.70	93	93	93					Ŧ		~~~
			-						0	Between 21.47m and 21.7	Om: Strong green	ļ.		~~~
			-						U	fine crystalline possibintrusive.	oly igneous	F		~~~
												Ŧ.		~~~
			-							At 21.86m: Fracture inc	lined 30 degrees	†		~~~
		(100)	21.70-2	22.10	100	75	33			undulating smooth clear Between 21.95m and 22.0		ŧ		~~~
			_						NI	subvertical to inclined		F		~~~
			-									ţ		~~~
			-							At 22.25m: Induced frac		<u> </u>		~~~
		(100)	22.10-2	22.50	100	100	85			degrees clean smooth plopen.	anar moderately	£		~~~
												22.50	-10.53	~~
			-							Strong fractured foliat SCHIST. Moderately weat		ļ.		~~~
			-							light brown yellow.		Ŧ		~~~
			-									F		~~~
			-									ţ		~~~
			-						2			F		~~~
			_									Ē		~~~
		(100)	_ 22.50-2	24.00	100	97	93					ţ.		~~~
			-									ţ		~~~
			Ė									F		~~~
			_									Ŧ		~~~
			- -									ļ.		~~~
			Ē									Ŧ		~~~
	24.00	DRY										£		~~~
09/02 09/02	24.00 24.00	3.90	ļ						AZCL			‡		
	24.00		È									ŧ		
,			[F		~~~
			<u>.</u>							At 24.49m: Induced frac		‡		~~~
			⊢ •							planar rough moderately	open clean.	F		~~~
			Ē									Ī		~~~
		(100)	24.00-2	25.50	91	88	81					f		~~~
			ļ.									†		~~~
			<u>-</u>									‡		~~~
Remar	ks						Щ					_	<u> </u>	

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR2 (5 of 8)

Equipment Dando 2000 Knebel

Drill Fluid Water/Polymer
Drill Crew
Dates Drilled Start 25/01/2010
End 10/02/2010

Borehole Diameter 200mm to 12.00m 140mm to 35.00m Casing Diameter 200mm to 12.00m 140mm to 35.00m BOREHOLE No.
Coordinates
(Local Grid)
Ground Level

232144 E 709839 N 11.97 m OD

BH2

Logged by Compiled by Approved by

	End 10/02/2010							.0 29/0	1/2010 12/04/2010			
Doto	Casina	Water Depth	Sample/Co	re Rec	overy		SPT Blows	Result		Depth		
	Casing	(m)	Double (m)	Туре	No.		/N	or	Description of Strata	(Thick-	Level	Legend
& Time	Depth (m)	(Flush Return)	Depth (m)		SCR	POD	Core Size (mm)	Fracture	Description of Strata	`ness)		
Time	(111)	Return) %	From To	%	%	%	(mm)	Index		(m)		
			-									~~~
			-							<u> </u>		~~~
			-							-		$\sim\sim$
			F							F		~~~
			<u> </u>							<u> </u>		$\sim\sim$
										(6.00)		$\sim\sim$
										,		~~~
			F							Į		$\sim\sim$
			- -							<u> </u>		~~~
			-									~~~~
			<u> </u>							<u> </u>		$\sim\sim$
			_							_		~~~
			F							ļ l		$\sim\sim$
										ļ		$\sim\sim$
		(100)	25.50-27.00	100	100	100		1		-		~~~
			ŧ					-		<u>t</u>		~~~
			ļ -							}		~~~
			F						At 26.60m: Induced fracture inclined 15	F		~~~~
			ļ.						degrees undulating rough tight clean.	 		~~~
			_							Ŀ		~~~
			-							-		$\sim\sim$
			F							ļ l		$\sim\sim$
			-						Between 27.06m and 27.10m: Induced			~~~
			-						fracture inclined horizontal (80%) and 85 degrees (20%) stepped smooth very open			$\sim\sim$
			ŀ						clean.	<u> </u>		$\sim\sim$
			-							-		~~~~
			F							Ţ		$\sim\sim$
			<u>-</u>						At 27.48m: Induced fracture inclined 20	<u> </u>		~~~
			<u> </u>						degrees undulating smooth tight clean surfaces.	_		$\sim\sim$
									barraces.	ŀ		~~~
		(100)	27.00-28.50	100	100	100				[~~~
		(100)	- 27.00-20.30	100	100	100				[$\sim\sim$
			<u> -</u>							<u> </u>		~~~
			<u>-</u>							<u>L</u>		~~~
			-							-		$\sim\sim$
			F							Į į		~~~
			ļ.							ļ		$\sim\sim$
			ţ							ţ l		~~~
			ŧ							<u>t</u>		~~~~
								AZCL	Strong fractured foliated dark and light	_28.50	-16.53	$\approx \approx$
			F					AZCL	grey quartz mica SCHIST. Slightly	Į į		~~~
			ļ.						weathered to fresh. Fractures are	ţ l		~~~~
			ļ ,					8	subhorizontal to subvertical closely to medium spaced planar rough and undulating	 		~~~
			t						rough locally cemented with quartz.	<u>t</u>		~~~
			ŀ						Between 28.68m and 28.75m: Fractures inclined 15 degrees planar smooth	}		~~~~
			F						moderately open to open slight yellowish	F		$\sim\sim$
			ļ.						staining on surfaces. At 29.02m: Fracture subhorizontal	ţ l		~~~
		(100)	28.50-30.00	95	92	86			undulating smooth moderately open	<u>t</u>		$\sim\sim$
			Ł						slightlyellow staining on surfaces.	ł l		~~~
										Į į		~~~~~
			Ļ					_		Ļ ļ		~~~
			ţ					0		ţ l		~~~
			ŧ							<u>t</u>		$\sim\sim$
			 -							 		~~~
			ļ.							ļ l		~~~~
			ţ							t		$\sim\sim$
			-							 		
Remar				—	—	—				——		

Remarks (See notes & keysheets)

Scale 1:25

TUGRO

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR2 (6 of 8)

Equipment Dando 2000 Knebel Drill Fluid Water/Polyn

Drill Fluid
Drill Crew
Dates Drilled

Start 25/01/2010
End 10/02/2010

Borehole Diameter 200mm to 12.00m 140mm to 35.00m Casing Diameter 200mm to 12.00m 140mm to 35.00m BOREHOLE No.
Coordinates
(Local Grid)
Ground Level

232144 E 709839 N 11.97 m OD

BH2

Logged by Compiled by Approved by 27/01/2010 29/01/2010 12/04/2010

		End	10/02/20				27/	01/201		1/2010 12/04/2010			
Doto	Cocina	Water Depth	Samp	ole/Co	re Rec	overy		SPT Blows	Result		Depth		
&	Casing Depth	(m) (Flush	Depth (ı	m)	Туре			/N	or Fracture	Description of Strata	(Thick- ness)	Level	Legend
Time	(m)	Return)	From	То	TCR %	SCR %	RQD %	Core Size (mm)	Index		(m)		
		-/0			<u> </u>			()		At 30.08m: Fracture subhorizontal	(,		~~~
			-							undulating smooth open clean.	ł		~~~
			F								Ŧ		~~~
			-							At 30.28m: Induced fracture inclined 45 degrees planar smooth moderately open	‡		~~~
			_							clean.	t		$\sim\sim$
			_							At 30.40m: Induced fracture 35 degrees planar smooth open clean.	Ł		~~~
			-							planar smooth open tream.	F		~~~
			-							Between 30.72m and 31.50m: Closely spaced	Į.		$\sim\sim$
		(100)	30.00-3	1.50	100	96	62		5	horizontal and subhorizontal planar rough	‡		~~~
			-							locally infilled with fine to medium sand <3mm thick.	‡		~~~
			-							Shim Chick.	t		$\sim\sim$
			F								┝		~~~
			-								Ŧ		$\sim\sim$
			-							Between 31.23m and 31.39m: Quartz band.	‡		~~~
			<u>-</u>								ţ		~~~
			<u>t</u>								t		
			-							Between 31.50m and 32.50m: Quartzite	ł		~~~
			-						NI	bands ranging from 40mm to 180mm thick.	F		~~~
			- -							Between 31.50m and 31.70m: Non intact.	‡		$\sim\sim$
			_								(6.50)		~~~
			-								+ ```		$\sim\sim$
			F							At 31.90m: Induced fracture inclined 40 degrees stepped smooth tight clean.	Ŧ		$\sim\sim$
			<u>.</u>							degrees stepped smooth ergne eredn.	ļ.		
			-								‡		$\sim\sim$
			-			0.3	86				t		~~~
		(100)	_ 31.50-3	33.00	100	93			1		ł		~~~
			F						1		Ŧ		$\sim\sim$
			-								‡		~~~
			-								<u> </u>		~~~
			-								t		$\sim\sim$
			-								ł		~~~
			Ē								F		~~~
			-								‡		$\sim\sim$
										Between 33.00m and 35.00m: Closely spaced	L		~~~
			-						AZCL	fractures horizontal and subhorizontal undulating stepped smooth open to very	t		$\sim\sim$
			-							open with quartzite veins <10mm thick on	ŧ		~~~
			F							surfaces and slight yellow brown	F		
			F							staining.	Ŧ		~~~
			ļ.								‡		~~~
			<u> </u>								<u>F</u>		~~~
			E								t		$\sim\sim$
			F						3		Ŧ		~~~
		(100)	33.00-3	4.60	99	97	61				Ŧ		$\sim\sim$
			<u> </u>								‡		~~~
			Ŀ								Ł		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
			}								ł		$\sim\sim$
			F								Ŧ		~~~
			<u> </u>								‡		\~~~~
			t								t		~~~
			F								ł		<u> </u>
			F						ARCT.		F		$\sim\sim$
			<u> </u>					Į.	AZCL		‡		~~~
			<u>t</u>								‡		~~~
		(100)	_ - 34.60-3	5.00	90	90	68		2		t		~~~
			- 500-3		50	00	30				Ŧ		
10/02	35.00	3.60	Ī								35.00	23 03	$\sim\sim$
			T					1		End of Borehole		23.03	

Remarks

(See notes & keysheets)

Scale 1:25

TUGRO

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No.

CON103001

Figure No.

FR2 (7 of 8)

Drilling Method Cable Percussion & Rotary **BOREHOLE No. Borehole Diameter** BH2 200mm to 12.00m 140mm to 35.00m Coordinates (Local Grid) Ground Level Equipment Dando 2000 232144 E Knebel 709839 **N** 11.97 m OD Water/Polymer **Drill Crew** Approved by Logged by Compiled by **Dates Drilled** 25/01/2010 Start 27/01/2010 29/01/2010 12/04/2010 End 10/02/2010 Installation Water Strata Level **Installation Details** Strata Details Strikes m OD Depth (m) Depth (m) Bentonite Grout TOPSOIL 0.50 Instrumentation: MADE GROUND 19mm standpipe piezometer tip at 10.00m 4.00 Coarse/Medium grained Metamorphic (SCHIST) 5.00 6.97 Bentonite Seal 6.00 5.97 Gravel Filter 10.00 1.97 Bentonite Seal 0.97 11.00 Bentonite Grout 35.00 -23.03 35.00 Base of Hole Remarks (See notes & keysheets) Water Strike ▼ Water Rise Upstanding cover. Pipe diameter 19mm to 10.00m. Project Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR2 (8 of 8)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No. BH3** 200mm to 6.60m 200mm to 6.60m Coordinates (Local Grid) Ground Level Equipment 131mm to 35.00m 150mm to 6.60m Dando 2000 232162 E Massenza MI6 709840 N 12.04 m OD Compiled by **Drill Crew** Approved by Logged by

Dates	Drilled	Start End	21/01/20 10/02/20				25/01/		29/01/2010 12/04/2010			
&	Casing Depth	Depth to Water	Sam Depth (etails	ı	SPT Blows/N Drive mm	U100 Blows/ Recovery mm	Description of Strata	Depth (Thick- ness)	Level	Legend
Time	(m)	(m)		To	Туре	No.	Test	Result		(m)		
21/01			-						Grass over sandy gravelly TOPSOIL.	(0.10) 0.10	11.94	
			<u>-</u>						MADE GROUND: Tarmacadam	(0.10)		
			= = =						MADE GROUND: Type 1 fill.	(0.15) 0.35		
			- - 0.50-0 -	0.80	В	1			MADE GROUND: Composed of sandy gravelly subangular cobbles and boulders of schist. Gravel is subangular fine to coarse mica schist. Sand is medium to coarse.	(0.55)		
			0.80		D	2				F		
			-							0.90	11.14	
			_ 0.95		ES	3			MADE GROUND: Composed of dark brown clayey sandy angular to subrounded fine to coarse	-(0.15) 1.05	10.99	
									gravel of mica schist with many subangular cobbles. Hydrocarbon / diesel odour. Sand is	E		
		DRY	1.20-1 1.20-1		C B	4	C31		fine to coarse.	Ė , , , , , , ,		
/			- -						MADE GROUND: Composed of stiff grey brown gravelly clay. Gravel is subangular fine to coarse schist.	(0.65) - -		
21/01	1.70	DRY	-							1.70	10.34	
22/01	1.70		- -						MADE GROUND: Composed of very dense brown silty gravelly medium to coarse sand with occasional angular cobbles. Gravel is	(0.30)		
			1.90-2	2.10			C40/ 105		occasional angular comples. Gravel is angular to subangular fine to coarse mica schist.	2.00	10.04	
	1.90	DRY	2.00		C D	5	105		Possibly MADE GROUND: Composed of grey sandy	2.00	10.04	
			2.00-2	2.50	В	6			gravelly angular to subangular cobbles of mica schist with many boulders. Gravel is angular to subangular fine to coarse mica schist. Sand is medium to coarse.	- - - -		
			_							<u>-</u> -		
			-							<u>-</u>		
			-							[
	2.90	DRY	3.00		С		C50/ 20			<u> </u>		
			3.00-3	3.50	В	7	20			-		
			- -							-		
			- -							-		
			-							F		
			-							ļ.		
			3.70		D	8				Ē.		
			-							Ė		
	3.90	DRY	4.00		С		C50/ 70			F		
			4.00-4	4.50	В	9				 		
			F -							(4.50)		
			-							!		
			<u> </u>							F		
			- -							 		
			4.70		D	10				Ę.		
			-							 		
	4.80	DRY	5.00		С		C50/			F		

Remarks 1 (See notes & keysheets) 2

Scale 1:25

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located. Rotary coring was carried out using a core barrel 131mm in diameter.

The borehole was advanced by cable percussion means to 6.60m and then progressed by rotary coring means to 3 35.00m.

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 2.20m to 2.60m (45 mins); from 2.70m to 3.00m (30 mins); from 3.00m to 3.40m (45 mins); from 3.80m to 4.00m (30 mins); from 4.20m to 4.60m (45 mins); from 5.20m to 5.60m (45 mins);



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR3 (1 of 8)

Equipment

Dando 2000

Drill Fluid Drill Crew Dates Drilled Massenza MI6 Water

Start 21/01/2010 End 10/02/2010 Water

Borehole Diameter 200mm to 6.60m 131mm to 35.00m

25/01/2010 29/01/2010

Compiled by

Logged by

Casing Diameter 200mm to 6.60m 150mm to 6.60m

BOREHOLE No.

Coordinates (Local Grid) Ground Level 232162 E 709840 **N** 12.04 m OD

BH3

Approved by 12/04/2010

	End 10/02/2010					25/		.0 29/0	1/2010 12/04/2010			
D-1	C = -!	Water Depth	Sample/Co	re Rec	overy		SPT Blows /N	Result or U100		Depth		
Date	Casing	(m)		Туре	No.		/N	Blows/	Description of Otroto	(Thick-	Level	Legend
_&	Depth	(Flush	Depth (m)					Rec. mm or	Description of Strata	ness)		
Time	(m)	Return)	From To	TCR %	SCR %	RQD %	Core Size (mm)	Fracture		(m)		
		76	110111 10	/0	70	/0		Index		(111)		XXXXX
			5.00-5.50	В	11		10					
			5.00-5.50	Р.	111							
			Ļ							-		
			_									
			<u> </u>							_		
			į									
			_									
			5.70	D	12					Ĺ		
			t									
	5.90	DRY	_ 6.00	C			C50/			-		
			6.00-6.50	В	13		125					
			0.00-0.50	"	13							
			ŀ							<u> </u>		
			ļ.									
22/01	5.90	DRY	ţ							<u> </u>		
	F 22		L						Barailla maliam abara di garaga	_ 6.50	5.54	
25/01 25/01	5.90 6.60	4.00	F		-				Possibly medium strong mica SCHIST recovered as angular fine to coarse	(0.10) 6.60	5.44	$\Rightarrow\Rightarrow\Rightarrow$
		\	į					AZCL	gravel and cobble sized fragments.		5111	~~~
05/02	6.60	(0)	- 6.60	С	14		C50/ 10		Variations from the land thin land to the land	_		$\sim\sim$
			_				10		Very strong fractured thinly foliated light and dark grey mica SCHIST with			~~~
									bands of quartz.			$\sim\sim$
			-							_		~~~
			_									$\sim\sim$
												~~~
			_							-		~~~
			_									$\sim\sim$
								3				~~~~
			-							-		$\sim\sim$
		(100)	6.60-8.60	93	92	68						~~~
												$\sim\sim$
			-									~~~
			_									$\sim\sim$
			[									$\sim\sim$
			-							-		$\sim\sim$
			<u> </u>							<u> </u>		~~~
			-									~~~~
			<del>-</del> -							Ī		~~~
1	I		<u> </u>							<u> </u>		~~~~
			ŀ							-		~~~
			F							Γ .		~~~~
			-		1							$\sim\sim$
I	I		<u>L</u>						At 8.78m: Induced fracture inclined 25	Ŀ		~~~`
			-						degrees planar smooth tight clean.			$\sim\sim$
			-									~~~
			<u> </u>						At 9.00m: Induced fracture inclined 25	<u> </u>		$\sim\sim$
			-						degrees planar smooth tight clean			
I	I		Ī						At 9.10m: Induced fracture inclined 25 degrees and 75 degrees planar smooth	ļ		$\sim\sim$
			<u> </u>						stepped rough tight clean.	<u>L</u>		~~~
			<del> </del>							-		~~~~
			-						At 9.47m: Induced fracture inclined 15			~~~
			<u> </u>						degrees undulating striated tight clean.	Ŀ		~~~~
			-									~~~
			F									~~~~
			<u>L</u>							<u> </u>		~~~
I	I		<del> </del>									~~~~
			ļ.							-		$\sim\sim$
		(100)	8.60-11.40	100	100	85				L		~~~
						Щ.						
Remai	rke	from E			mina	\. f~	om 6 E		60m (45 ming)			-

Remarks (See notes & keysheets)6

from 5.80m to 6.00m (30 mins); from 6.50m to 6.60m (45 mins).

See installation details on final sheet.

Groundwater was not apparent during boring.

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (2 of 8)

Equipment

Drill Fluid **Drill Crew Dates Drilled**  Dando 2000 Massenza MI6

Start End 21/01/2010 10/02/2010 **Borehole Diameter** 200mm to 6.60m 131mm to 35.00m **Casing Diameter** 200mm to 6.60m 150mm to 6.60m

**BOREHOLE No.** Coordinates (Local Grid) Ground Level

232162 **E** 709840 **N** 12.04 m OD

BH3

Logged by Compiled by Approved by 25/01/2010 29/01/2010 12/04/2010

		End	10/02/2010			25/	01/201		1/2010 12/04/2010			
Date	Casing	Water Depth	Sample/Co	_	. <u> </u>		SPT	Result		Depth		
&	Depth	(m)	Depth (m)	Type	No.	1	/N	or	Description of Strata	(Thick-	Level	Legend
Time	(m)	(Flush Return)			SCR		Core Size	Fracture Index	2000.10.0.0.0.0.0.0	ness)		
		% ′	From To	%	%	%	(mm)			(m)		,,,,,
			-					1	At 10.17m: Induced fracture inclined 60	‡		~~~
			-						degrees slightly undulating slightly	‡		~~~~
			<del>-</del> -						rough moderately open clean.	<u>t</u>		$\sim\sim$
			_							ł I		$\sim\sim$
										Ŧ I		~~~
			-							F		$\sim\sim$
										Į		$\sim\sim$
			-						At 10.79m: Induced fracture inclined 10	‡		$\sim\sim$
			-						degrees undulating striated moderately open clean.	‡		~~~
			-						opon oloun	‡		$\sim\sim$
			<del>_</del> -							(8.80)		$\sim\sim$
			-							‡		~~~
			- -						At 11.28m: Quartz band (30mm).	<u>t</u>		~~~
										ŧ l		$\sim\sim$
					1				At 11.40m: Fracture inclined 35 degrees undulating rough open infilled with	ŧ l		$\sim\sim$
			<u> </u>						clayey sand <5mm.	F		~~~
									At 11.53m: Induced fracture inclined 40 degrees planar smooth moderately open	Ŧ I		~~~
		(100)	11.40-12.00	100	100	33			with quartzite veins penetrating <5mm.	Ŧ		$\sim\sim$
			<del>-</del> -							Ī		$\sim\sim$
05/02	6.60	1.00								‡		$\sim\sim$
		1.00	<u>-</u>					-		<b>Ļ</b> │		~~~~
09/02	6.60		-					AZCL	At 12.17m: Fracture inclined 45 degrees	‡		$\sim\sim$
			-						slightly stepped rough open infilled with	‡		$\sim\sim$
			-						clayey sand <3mm.	‡		~~~
			<u>-</u>							<u> </u>		
			- -							Ł		$\sim\sim$
			_							ŧ l		$\sim\sim$
			_							ŧ l		~~~
										F		~~~~
										Ŧ I		$\sim\sim$
			-							Į l		$\sim\sim$
			-						At 13.08m: Induced fracture inclined 30	F		$\sim\sim$
									degrees undulating rough moderately open clean.	Į l		~~~
		(80)	12.00-14.50	97	96	74			020411	‡		$\sim\sim$
			-							‡		$\sim\sim$
			-							‡		$\sim\sim$
			<del>-</del> -							<b>F</b>		~~~
			-							‡		$\sim\sim\sim$
			- -					1	At 13.75m: Induced fracture inclined 45	‡		$\sim\sim$
									degrees undulating slightly rough tight to moderately open clean.	<b>‡</b>		$\sim\sim$
			- -						At 13.92m: Induced fracture inclined 30	<u>†</u>		~~~
			<u> </u>						degrees stepped smooth moderately open clean.	<u> </u>		~~~
									0-34	ŧ l		$\sim\sim$
			<u>-</u>							<u>t</u>		$\sim\sim$
										ŧ I		~~~
			-							<b>F</b>		~~~~~
		-	<del>-</del>		1					<b>-</b> ∣		$\sim\sim$
										<b>[</b>		$\sim\sim$
			-							Į		~~~
			<del>-</del> -							‡		~~~~~
										‡		$\sim\sim$
			<u>-</u>							<b>Ļ</b> │		~~~
Remar				<u> </u>								<u> </u>

Remarks

(See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (3 of 8)

Equipment

Dando 2000 Massenza MI6 Water

**Drill Crew Dates Drilled** 

Drill Fluid

21/01/2010 Start End 10/02/2010 **Borehole Diameter** 

200mm to 6.60m 131mm to 35.00m

**Casing Diameter** 200mm to 6.60m 150mm to 6.60m **BOREHOLE No.** 

Coordinates (Local Grid) Ground Level

BH3

232162 **E** 709840 N 12.04 m OD

Logged by	Compiled by	Approved by
25/01/2010	29/01/2010	12/04/2010

	O!	Water Depth	Sai	mple/Co	re Rec	overy		SPT	Result		Depth		
	Casing Depth	(m)	Depth	(m)	Туре	No.		Blows /N	or	Description of Strata	(Thick-	Level	Lege
me	(m)	(Flush Return)	•	` '		SCR	RQD	Core Size	Fracture Index	Description of Strata	ness)		
		% ′	From	То	%	%	%	(mm)			(m)		-
			<u>-</u> -								ł		$\sim$
			- -								ŧ		$\sim$
			<u>-</u> -								ł		$\sim$
			- -								15.40	-3.36	$\sim$
			_							Strong fractured thinly foliated dark and light grey quartz mica SCHIST with	<b>+</b>		$\sim$
			-							frequent 20mm to 50mm bands of quartzite.	<b>†</b>		~
			<u>.</u>							Slightly weathered to fresh. Fractures are subhorizontal to subvertical closely	ţ		$\sim$
			-							to medium spaced planar smooth and undulating smooth locally cemented with	<b>†</b>		$\sim$
			-							quartz and rarely infilled with clayey	<b>†</b>		$\sim$
		(80)	14.50	-17.50	100	98	35			gravel. At 16.00m: Closely spaced fractures	<b>+</b>		$\sim$
			-							inclined 15 degrees to 70 degrees planar	<b>†</b>		$\sim$
			- -							<pre>smooth and stepped very open some light grey silt infill &lt;2mm.</pre>	ŧ		~
			- -							52	ŧ		$\sim$
			<u>-</u>								ł		$\sim$
			_								H		$\sim$
			- -								ł		$\sim$
			-						10	At 16.70m: Induced fracture inclined 15 degrees undulating rough tight clean.	Ł		$\sim$
			-							405100 <i>b</i> 411441401115 104511 015110 0154111	ł		$\sim$
			-								Ŧ		$\sim$
			_								F		$\sim$
			- -								Ŧ		$\sim$
			-							At 17.25m: Fracture inclined 45 degrees	E		~
			-							planar smooth very open infilled <100mm	Ŧ		$\sim$
			-							with gravel.	Ŧ		$\sim$
			_								F		$\sim$
			-								Ŧ		$\sim$
			- -							At 17.78m: Fracture inclined 15 degrees	E		$\sim$
			 - -							slightly stepped smooth open clean.	Ŧ		$\sim$
			-								Ŧ		$\sim$
			_								F		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
			_								F		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
			-								Ŧ		$\sim$
		(80)	17.50	-20.50	100	96	65				F		$\sim$
			_								£		$\sim$
			- -						12		E		$\sim$
			-								F		$\sim$
			-								Ŧ		$\sim$
			_								F		$\sim$
			-							At 19.65m: Fracture inclined 90 degrees	Ŧ		$\sim$
			-							planar rough open clean. At 19.65m: Induced fracture inclined 30	Ŧ		$\sim$
			-							degrees planar striated tight clean.	Ŧ		~
			-								Ŧ		$\sim$
			-		I	1 1					t		$\sim$

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (4 of 8)

Equipment Dando 2000

Massenza MI6 Water

Drill Fluid

**Drill Crew Dates Drilled** 

21/01/2010 Start End 10/02/2010 **Borehole Diameter** 

200mm to 6.60m 131mm to 35.00m

**Casing Diameter** 200mm to 6.60m 150mm to 6.60m **BOREHOLE No.** 

Coordinates (Local Grid) Ground Level

232162 **E** 709840 **N** 12.04 m OD

BH3

Logged by Compiled by Approved by 25/01/2010 29/01/2010 12/04/2010

Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date   Date			End	10/02/2				25/		.0 29/0	1/2010 12/04/2010			
Time (m) Regular To To To Sco No. (m) To To Sco No. (m) To No. (m)	Data	Casina	Water	Sam	nple/Co	re Rec	overy		SPT	Pocult		Depth		
Time (m) Regular To To To Sco No. (m) To To Sco No. (m) To No. (m)			(m)		, ,	Туре	No.		/N	or	December of Streets	(Thick-	Level	Legend
At 20.30m; Quarts voin. At 20.12m; Closely spaced fractures and stepped rough. At 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined to subvertical fracture open 5mm and infilled with sensy clay.  NI  (90) 20.50-22.50 93 69 30  2  NI  At 23.50m; Induced fracture inclined 40 degrees planar slightly rough clean.			(Flush	Depth	(m)			DO D	Core	Fracture	Description of Strata	`ness)		
At 20.30m; Quarts voin. At 20.12m; Closely spaced fractures and stepped rough. At 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined 10 and at 20.40m; Inclined to subvertical fracture open 5mm and infilled with sensy clay.  NI  (90) 20.50-22.50 93 69 30  2  NI  At 23.50m; Induced fracture inclined 40 degrees planar slightly rough clean.	Time	(m)	Return)	From	To	I CR	SCR %	RQD %	Size (mm)	Index		(m)		
09/02 6.60 1.40			-,-						(,			(,		
09/02 6.60 1.40				_								ł		$\sim\sim$
09/02 6.60 1.40				-								Į.		~~~
09/02 5.60 1.40				-							At 20.30m: Quartz vein.	ţ.		~~~
10/02 6.60 1.40				-							inclined 30 degrees to 40 degrees plans	ar		~~~
AZCL	09/02	6.60	1.40	-							and stepped rough.	1		~~~
Miles	10/02	6 60								AZCT.	At 20.43m: Induced fracture inclined 1	0 -		~~~
Setween 21.04m and 21.44m: Inclined to subwartical fracture open 5mm and infilled with sandy clay.   NI	10,02	0.00								АДСЬ	moderately open clean.	Lea		~~~
Between 21.04m and 21.44m; Inclined to subvertical fracture open Smm and infilled with sandy clay.				-								Į		~~~
Between 21.04m and 21.44m; Inclined to subvertical fracture open Smm and infilled with sandy clay.				-						3		ţ		~~~
Subvertical fracture open Sms and infilled with sandy clay.				_						,		ŧ		~~~
Subvertical fracture open Sms and infilled with sandy clay.											Datasan 01 04m and 01 44m. Turking to	Ŧ		~~~
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				_						-	subvertical fracture open 5mm and	F		$\sim\sim$
(90) _ 20.50-22.50 93 69 30				_							infilled with sandy clay.	ŧ		~~~
(90) _ 20.50-22.50 93 69 30												Ŧ		$\sim\sim$
(90) 22.50-23.90 100 100 84  At 23.80m; Induced fracture inclined 40 degrees planar slightly rough clean.										NI		ţ		~~~
(90) 22.50-23.90 100 100 84  At 23.80m; Induced fracture inclined 40 degrees planar slightly rough clean.				_								t		$\sim\sim$
(90) 22.50-23.90 100 100 84  At 23.80m; Induced fracture inclined 40 degrees planar slightly rough clean.			(00)	20 50	.22 =^	0.2	60	20				ł		~~~
NI			(30)	_ 20.30- -	22.50	93	69	30				F		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
NI				_						_		t		~~~
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-						2		+		~~~
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.												Į.		~~~
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.												<u> </u>		~~~
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								+		$\sim\sim$
(90) 22.50-23.90 100 100 84  At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.												F		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.										NI		<u> </u>		$\sim\sim$
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								+		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								F		$\sim\sim$
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.												į.		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				_								Ł		$\sim\sim$
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								F		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								Į.		~~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				_								Ł		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								ł		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								Į.		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.												Ł		~~~
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.				-								ł		$\sim\sim$
At 23.80m: Induced fracture inclined 40 degrees planar slightly rough clean.			(90)	22 50-	.23 90	100	100	84				Ī		~~~
degrees planar slightly rough clean.			(30)		20.30	1 -00	1.00	04				ţ.		$\sim\sim$
degrees planar slightly rough clean.				<u> </u>								t		<b>~~~</b>
degrees planar slightly rough clean.												Ī		$\sim\sim$
degrees planar slightly rough clean.				_								<b>‡</b>		~~~
degrees planar slightly rough clean.				-								t		~~~
degrees planar slightly rough clean.				-								Ŧ		~~~
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Damanica	Remar	L	Ļ			<u> </u>	Щ_		Щ			ļ.	<u> </u>	<del></del>

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (5 of 8)

Equipment

Dando 2000

**Drill Fluid Drill Crew Dates Drilled**  Massenza MI6

Water

**Borehole Diameter** 200mm to 6.60m 131mm to 35.00m **Casing Diameter** 200mm to 6.60m 150mm to 6.60m

**BOREHOLE No.** Coordinates (Local Grid) Ground Level

232162 E 709840 N 12.04 m OD

BH3

Logged by Compiled by Approved by

Drill C Dates	rew Drilled		21/01/2010				ged by			pproved by				
		End Water	10/02/2010 Sample/Co	ro Por	overv			.0 29/0	1/2010	12/04/2010		Τ		Ī
Date & Time	Casing Depth (m)	Depth (m)		Туре	No.		SPT Blows /N	Fracture		Description of	of Strata	Depth (Thick- ness)	Level	Legend
Tille	(111)	(Flush Return) %	From To	%	SCR %	%	Size (mm)	Index				(m)		
			- - - - -								ture inclined 20 th tight clean.	(19.60)		
		(90)	23.90-27.00	100	100	93					ture inclined 30	-		
			- - - - - - -						degrees p	lanar rough ti	ght clean.	<u> </u>		
			- - - - - -											
			- - - - -							: Induced fractilanar smooth t	ture inclined 30 ight clean.			
			-									<del>-</del>		
			- - - - -									<u> </u>  - 		
			- - - - -									<u>-</u>		
			- - - -											
		(90)	27.00-30.00 	100	95	86		2				-  -  -  -  -		
			- - - - -						At 29.10m planar sm	: Fracture inc	lined 30 degrees an	<u> </u>  -  -		
			- - - -							: Fracture incooth open clea	lined 20 degrees	<del>-</del>		
			[ -							: Fracture inc g smooth tight	lined 20 degrees clean.	<u> </u>		~~~ ~~~

Remarks

(See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (6 of 8)

Equipment

Drill Fluid **Drill Crew Dates Drilled**  Dando 2000 Massenza MI6

Water Start End 21/01/2010 **Borehole Diameter** 200mm to 6.60m 131mm to 35.00m **Casing Diameter** 200mm to 6.60m 150mm to 6.60m **BOREHOLE No.** 

232162 E

BH3

Logged by Compiled by Approved by Coordinates (Local Grid) Ground Level 709840 N 12.04 m OD

Date & Casing Net (m) Depth (m) Depth (m) (m) Depth (m) (m) No. Depth (m) (m) No. TCR SCR RQD (mm) (m) No. TCR SCR RQD (mm) No. TCR SCR	. Level	Legend
Time   Depth (m)   Figure   Town	. Level	Legend
Depth (m)   Flush Return   To   To   To   Size   Rob   Size   Fracture   Index   Return   Return   Return   From   To   To   To   Size   Rob   R		
(90) - 30.00-31.50 96 86 68   At 31.37m: Quartz band (40mm).		
0		
(90) 30.00-31.50 96 86 68  At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  (90) - 31.50-33.00 100 100 89  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  (90) - 31.50-33.00 100 100 89  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.37m: Quartz band (40mm).  At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		**************************************
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		$\sim\sim$
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.	1	~~~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.	1	$\sim$
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
At 31.77m: Induced fracture inclined 20 degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
degrees smooth planar open clean.  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
(90) - 31.50-33.00 100 100 89  At 32.30m: Induced fracture inclined 25 degrees planar slightly rough moderately open clean.		~~~
degrees planar slightly rough moderately open clean.		~~~
degrees planar slightly rough moderately open clean.		~~~
degrees planar slightly rough moderately open clean.		~~~
degrees planar slightly rough moderately open clean.		~~~
degrees planar slightly rough moderately open clean.		~~~
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[90) [33.00-34.50 99 93 82] 1		~~~
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		~~~
		~~~
		~~~
		~~~
		~~~
AZCL		~~~
Between 34.72m and 34.85m: Quartz band.	1	~~~
(90) 34.50-35.00 90 90 54 0	1	~~~
		~~~
10/02 6.60 1.40		~~~
End of Borehole		'
Remarks	-22.96	~~~

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR3 (7 of 8)

Drilling Method Cable Percussion & Rotary **BOREHOLE No. Borehole Diameter** BH3 Coordinates (Local Grid) Ground Level Dando 2000 131mm to 35.00m Equipment 232162 E Massenza MI6 709840 **N** 12.04 m OD Water **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** Start 21/01/2010 End 25/01/2010 12/04/2010 10/02/2010 29/01/2010 Installation Water Strata Level **Installation Details** Strata Details Depth (m) Strikes m OD Depth (m) Bentonite Grout MADE GROUND Instrumentation: 50mm slotted section from 2.00 to 4.76m 2.00 10.04 Gravel Filter 4.76 7.28 Bentonite Grout 6.50 Coarse/Medium grained Metamorphic (SCHIST) 35.00 -22.96 35.00 Base of Hole Remarks (See notes & keysheets) Water Strike ▼ Water Rise Upstanding cover. Pipe diameter 50mm to 4.76m. Project Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR3 (8 of 8)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No.** 200mm to 3.50m 200mm to 2.90m Equipment Dando 2000 131mm to 35.00m 150mm to 5.50m 232158 Massenza MI6 **Drill Fluid**

Water **Drill Crew** Compiled by Logged by Approved by **Dates Drilled** 21/01/2010 Start

BH4 Coordinates (Local Grid) Ground Level F 709854 N 13.15 m OD

Dates	Dillica	End	04/02/2010			27/	01/201	.0 29/0	1/2010 12/04/2010			
Date	Casing	Water Depth	Sample/Co	1	r -		SPT Blows /N	Result or U100		Depth		
&	Depth	(m) (Flush	Depth (m)	Туре		ĺ		Rec. mm	Description of Strata	(Thick- ness)	Level	Legend
Time	(m)	Return)	From To	TCR %	SCR %	RQD %	Core Size (mm)	or Fracture Index		(m)		
21/01		,,,	-				()	IIIGEX	TOPSOIL: Composed of dark brown organic	(0.10)		
			-						sandy angular to subrounded fine to medium gravel of mica schist with	0.10	13.05	
			-						frequent roots. Sand is medium to coarse with frequent mica.	(0.30)		
			-						MADE GROUND: Composed of sandy gravelly	0 40	12.75	
			_						subangular cobbles of mica schist. Gravel	- 0.40	12.75	
			-						is subangular fine to coarse mica schist. Sand is micaceous medium to coarse.			
			-						MADE GROUND: Composed of dark brown	-		
									locally clayey sandy gravelly angular cobbles and boulders of mica schist.			
			<u>.</u>						Gravel is angular to subangular fine to coarse mica schist. Sand is medium to	-		
			-						coarse.			
		Dry	1.20-1.65	c			C50/			_		
		-	1.20	В	1		160			(1.80)		
			1.20-1.70	В	2							
			- :							-		
			-									
			-							-		
	1.90	Dry	_ 2.00-2.45 - 2.00	C D	3		C17			-		
			2.00-2.50	В	4					2.20	10.95	
			<u> </u>						MADE GROUND: Composed of brown sandy angular to subangular fine to coarse	[
									gravel of mica schist and occasional quartz with many angular to subangular			
			<u>-</u>						cobbles of mica schist up to 130mm. Sand is fine to coarse.	-		
									is line to coarse.	-		
			2.70	D	5					- (1 22)		
										(1.30)		
	2.90	DRY	3.00-3.45	С			C72/			-		
			3.00-3.50				125					
			3.10-3.60	С								
			-									
			<u>. </u>							3.50	9.65	
21/01		DRY	-	<u> </u>					Possibly medium strong mica SCHIST recovered as angular fragments.	(0.10)	9.55	~~~
22/01		(0)	3.60-3.64				C50/ 10			3.00	,.,,	
			- - -				1	0	Strong to very strong fractured grey and dark grey quartz mica SCHIST. Moderately	[
			- -						weathered. Fractures are closely spaced inclined with orange staining on surfaces			~~~
			<u> </u>						penetrating <5mm. Occasional quartz bands 20mm to 40mm thick.	-		~~~
			- - -					NI	Between 3.60m and 5.90m: Fractures are predominantly subvertical to vertical	[~~~
			- -						closely to medium spaced and cemented with quartz.	-		~~~
			-						At 4.40m: Subvertical open fracture with clay infill 5mm to 10mm.			~~~
			- -							<u> </u>		$\sim\sim$
			- -									
		(80)	3.60-5.90	81	68	35				(2.30)		~~~~
			- - -							[~~~
			<u>-</u>						At 5.03m: Subvertical fracture open with	_		~~~
									clayey infill up to 5mm.			

Remarks 1 (See notes & keysheets)2

Scale 1:25

A hydraulic breaker was used to break out hard strata at ground level.

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.

Rotary coring was carried out using a core barrel 131mm in diameter.

The borehole was advanced by cable percussion means to 3.60m and then progressed by rotary coring means to

4

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 1.60m to 1.90m (60 mins); from 3.50m to 3.60m (60 mins).



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR4 (1 of 8)

Equipment Dando 2000

Drill Fluid **Drill Crew** Massenza MI6

Water

21/01/2010 04/02/2010 **Dates Drilled** Start

Borehole Diameter 200mm to 3.50m 131mm to 35.00m **Casing Diameter** 200mm to 2.90m 150mm to 5.50m

Coordinates (Local Grid) Ground Level

BOREHOLE No.

BH4 232158 E

709854 **N** 13.15 m OD

Logged by Compiled by Approved by

		End	04/02/2010			27/	01/201	.0 29/0	1/2010 12/04/2010			
Data	Casina	Water Depth	Sample/Co	re Rec	overy		SPT Blows	Result		Depth		
Date &	Casing	(ṁ)	Douth (m)	Туре	No.	l	/N	or	Description of Strata	(Thick-	Level	Legend
Time	Depth (m)	(Flush Return)	Depth (m)		SCR	POD	Core	Fracture	Description of Strata	`ness)		
Tille	(111)	Return)	From To	%	%	%	Core Size (mm)	Index		(m)		
			-					4		<u> </u>		~~~
			-							ţ		~~~
			<u>-</u> -						At 5.27m: Slightly weathered subvertical	ţ		$\sim\sim$
			-						fracture open with clay infill up to 5mm.	ł		$\sim\sim$
			-							Ŧ		~~~
			_							<u> </u>		$\sim\sim$
			-							‡		~~~
			-							t		~~~~
			_							 		$\sim\sim$
			-							Ŧ		~~~
										5.90	7.25	$\sim\sim$
			- -					AZCL	Strong to very strong fractured grey and dark grey quartz mica SCHIST. Moderately	ļ ,		$\sim\sim$
			-						to slightly weathered. Fractures are	t		~~~
			-						closely to medium spaced inclined with some orange staining on surfaces	ł		~~~~
			<u>-</u> -						penetrating <2mm. Occasional quartz bands	F		$\sim\sim$
		(100)	5.90-7.00	92	63	30			20mm to 40mm thick. Between 5.90m and 6.40m: Fractures are	Ŧ		~~~
		(100)	_ 5.90-7.00	92	63	30		1	drilling induced predominantly	L		$\sim\sim$
			_ _						subvertical to vertical medium to widely	t		~~~
			-						spaced and cemented with quartz.	ł		~~~
			<u>-</u>							F		$\sim\sim$
										Į.		$\sim\sim$
22/01	5.50	1.50	= =							ţ		~~~
25/01	5.50					-				<u>F</u>		$\sim\sim$
23,01	3.30		-							ł		~~~
			-							F		~~~~
			= -							F		$\sim\sim$
			- -							ţ		~~~
			<u>-</u>						Between 7.55m and 7.56m: Induced fracture	L		~~~~
			-						inclined 10 degrees planar rough	ţ		$\sim\sim$
			-						moderately open clean.	+		~~~
			=					1.0	Between 7.80m and 7.85m: Quartzite vein.	F		~~~
		(100)	7.00-8.80	100	100	81		10		‡		$\sim\sim$
		(===,								ţ		$\sim\sim$
			<u> </u>							F		~~~
			-						Between 8.15m and 8.18m: Fractures	ł		$\sim\sim$
			-	1					inclined 20 degrees stepped smooth open clean.	ł		~~~
			=							F		~~~~~
			-						Between 8.46m and 8.54m: Fractures	ţ		$\sim\sim$
			-						inclined 65 degrees stepped rough open	L		~~~
			- -						with micaceous crystals <3mm.	t		~~~~
			-					10		t		$\sim\sim$
			<u> </u>							+		~~~
										Į.		~~~~
			-							ţ		$\sim\sim$
			-							†		~~~
			_	1						t		$\sim\sim$
			_							Ł		~~~
			F	1						F		
			-						Between 9.47m and 9.49m: Induced fracture	ţ		$\sim\sim$
			<u>-</u>						inclined 30 degrees undulating smooth	L		~~~
			_	1					moderately open clean.	t		~~~~
			-							+		~~~
			F	1						F		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			-							Ŧ		~~~~
		(100)	8.80-11.10	100	100	88				†		~~~
			_									
Domor	<u> </u>		gtallation do			<u> </u>	Ь—					

Remarks 7 (See notes & keysheets) 8

See installation details on final sheet. Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (2 of 8)

Equipment

Drill Fluid **Drill Crew Dates Drilled** Dando 2000 Massenza MI6

Start 21/01/2010 **Borehole Diameter**

Casing Diameter 200mm to 3.50m 131mm to 35.00m 200mm to 2.90m 150mm to 5.50m **BOREHOLE No.**

Coordinates (Local Grid) Ground Level

232158 E 709854 N 13.15 m OD

BH4

Approved by Logged by Compiled by

		End	04/02/2010			27/		.0 29/0	1/2010 12/04/2010			
&	Casing Depth	Water Depth (m) (Flush	Sample/Co	Туре	No.		SPT Blows /N	Result or Fracture	Description of Strata	Depth (Thick- ness)	Level	Legend
Time	(m)	Return)	From To	TCR %	SCR %	RQD %	Core Size (mm)	Index		(m)		
		70	-				,			,		~~~
			_						Between 10.23m and 10.25m: Fracture inclined horizontal undulating rough open clean.	Į Į		~~~
			-						Between 10.38m and 10.43m: Fracture inclined 45 degrees undulating clean	ļ		~~~
			- -						moderately open.	<u> </u>		~~~
			<u>-</u>							-		~~~
										Ī Ī		~~~
25/01	5.50	1.00	_							<u>-</u>		~~~
26/01	5.50		- - -							<u> </u>		
			-					1		<u> </u>		
		(100)	- - 11.10-12.00	100	96	87				‡		~~~
			-							ļ		~~~~
										<u> </u>		~~~
			<u>-</u> -							<u> </u>		~~~
			-							<u> </u>		~~~
			- - -							‡		~~~
			<u>-</u>						Between 12.50m and 12.75m: Incipient fracture inclined 85 degrees stepped	<u> </u>		~~~
									rough tight to moderately open intersecting induced fracture clean.	[~~~
			- -						At 12.68m: Induced fracture horizontal planar rough clean.	‡		~~~
			- - -							<u> </u>		~~~
			-							(14.60)		~~~
										,		~~~
		(100)	_ 12.00-15.00	100	100	73						~~~
			-							‡		
			-							‡		~~~
										<u>E</u>		~~~ ~~~
			-					15	Between 14.10m and 14.12m: Induced fracture inclined 15 degrees undulating	F .		~~~
			<u>-</u>						rough. Between 14.37m and 14.39m: Induced	<u>†</u>		~~~
			-						fracture. Between 14.48m and 14.80m: Fracture subhorizontal undulating smooth open	‡		~~~ ~~~
			- - -						clean with quartz on surfaces <3mm.	Ī		~~~ ~~~
									Between 14.70m and 14.74m: Fracture inclined 25 degrees stepped smooth clean with quartz veining on surfaces.	Į.		~~~~
			-						quarter verning on surfaces.	†		~~~
										<u> </u>		

Remarks

(See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (3 of 8)

Equipment Dando 2000 Massenza MI6

Drill Fluid

Water

Drill Crew Dates Drilled

21/01/2010 Start End 04/02/2010 **Borehole Diameter** 200mm to 3.50m 131mm to 35.00m **Casing Diameter** 200mm to 2.90m 150mm to 5.50m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232158 E 709854 **N** 13.15 m OD

BH4

Logged by Compiled by Approved by 27/01/2010 29/01/2010 12/04/2010

		End	04/02/2010			2//	01/201	0 23/0	1/2010 12/04/2010	_	1	1
Date	Casing	Water Depth	Sample/Co	1			SPT Blows	Result		Depth	l	1.
&	Depth	(m) (Flush	Depth (m)	Туре		DOD.	/N Core	or Fracture	Description of Strata	(Thick- ness)	Level	Legen
ime	(m)	Return)	From To	%	SCR %	RQD %	Size (mm)	Index		(m)		
			-				`			+ ` ′		~~~
			-							ţ		~~~
			-							ļ.		~~~
			-							†		~~~
			<u>-</u>							t		~~~
			<u> </u>					1		<u>F</u>		~~~
			-							ł		~~~
			-							Ŧ		$\sim\sim$
			=							Ŧ		~~
			-							Į.		~~
			-							L		$\sim\sim$
			- -							†		\sim
			- -						Between 16.20m and 16.24m: Fracture	ţ		~~~
			-						inclined 20 degrees stepped rough open clean abundant quartz crystals.	ł		\sim
			-						Between 16.20m and 16.24m: Fracture	ł		$\sim\sim$
		(100)	_ 15.00-18.00	100	98	74			inclined 45 degrees stepped smooth moderately open clean.	Ŧ		~~~
		(100)	- 13.00-18.00	100	90	/ =			Between 16.28m and 16.31m: Induced	F		~~~
			-						fracture.	‡		$\sim\sim$
			- -						Between 16.75m and 16.84m: Fracture	‡		~~
			-						inclined 75 degrees stepped rough moderately open clean.	†		~~~
			<u>-</u>						Between 16.75m and 17.05m: Fracture	t		$\sim\sim$
			<u> </u>						inclined 85 degrees undulating smooth clean.	<u> </u>		$\sim\sim$
			-					7	Between 17.05m and 17.10m: Fracture	+		~~~
			-						inclined 45 degrees undulating smooth clean.	Ŧ		~~~
			= =						Clean.	Ŧ		$\sim\sim$
			-							‡		~~
			<u>-</u>							1		~~~
			= =							‡		\sim
			-					0		†		$\sim\sim$
			= -							<u>†</u>		~~~
			-						Between 17.88m and 18.15m: Dark grey	ł		~~~
6/01	5.50	1.30	-						structureless psammite layer. Quartz veining abundant.	Ŧ		$\sim\sim$
7/01	5.50							AZCL	Between 18.03m and 18.08m: Fracture	F		~~~
									subvertical inclined 20 degrees stepped rough to smooth clean.	Ī		~~~
			-						2003 00 5000 0200	‡		$\sim\sim$
			=							‡		~~~
			-							‡		~~~
			-							†		~~~
			-							ł		$\sim\sim$
			-							Ł		~~~
			-						Between 18.85m and 19.93m: Fracture	+		~~~
			=						inclined 45 degrees undulating smooth with quartz on surfaces clean.	Ŧ		\sim
			<u>-</u>						2	1		$\sim\sim$
			-							1		~~~
		(100)	10 00 00 50							‡		~~~
		(100)	18.00-20.50	96	96	64		1		†		$\sim\sim$
			- -							t		~~~
			<u>-</u> _							Ł		~~~
			-							F		$\sim\sim$
			F							Ŧ		~~
			<u>-</u>							‡		~~`
			<u>-</u>							ţ		~~~
			ŀ		1		Ī			t		~~~
			<u> </u>							+		$\sim\sim$

Remarks

(See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (4 of 8)

Equipment Dando 2000

Drill Crew Dates Drilled Massenza MI6

Drill Fluid Water

> Start End 21/01/2010 04/02/2010

Borehole Diameter

200mm to 3.50m 131mm to 35.00m

Casing Diameter 200mm to 2.90m

150mm to 5.50m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232158 E 709854 N 13.15 m OD

BH4

Logged by Compiled by Approved by 27/01/2010 29/01/2010 12/04/2010

		End	04/02/2010			27/	01/201	0 29/0	1/2010 12/04/2010			
Data	Ci	Water Depth	Sample/Co	re Rec	overy		SPT Blows	Result		Depth		
	Casing	(m)	B (1 /)	Туре	No.	l	/N	or	Description of Strate	(Thick-	Level	Legend
& Time	Depth	(Flush	Depth (m)	TCD	SCR	BOD	Core	Fracture	Description of Strata	`ness)		
Time	(m)	Return)	From To	%	%	%	Size (mm)	Index		(m)		
					_		` ,		At 20.10m: Induced fracture.	,		2000
			-						At 20.10m. Induced Hacture.	ł		~~~
			=						At 20.23m: Inclined 80 degrees (75%) and	F		~~~
			-						inclined 10 degrees (25%) undulating smooth clean.	ţ		~~~~
			-							ţ		$\sim\sim$
27/01	5.50	1.30	-							_20.50	-7.35	~~~
29/01	5.50	1.40	_					AZCL	Very strong fractured grey green quartz	F20.30	-7.33	$\sim\sim$
			-						mica SCHIST. Slightly weathered.	Į.		~~~
			- -						Fractures are medium spaced inclined planar rough to planar smooth.	ţ		~~~
			_							t		$\sim\sim$
			- -							Ŧ		~~~
			_							<u>t</u>		~~~
			-							‡		$\sim\sim$
			<u>-</u>						Between 21.22m and 21.25m: Induced	t		~~~
			_						fracture inclined 16 degrees undulating	F		$\sim\sim$
			-						rough clean.	Į.		~~~
			- -							ţ		~~~
			<u> </u>							F		$\sim\sim$
			-							ł		~~~
			-						Determine 01 EEs and 01 00m Tedans d	Į.		$\sim\sim$
			- -						Between 21.75m and 21.80m: Induced fracture inclined 75 degrees undulating	ţ		~~~
		(80)	20.50-23.25	99	99	59			open rough clean.	ţ		~~~~
			-							-		$\sim\sim$
			-						Between 22.12m and 22.17m: Induced	F		~~~
			-						fracture inclined 70 degrees (25%) and	ţ		$\sim\sim$
			<u> </u>						inclined 15 degrees (75%) undulating rough tight clean.	Ł		~~~
			-							-		~~~
										Į.		$\sim\sim$
			- -							<u>t</u>		~~~
			-						Determine 00 CFm and 00 F0m. To deduct	ţ		$\sim\sim$
			-					1	Between 22.67m and 22.70m: Incipient fracture inclined 80 degrees undulating	ł		~~~
			-					_	smooth tight clean and horizontal	Į.		~~~
			-						fracture intersecting undulating rough clean.	ţ .		~~~
			-						Between 22.85m and 24.05m: Quartz vein.	ţ		~~~
			_							┢		$\sim\sim$
										Į.		~~~
29/01	5.50	1.40	-							ţ		$\sim\sim$
04/02	5.50		-							t		~~~
04/02] 3.30		-							F		~~~~~
			F							F		$\sim\sim$
			- -							F		~~~
			- -						Between 23.65m and 23.69m: Fracture	t		~~~
			_						inclined 10 degrees stepped rough moderately open clean.	Ł		~~~
	1		F			Ī				F		~~~~
			-							ţ		$\sim\sim$
			<u>-</u>							L		~~~
			-							ł		$\sim\sim$
		(80)	23.25-25.10	100	100	81				Ţ.		~~~
		(30)				"-			Between 24.30m and 24.84m: Quartz vein	‡		~~~~
	1		Ė			Ī			with assimilated schist. Highly fractured	t		~~~
			-						on content with schist at the base. All fractures are cemented.	 		~~~~
			_							ļ.		~~~
	1		- -			Ī				t		~~~
			<u>-</u> -						Between 24.70m and 25.50m: Fractures	t		~~~~
			<u>-</u>						inclined 45 degrees undulating smooth	+		~~~
			-						open slightly weathered.	Ī		~~~~
			-							t		$\sim\sim$
			-							 		

Remarks

(See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (5 of 8)

Equipment

Drill Crew

Dando 2000 Massenza MI6

Drill Fluid Water

21/01/2010 Start End **Dates Drilled** 04/02/2010 **Borehole Diameter**

200mm to 3.50m 131mm to 35.00m

Casing Diameter 200mm to 2.90m 150mm to 5.50m **BOREHOLE No.**

Coordinates (Local Grid) Ground Level

232158 E 709854 **N** 13.15 m OD

BH4

Logged by Compiled by Approved by 27/01/2010 29/01/2010 12/04/2010

		Water		mml-12	- n		-	CDT	1				1
ate	Casing	Depth	Sar	nple/Co	_	-		SPT Blows	Result		Depth	١	l.
&	Depth	(m) (Flush	Depth	(m)	Туре			/N Core	or Fracture	Description of Strata	(Thick- ness)	Level	Lege
ime	(m)	Return)	From	To	TCR	SCR %	RQD %	Size (mm)	Index		(m)		
			-								· ` ′		\sim
			-						_		ţ		~~
			- -						6		‡		~~~
			<u>-</u>								t		\sim
			-								ł		\sim
			-							Between 25.54m and 27.00m: Quartz vein	F		~~~
			-							with cemented fractures.	F		\sim
		(00)	05 10	-26.45	100	96	63				Į		\sim
		(80)	25.10	-26.45	100	96	63				ţ		~~
			-							Between 25.90m and 25.92m: Induced	‡		~~
			- -							fracture horizontal undulating rough tight clean.	L		\sim
			-							Between 26.10m and 27.45m: Quartzite band.	ţ		~~
			-							Between 26.20m and 26.21m: Fracture	t		~~
			-							horizontal undulating smooth open along schistose vein and pyrite <5mm.	ł		\sim
			-							Between 26.37m and 26.47m: Fracture	Ŧ		~~
			-							inclined horizontal 30 degrees undulating	Ŧ		~~
			-							to stepped smooth rough clean.	F		\sim
			-								ţ		\sim
			- -								‡		~
			-								ţ		~~
		(80)	26.45	-27.45	100	98	89				ł		\sim
			_								F		\sim
			-								F		~
			-								ļ.		\sim
			- -								ţ		\sim
			- -								‡		~
			_								L		~~
			-								ţ		\sim
			-								ł		\sim
			_							Between 27.82m and 27.84m: Induced	(14.50)		~~
			-							fracture inclined 80 degrees and 30	Ŧ		\sim
			-							degrees undulating smooth tight clean. Between 27.92m and 27.93m: Fracture	Į.		\sim
			-							horizontal undulating rough and smooth	F		~
			- -							open clean with abundant mica <3mm.	‡		\sim
			-								ţ		\sim
			- -							Between 28.36m and 28.41m: Induced	ţ		~
			-							fracture inclined 45 degrees planar rough moderately open with abundant mica <3mm.	ł		~
			_								⊢		\sim
			-								F		\sim
			-								Ŧ		~
			- -								Ī		\sim
			-								‡		\sim
		(80)	27.45	-30.60	100	96	83				L		\sim
			-							Between 29.09m and 29.12m: Induced fracture inclined 25 degrees undulating	t		~~
			-							rough tight with frequent mica crystals.	t		\sim
			-								t		\sim
			<u>-</u>							Between 29.38m and 29.39m: Induced	ł		~
			-							fracture inclined 15 degrees planar smooth clean.	F		\sim
			-							pmoodii Cicaii.	F		\sim
			-								‡		~
			<u>-</u>								‡		\sim
			-							Between 29.91m and 29.94m: Induced	t		\sim
			-							fracture 45 degrees undulating rough open	t		~~
			_			1				clean.	L	l	Γ

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (6 of 8)

Equipment

Drill Crew

Dando 2000 Massenza MI6

Drill Fluid Water

21/01/2010 Start End **Dates Drilled** 04/02/2010 **Borehole Diameter** 200mm to 3.50m 131mm to 35.00m

Compiled by

29/01/2010

Logged by

27/01/2010

Casing Diameter 200mm to 2.90m 150mm to 5.50m

Approved by

12/04/2010

BOREHOLE No.

Coordinates (Local Grid) Ground Level

232158 E 709854 N 13.15 m OD

BH4

		Ena	04/02/2						0 29/0	1/2010 12/04/2010			
		Water	Sam	ple/Co	re Rec	overy		SPT Blows	Deguit		Depth		
	Casing	Depth (m)			Туре	r -	ı	Blows /N				Level	Legen
&	Depth		Depth	(m)	ype	INO.			or	Description of Strata	(Thick- ness)	Level	Legen
Time	(m)	(Flush Return)	•	` '	TCR	SCR	RQD	Core Size (mm)	Fracture Index	•	ness)		
	(,	%	From	To	%	%	%	(mm)	inaex		(m)		
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			<u> </u>								t		$\sim\sim$
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			Ī								t		~~
			_								ŀ		~~~
			-								t		\sim
			L							Between 31.00m and 31.18m: Induced	L		~~~
			L							fractures inclined between 30 and 45	}		$\sim\sim$
			Ī							degrees smooth planar open clean.	t		$\sim\sim$
			Į.								Į.		\sim
			-								+		~~~
										Determine 21 42m and 21 45m Today 3	I	Ī	~~~
			ļ							Between 31.43m and 31.47m: Induced fracture inclined 30 degrees undulating	}	Ī	~~
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		(00)	20.60	22 25	100		70			D-1	}		$\sim\sim$
		(80)	_ 30.60-	33.35	100	94	79			Between 32.04m and 32.20m: Induced fractures inclined 15 degrees undulating	Ħ		\sim
										smooth open to very open clean.	Į		~~~
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			Ė.							Between 32.75m and 32.77m: Induced	t		$\sim\sim$
			<u>[</u>							fracture inclined 15 degrees undulating	Į.		$\sim\sim$
			-							smooth open clean.	ł		~~
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			Ī							Between 33.23m and 33.16m: Induced	Į		~~~
			L							fracture inclined 20 degrees stepped	}		~~
			-							<pre>smooth moderately open clean. Between 33.23m and 33.31m: Fracture</pre>	Ţ	Ī	$\sim\sim$
					 					inclined 45 degrees undulating smooth	}		~~~
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			-							Between 33.74m and 33.79m: Fracture	-	Ī	~~
			<u> </u>							inclined 20 degrees (70%) and 65 degrees (30%) smooth planar clean.	t		~~~
										(50%) SMOOCH Planal Clean.	ļ	Ī	~~~
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			-							At 34.10m: Induced fracture inclined 15	-	Ī	$\sim\sim$
			+							degrees undulating rough tight clean.	t		$\sim\sim$
		(80)	33.35-	35.00	100	95	83				Ī		~~
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4/02	5.50	1.10										01.55	$\overset{\sim}{\sim}$
4/02	5.50	1.10								End of Borehole	_35.00	-21.85	$\overset{\sim}{\sim}$

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR4 (7 of 8)

Drilling Method Cable Percussion & Rotary **BOREHOLE No. Borehole Diameter** BH4 Coordinates (Local Grid) Ground Level 131mm to 35.00m Equipment Dando 2000 232158 E Massenza MI6 709854 **N** 13.15 m OD Water **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** 21/01/2010 Start 27/01/2010 29/01/2010 12/04/2010 End 04/02/2010 Installation Water Strata Level **Installation Details** Strata Details Depth (m) Strikes m OD Depth (m) Bentonite Grout Coarse/Medium grained Instrumentation: Metamorphic (SCHIST) 50mm slotted section from 2.00 to 4.00m 2.00 Gravel Filter 4.00 9.15 Bentonite Grout 35.00 -21.85 35.00 Base of Hole Remarks (See notes & keysheets) Water Strike ▼ Water Rise Upstanding cover. Pipe diameter 50mm to 4.00m. Project Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR4 (8 of 8)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter** BOREHOLE No. BH₅ 200mm to 1.30m 200mm to 1.30m Equipment Dando 2000 131mm to 3.40m 150mm to 3.20m Coordinates 232127 F Massenza MI6 Local Grid) 709854 **N Drill Fluid** Ground Level 12.10 m OD Water **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** Start 27/01/2010 End 29/03/2010 23/02/2010 02/02/2010 12/04/2010 Water Sample/Core Recovery SPT Depth Depth (m) Blows /N Date Casing No. Level Legend Type (Thick-Depth & Depth (m) **Description of Strata** (Flush Return) % ness) Core Size (mm) Time (m) TCR % SCR RQD From To % (m) MADE GROUND: Composed of brown organic sandy angular to subrounded fine to coarse gravel of mica schist and quartz. 27/01 (0.10) 12.00 0.10 Sand is fine to coarse. Frequent roots. MADE GROUND: Composed of grey and dark brown organic medium to coarse sand and angular to subangular fine to coarse gravel of concrete, coal, schist, wood and quartz with many angular cobbles of schist. Occasional roots. 0.60-1.20 в 1 (1.10)27/01 1.20 DRY 1.20 10.90 C75/ 28/01 1.20 1.20 C Possibly medium strong weathered mica SCHIST recovered as angular to subangular cobble sized fragments. 1.30 DRY 28/01 23/02 1.30 (2.20) At 3.40m: Possible obstruction. (40) 3.20-3.40 23/02 3.40 3.40 8.70 End of Borehole Remarks 1 Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m (See notes & keysheets) depth and rescanned using the CAT to check for services. Services were not located. On completion the borehole was grouted up to ground level.

Possible metal obstruction encountered at 3.40m. Borehole abandoned and redrilled as BH5A.

The borehole was advanced from ground level to 1.30m by cable percussive techniques; to 3.20m by rotary open hole drilling and from 3.20m to 3.40m by rotary symmetrix drilling. Rotary coring was carried out using a core barrel 131mm in diameter. An amount of water was added to facilitate boring in granular strata. Scale 1:25 Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR5 (1 of 2) 308/03

Equipment Dando 2000

Drill Fluid Drill Crew Massenza MI6

Water

Dates Drilled 27/01/2010 End 23/02/2010 **Borehole Diameter** 200mm to 1.30m 131mm to 3.40m

Casing Diameter 200mm to 1.30m 150mm to 3.20m **BOREHOLE No.**

Coordinates (Local Grid) Ground Level 232127 E 709854 **N** 12.10 m OD

BH5

Logged by Compiled by Approved by 29/03/2010 02/02/2010 12/04/2010

		End	23/02/2010				02/201		3/2010 12/04/2010				
Dette	Contin	Water	Sample/Co				SPT				Depth		
Date	Casing	l (''' <i>)</i>	Don't (a)	Туре	No.		SPT Blows /N		Description of	Strata	Depth (Thick- ness)	Level	Legend
& Time	Depth (m)	(Flush	Depth (m)			ROD	Core		Description of	Sudid	ness)		
e	\''' <i>'</i>	(Flush Return) %	From To	%	%	RQD %	Core Size (mm)				(m)		
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Remarks 7 (See notes & keysheets) 8 Borehole advanced by chiselling from 1.20m to 1.20m (20 mins); from 1.20m to 1.30m (20 mins). Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR5 (2 of 2)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No.** BH5A 131mm to 6.20m 150mm to 3.70m Coordinates (Local Grid) Ground Level Equipment 232135 Massenza MI6 709856 N **Drill Fluid** 12.10 m OD **Drill Crew** Logged by Compiled by Approved by **Dates Drilled**

ales L	Jilleu	End	23/02/2010			01/	03/201	0 04/0	3/2010 12/04/2010			
	Casing	Water Depth (m)	Sample/Co	Type			SPT Blows /N	Fracture Index	·	Depth (Thick-	Level	Lege
& ime	Depth (m)	(Flush Return) %	Depth (m) From To		SCR %	RQD	Core Size (mm)	or Result	Description of Strata	ness)		
3/02		70		70	/0	70	(11111)		Grass over MADE GROUND (drillers	(m)		
			- - -						description)	<u> </u>		
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			-							(1.20)		
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			<u> </u>							F I		
			- -						MADE GROUND: Composed of gravel with	1.20	10.90	
			- - -						cobbles and boulders (drillers description).	F		
			<u>-</u>						descripción/.	Ł I		
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			-							(2.50)		
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										3.70	8.40	
		(0)	3.70-3.90	100	100	0			Medium strong fractured thinly foliated light grey green grey and dark grey	<u> </u>		\sim
			-						quartz mica SCHIST. Slightly weathered. Fractures are very closely spaced subvertical open and infilled with fine	ļ		~~
			_						gravel of mica schist. Locally with orange staining on fracture surfaces.	F		\sim
			<u>-</u> -					>20		‡		\sim
			- -							F		\sim
		(90)	3.90-4.90	100	100	13				<u> </u>		\sim
			-							ţ		\sim
			_	I						†		\sim
									Between 4.75m and 6.20m: Fractures are	L '		
			-						Between 4.75m and 6.20m: Fractures are subhorizontal to subvertical closely spaced predominantly planar smooth.	(2.50)		~ ~ ~

Remarks 1 (See notes & keysheets)

- Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.
- On completion the borehole was grouted up to ground level.
- Rotary coring was carried out using a core barrel 131mm in diameter. The borehole was advanced by open hole (symmetrix) means to 3.70m and rotary coring to 6.20m. Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR6 (1 of 2)

Drilling Method Rotary Open Hole/Coring Borehole Diame

Equipment Massenza MI6

Drill Fluid Drill Crew Water

Dates Drilled Start 23/02/2010 End 23/02/2010 Borehole Diameter 131mm to 6.20m Casing Diameter 150mm to 3.70m BOREHOLE No.

 Coordinates
 232135
 E

 (Local Grid)
 709856
 N

 Ground Level
 12.10
 m OD

BH5A

Logged by Compiled by Approved by 01/03/2010 04/03/2010 12/04/2010

		End 23/02/2010				03/201	10 04/0	3/2010 12/04/2010				
Det -	Casimo	Water Depth (m)	Sample/Co	re Rec	overy		SPT	Fracture		Depth		
Date &	Casing Depth	(m)	Depth (m)	Туре	No.	l	/N	Fracture Index	Description of Strata	Depth (Thick- ness)	Level	Leger
τime	(m)	(Flush Return) %	Depth (III)	TCR	SCR %	RQD	Core Size (mm)	or Result	Description of Strata			
	()	%	From To	%	%	%	(mm)	Result		(m)		
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		(90)	4.90-6.20	100	94	76				Į.		\sim
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			-						Between 5.90m and 6.18m: Dark grey fine crystalline band.	ţ		\sim
			<u>-</u>						crystalline band.	Ł		\sim
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/02	3.70	0.90								6.20	5.90	\sim
			-						End of Borehole	ļ ***	""	
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Remarks (See notes & keysheets)

Scale 1:25

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SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No.

CON103001

Figure No.

FR6 (2 of 2)

Equipment

Dando 2000

Drill Fluid Drill Crew Massenza MI6

Borehole Diameter 200mm to 2.40m 131mm to 35.00m **Casing Diameter** 200mm to 2.40m 150mm to 6.20m

Coordinates (Local Grid) Ground Level

BOREHOLE No.

BH6 232138 E

709856 N 12.33 m OD

Drill Co Dates	rew Drilled	Start End	28/01/2010 23/02/2010	ged by 01/201		piled by 2/2010	Approved by 12/04/2010
		Water Depth	Sample/Core Recovery	SPT	Result or		
Date	Casing	(m)	Type No.	Blows /N	U100 Blows/		.

Date	Casing	Water Depth	Sample/0	_	, – –		SPT Blows	Result or U100		Depth		L
& ime	Depth (m)	(m) (Flush Return)	Depth (m)	Type	No. SCR	RQD	/N Core Size	Blows/ Rec. mm or Fracture	Description of Strata	(Thick- ness)	Level	Lege
	` ′	%	From To	%	%	%	(mm)	Index		(m)		7247
8/01			0.50-1.20) В	1				Grass over dark brown organic sandy gravelly TOPSOIL. MADE GROUND: Composed of brown organic sandy subangular to subrounded fine to coarse gravel of schist and quartz with many angular to subangular cobbles and occasional boulders. Sand is fine to coarse. Between 0.30m and 0.50m: Schist boulder. Decreasing organic content with depth.	(0.10) 0.10	12.23	
	1.10	0.90	1.20-1.65		2		C50/ 175			(2.30)		
			1.80	D	3					‡ - - - - -		
	1.90		2.00	С	4		C75/ 110*			 - -		
3/01	2.40	0.90	-							<u> </u>		
5/02	2.40	(0)	2.40	С			C75/ 30*	4	Medium strong to strong light grey and locally dark grey thinly foliated quartz mica SCHIST. Slightly to moderately weathered. Fractures are medium, locally closely spaced, inclined 30 degrees planar rough. Surfaces stained orange brown penetrating <pre>Smm. Fractures locally infilled with quartz mineralisation.</pre>	2.40	9.93	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
		(60)	2.40-4.00	75	50	44			Between 3.10m and 3.80m: Non intact. Locally reduced to weak. Surfaces stained dark brown.	(1.40)		<pre><pre></pre></pre>
								NI				·
									Strong fractured foliated light grey mica SCHIST interbedded with quartzite. Slightly weathered. Fractures medium	3.80	8.53	}
		(70)	4.00-4.75	5 100	100	100		5	spaced subhorizontal and subvertical planar rough moderately open tight grey sandy silt infill <2mm. Between 4.00m and 4.75m: Surfaces locally stained brown penetrating <2mm with clayey sand infill <3mm thick.	† • • • • •		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
										+ + + + + + +		
										<u> </u>		\sim

(See notes & keysheets)

Scale 1:25

depth and rescanned using the CAT to check for services. Services were not located.

Rotary coring was carried out using a core barrel 131mm in diameter.

The borehole was advanced by cable percussion means to 2.40m and then progressed by rotary coring means to 3 35.00m.

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 2.20m to 2.40m (60 mins). See installation details on final sheet.

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR7 (1 of 8)

Equipment

Dando 2000

Drill Fluid Drill Crew Dates Drilled Massenza MI6

28/01/2010 Start End 23/02/2010 **Borehole Diameter** 200mm to 2.40m 131mm to 35.00m

29/01/2010

Casing Diameter 200mm to 2.40m 150mm to 6.20m

12/04/2010

Coordinates

BOREHOLE No.

Local Grid)

Ground Level

232138 F 709856 Ν 12.33 m OD

BH₆

Compiled by Logged by Approved by

02/02/2010

Water Sample/Core Recovery SPT Depth Depth (m) Blows /N Result Date Casing No. Level Legend Type (Thickor Depth **Description of Strata** & Depth (m) (Flush Return) % ness) Core Size (mm) Fracture Time (m) TCR % SCR RQD Index From To % (m) 8 (70) 4.75-6.30 100 100 100 Between 5.60m and 6.30m: Fractures locally closely spaced subhorizontal and subvertical. Some to many randomly orientated quartz veins 5mm to 10mm 15/02 6.20 0.40 16/02 6.20 (100) 6.30-7.30 100 90 80 Between 7.20m and 7.40m: Non intact, probably drilling induced. ΝI Between 7.40m and 7.60m: Subvertical fracture along core axis open 10mm to 20mm and infilled with soft grey sandy clay <20mm. 16/02 6.20 0.20 8 17/02 6.20 (100) 7.30-8.80 100 100 66 (8.60) Below 8.80m: Fractures become medium to widely spaced subhorizontal and inclined 20 to 30 degrees planar rough. (100) 8.80-10.40 100 100 100 3

Remarks 7 Groundwater was not apparent during boring. (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (2 of 8)

Equipment

Drill Fluid Drill Crew Dando 2000 Massenza MI6

Start End **Dates Drilled** 28/01/2010 23/02/2010 **Borehole Diameter** 200mm to 2.40m 131mm to 35.00m

Casing Diameter 200mm to 2.40m 150mm to 6.20m

BOREHOLE No.

BH6

Coordinates (Local Grid) Ground Level 232138 E 709856 **N** 12.33 m OD

Logged by Compiled by Approved by 02/02/2010 12/04/2010 29/01/2010

		End Water	23/02/201		- P- ·			01/201		2/2010 12/04/2010			1
ate	Casing	Depth	Sample					SPT Blows			Depth	Lovel	1000
& ime	Depth (m)	(III) (Flush	Depth (m)	Type		RQD	/N Core	or Fracture	Description of Strata	(Thick- ness)	Level	Lege
iiie	(111)	Return)	From 1	ъ	%	%	%	Size (mm)	Index		(m)		
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			- -							Detroca 10 40m and 10 70m. Inclined	‡		\sim
			-					1		Between 10.40m and 10.70m: Inclined fracture 70 to 80 degrees infilled 20mm	İ		\sim
			_							to 30mm with soft grey sandy clay.	\vdash		~~
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			-							Strong thinly foliated light grey quartz	12.40	-0.07	\sim
		(50)		4.0	100	100	100			mica SCHIST. Slightly weathered.	ł		\sim
		(60)	11.80-13	.40	100	100	100		2	Fractures are generally medium spaced subhorizontal to inclined 30 degrees	Ŧ		~
			=							planar rough locally planar smooth.	Į.		~
]								Ŧ		\sim
			- -								‡		\sim
			-								F		~
			-							Between 13.15m and 14.00m: Subvertical	†		\sim
			-							fractures 80 to 90 degrees open 30mm to 50mm infilled with sandy clay and schist	ţ		\sim
			-							fragments. Some blue grey staining of	ł		~
02	6.20	0.50	-					ł		surfaces penetrating <2mm. Below 13.50m becomes closed and clean.	Ŧ		\sim
02	6.20		_								F		\sim
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		(100)	13.40-15	.00	100	100	100				Ŧ		\sim
		,									‡		~
			-							Between 14.40m and 14.60m: Ouartzite band	‡		\sim
			- -							closely fractured, probably drilling	t		\sim
			<u> </u>							induced.	\vdash		\sim
			-								Ŧ		\sim
			- -						6	At 14.70m: Inclined fracture 40 to 50	Ī		\sim
			-							degrees open 10mm to 20mm and infilled with firm sandy clay.	‡		~
			_							_ -	†		\sim
											ł		~
				ī							Г	Ī	1

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (3 of 8)

Equipment

Drill Fluid Drill Crew Dando 2000 Massenza MI6

Start End **Dates Drilled** 28/01/2010 23/02/2010 **Borehole Diameter** 200mm to 2.40m 131mm to 35.00m **Casing Diameter** 200mm to 2.40m 150mm to 6.20m

BOREHOLE No. Coordinates (Local Grid) Ground Level

BH6 232138 E 709856 **N** 12.33 m OD

Approved by 12/04/2010 Logged by Compiled by 29/01/2010 02/02/2010

		Water	Sai	mple/Co	re Roc	OVERV		SPT		•			
	Casing	Water Depth (m)	Jai	inpie/Co	Туре	_	l	SPT Blows /N	Result		Depth (Thick-	Level	l ege
& ime	Depth (m)	(Flush Return) %	Depth	(m)		SCR	ROD	Core	Fracture	Description of Strata	ness)	Levei	Lege
	()	Keturn) %	From	То	%	%	%	(mm)	Index		(m)		
			-								+		~~
											Ŧ		\sim
			Ī								Ŧ		~~
											‡		~~
											‡		\sim
			_								†		~~
			_								t		~
			_								Ł		\sim
		(100)	15.00	-16.60	100	100	100				F		\sim
			Ī								Ŧ		~
			_								‡		\sim
											‡		\sim
			-								‡		\sim
			-								ţ		~
			_								ł		\sim
			_						3		Ł		\sim
											F		Ň
											Ŧ		\sim
			-								‡		\sim
										At 16.88m: Fracture inclined 65 degrees	‡		~
										planar smooth moderately open grey silt	‡		\sim
			-							infill <2mm on subfracture surfaces.	<u> </u>		\sim
			_								ł		\sim
			Ī							At 17.22m: Induced fracture subhorizontal planar smooth moderately open clean.	Ŧ		\sim
			Ī							pranar smooth moderatery open tream.	Ŧ		\sim
		(100)	16.60	-18.10	100	100	100				‡		\sim
			_								ļ.		~
			-							At 17.62m: Induced fracture subhorizontal undulating smooth moderately open clean.	‡		\sim
										undulating photen moderatery open cream.	<u> </u>		\sim
			_								<u>†</u>		\sim
			<u> </u>							At 17.93m: Induced fracture inclined 60	ł		\sim
			Ī							degrees stepped smooth moderately open clean.	Ŧ		\sim
									4	Clean.	F		\sim
			-								‡		~
											‡		\sim
			_								‡		\sim
			<u> </u>								ţ		~
			<u> </u>								<u> </u>		\sim
			<u> </u>								ł		\sim
			_								F		\sim
			F _								F		\sim
		(90)	18.10	-19.70	100	100	100				Ŧ		\sim
			_								Ļ.		\sim
			- -								‡		~
			-								ţ		\sim
			-								t		\sim
			_								t		\sim
			_								F		\sim
			-								F		\sim
			-								Ŧ		\sim
											‡		\~`
			-								‡		\sim
			<u> </u>								ţ		\sim
			<u> </u>		I		1				⊢		

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (4 of 8)

Equipment

Dando 2000 Massenza MI6

Drill Fluid **Drill Crew Dates Drilled**

28/01/2010 23/02/2010 Start

Borehole Diameter 200mm to 2.40m 131mm to 35.00m

200mm to 2.40m 150mm to 6.20m

Casing Diameter

BOREHOLE No. Coordinates (Local Grid) Ground Level

BH6 232138 E

709856 N 12.33 m OD

Logged by Compiled by Approved by

		End	23/02/2010					0 02/0	2/2010 12/04/2010				
Date	Casing	Water Depth (m)	Sample/Co	_	_		SPT Blows /N	Result			Depth		
&	Depth	(m)	Depth (m)	Туре				or Fracture	Description of	f Strata	(Thick- ness)	Level	Legen
Time	(m)	(Flush Return) %	From To	TCR %	SCR %	RQD %	Core Size (mm)	Index			(m)		
		/6		 	-/-		()			-	(''')		~~~
			-							‡			~~~
			- =							‡	-		~~~
			-							†			$\sim\sim$
		(90)	19.70-21.25	100	100	95				•			~~~
			-							•			~~~
			-							Ī			~~~
										H	-		~~~
													~~~
			- -							-			~~~
			- -										~~~
8/02	6.20	1.00	-							‡			~~~
9/02	6.20		-				1			<b>†</b>	-		~~~
			<del>-</del>							‡ •			~~~
			<u>-</u>							<b>‡</b>	· —		~~~
			-							‡	•		$\sim\sim$
			-							<b>†</b>			$\sim\sim$
			-							-			~~~
										I			~~~
		(90)	21.25-22.80	100	100	100		0		_	_		~~~
			-							<u> </u>			~~~
			- -								-		~~~
			-							‡			~~~
			-							•			~~~
			<del>-</del> -							<b>†</b>			~~~
			-							‡			$\sim\sim$
			<del>-</del>							‡	-		~~~
			-							<b>†</b>			~~~
			_							Ī			~~~
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										1			~~~
			<del>-</del> -							_	-		~~~
			- -										~~~
			- —							<b>†</b>	<del>-</del>		~~~
		(90)	22.80-24.35	100	100	100				‡ •			~~~
			-						At 23.76m: Induced fract		(22.60)		~~~
			-						degrees planar smooth mo		-		$\sim\sim$
			-						clean.	‡	•		~~~
			_						At 24.00m: Induced fract degrees undulating plans	ture inclined 15	_		~~~
			_	1					moderately open clean.	†			~~~
			- - -						At 24.14m: Induced fract degrees and 20 degrees p	ture inclined 40 planar smooth			~~~
			- -	1	1				tight clean.	ļ			~~~
			-	1						‡			~~~
			-							ļ	-		~~~
			-	1						‡			~~~
			<u>-</u>	1					Potroon 04 05 3 04 50	m. Emaghirin	-		~~~
			-						Between 24.85m and 24.70 inclined 75 degrees plan				
			-	1					moderately open clean. At 24.85m: Induced fract	t t			$\sim$
									degrees stepped smooth	open clean.	_		

Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (5 of 8)

Equipment

Dando 2000

**Drill Fluid Drill Crew Dates Drilled**  Massenza MI6

Start End 28/01/2010 23/02/2010 **Borehole Diameter** 200mm to 2.40m 131mm to 35.00m **Casing Diameter** 200mm to 2.40m 150mm to 6.20m

**BOREHOLE No.** Coordinates (Local Grid) Ground Level

232138 E 709856 **N** 12.33 m OD

BH₆

Logged by Compiled by Approved by 02/02/2010 29/01/2010 12/04/2010

		Ena	23/02/				237		.0 02/0	2/2010 12/04/2010			1
D-1-	Q	Water Depth	Sai	mple/Co	re Rec	overy		SPT	Result		Depth		
	Casing	(m)			Туре	No.		SPT Blows /N	or		(Thick-	Level	Legend
&	Depth		Depth	ı (m)				Core	Fracture	Description of Strata	ness)		
Time	(m)	(Flush Return)	F	<b>T</b> -	TCR	SCR	RQD	Core Size (mm)	Index		, ·		
		% ′	From	То	%	%	%	(mm)			(m)		, , , ,
			-								ŧ		~~~
		(90)	24.35	-25.95	100	96	38				Į		$\sim\sim$
			_							Between 25.30m and 25.70m: Very closely	ŧ		$\sim\sim$
			Γ							spaced fracture inclined 70 degrees	Į.		~~~
			_							planar smooth moderately open to tight	ŧ		^^~
			Į.							locally infilled with grey angular gravel	Į		~~~
			_							of schist <10mm.	<u> </u>		~~~
											ł		$\sim\sim$
											t		~~~
			ļ.								<del> </del>		$\sim\sim$
										At 25.87m: Induced fracture inclined 60 degrees undulating rough moderately open	1		~~~
										clean.	ł		$\sim\sim$
			L						AZCL	0104117	Į.		~~~~
			_								t		$\sim\sim$
											Ŧ		$\sim\sim$
			<u>L</u>								<u>t</u>		~~~
			ļ								ł		$\sim\sim$
			Ī								Į		~~~
1			<u> </u>								t		~~~
											<b>F</b>		~~~
			_							Between 26.65m and 26.70m: Quartzite	t		~~~
			[		١					vein.	Į		$\sim\sim$
		(90)	25.95	-27.55	97	97	73			Between 26.72m and 26.89m: Very closely spaced induced fractures inclined 60	t		~~~
										degrees planar rough tight clean.	ł		$\sim\sim$
									0		‡		~~~
			_								⊢		~~~
											Į		~~~
			_								ŧ		~~~
			<u>L</u>								Į.		$\sim\sim$
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									-		ł		$\sim\sim$
			_								Γ		~~~
											ł		~~~
1			Ī								Ī		~~~~
		(85)	27 55	-29.10	100	100	88		3		t		~~~
		(05)	47.33	-23.10	1 -00	1.00	30		3		ł		~~~
			_								‡		~~~
1			H								H		$\sim\sim$
			_								‡		~~~
			_								t		~~~
			Ī.								Į.		~~~~~
19/02	6.20	1.00	_								t		~~~
22/02	6.20		L							At 28.99m: Induced fracture inclined 40	ł		~~~~
22/02	0.20		L							degrees undulating rough tight clean.	L		~~~
1			ŀ							At 29.04m: Induced fracture inclined 35	ł		$\sim\sim$
1										degrees planar smooth tight clean.	ţ		~~~~
1			ŀ								ł		~~~
			Γ						0		Ţ		~~~~
			_								t		~~~
											Į		$\sim\sim$
			H							At 29.60m: Induced fracture inclined 20	H		~~~
			Ī							degrees planar rough tight clean.	Ŧ		~~~
			ļ.								t		~~~~
			<u> </u>							At 29.79m: Induced fracture inclined 10	+		~~~
		(80)	20 10	-30.70	100	100	60			degrees undulating smooth moderately open clean.	‡		~~~
1		(00)	29.10	50.70	1-00	1.00	00			At 29.85m: Induced fracture inclined 70	ł		~~~
			L							degrees undulating smooth tight to	L		$\sim\sim$
I					I	l		ĺ		moderately open clean.	I		1

Remarks

(See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (6 of 8)

Equipment

**Drill Fluid Drill Crew Dates Drilled**  Dando 2000 Massenza MI6

Start End 28/01/2010 23/02/2010 **Borehole Diameter** 

Logged by

200mm to 2.40m 131mm to 35.00m

Compiled by

**Casing Diameter** 200mm to 2.40m 150mm to 6.20m **BOREHOLE No.** 

BH6 232138 E

Approved by

Coordinates (Local Grid) Ground Level 709856 N 12.33 m OD

Dates	Drilled		28/01/2					gea by		plied by Approved by				
		End Water	23/02/2 Sar	2010 nple/Co	re Rec	overv	29/	SPT	0 02/0	2/2010 12/04/2010				
	Casing	Depth (m)		•	Туре		1	Blows /N	Result or			Depth (Thick-	Level	Legen
& -:	Depth	(Flush	Depth	(m)				Core	Fracture	Description of	of Strata	ness)	Love	Logoni
Time	(m)	Return)	From	То	W	SCR %	RQD %	Size (mm)	Index			(m)		
														~~~
			ļ									‡		~~~
			-							At 30.26m: Fracture inc	clined 30 degrees	‡		~~~
			-							stepped smooth open cle	ean.	ţ		$\sim\sim$
			_									ŧ		~~~
			F									F		~~~
												ł		~~~
		-	-					-				F		$\sim\sim$
			Ē							Between 30.86m and 31.0		F		~~~
			-							inclined 70 degrees ste open 10 to 20mm.	epped smooth very	‡		~~~
			_							open 10 to zomm.		Ļ		$\sim\sim$
			-									‡		~~~
			<u> </u>									‡		~~~
			-									ţ .		\~~~
			<u> </u>									ţ		$\sim\sim$
		(80)	30.70	-32.30	100	100	77					Ł		~~~
		(00)	- 50070	52.55						At 31.59m: Induced frac		F		~~~
			-							degrees clean stepped sopen clean.	smooth moderately	Ŧ		~~~
			F						2	At 31.74m: Induced frac		F		$\sim\sim$
			F							degrees undulating smoo	oth moderately open	Į.		~~~
			-									‡		~~~
			-									ļ		~~~
			ļ.									‡		$\sim\sim$
22/02	6.20	1.00	-									‡		~~~
23/02	6.20							-		At 32.33m: Fracture inc stepped rough moderatel		ţ		~~~
23/02	0.20		_									ŧ		$\sim\sim$
			_							Between 32.55m and 32.7 fracture inclined 70 de		F		~~~
										striated moderately ope	en to very open	ŧ		~~~
			L							quartzite on fracture s	surfaces.	Ł		~~~
			-									F		~~~
			-									Ŧ		$\sim\sim$
			-									ļ.		$\sim\sim$
		(80)	32.30	-33.90	100	100	76					‡		~~~
			-							35 33 05m Taland Suc		‡		~~~
			-							At 33.25m: Induced fraction degrees stepped striate		‡		$\sim\sim$
			<u> </u>							At 33.42m: Induced frac	cture inclined 25	‡		~~~
			Ė							degrees stepped striate clean.		L		
			<u> </u>							Between 33.55m and 33.6 inclined 40 to 60 degree		‡		\sim
			<u> </u>							open clean.		‡		~~~
			-							At 33.65m: Fracture inc planar smooth moderatel		ţ .		~~~
			-]		clean.		ţ		~~~
			L							At 34.05m: Induced frac	cture inclined 20	Ł		~~~
			<u> </u>							degrees undulating stri		t		~~~
			Ł									t		~~~
			ŀ						1			t		~~~
									_			Ŧ		~~~
		(80)	33.90	-35.00	100	95	91					F		\sim
			F									F		~~~
			-							At 34.70m: Induced frac	cture inclined 30	Ŧ		~~~
			-							degrees undulating smoo		‡		~~~
			-									‡		~~~
23/02	6.20	1.00	ļ									35.00	-22.67	~~~
								<u></u>		End of Bor	rehole		22.0/	
Remar	ks													•

(See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR7 (7 of 8)

Drilling Method Cable Percussion & Rotary **BOREHOLE No. Borehole Diameter** BH₆ 200mm to 2.40m Coordinates (Local Grid) Ground Level 131mm to 35.00m Equipment Dando 2000 232138 E Massenza MI6 709856 **N** 12.33 m OD **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** 28/01/2010 Start 29/01/2010 02/02/2010 12/04/2010 End 23/02/2010 Installation Water Strata Level **Installation Details** Strata Details m OD Strikes Depth (m) Depth (m) Bentonite Grout Coarse/Medium grained Instrumentation: Metamorphic (SCHIST) 19mm standpipe piezometer tip at 19.50m 9.00 3.33 Bentonite Seal 10.00 2.33 Sand Filter 20.00 -7.67 Bentonite Seal 21.00 -8.67 Bentonite Grout 35.00 -22.67 35.00 Base of Hole Remarks (See notes & keysheets) Water Strike ▼ Water Rise Upstanding cover. Pipe diameter 19mm to 20.00m. Project Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR7 (8 of 8)

Equipment Dando 2000

Drill Fluid Drill Crew Dates Drilled Knebel

Start 25/01/2010 End 25/01/2010 **Borehole Diameter** 200mm to 1.20m 150mm to 8.00m

Casing Diameter

200mm to 1.20m

Coordinates (National Grid) Ground Level

BOREHOLE No.

232148 709867 N

BH7

13.66 m OD

Logged by	Compiled by	Approved by
02/02/2010	25/02/2010	12/04/2010

		End Water	25/01/2010 Sample/Core Recovery					02/201 SPT		2/2010 12/04/2010	Τ_		T
Date	Casing	Depth	Sample/Co			<u> </u>		Blows			Depth	Lovol	Logo
& ime	Depth (m)	(m) (Flush Return)	Depth	` ,	Type TCR	SCR	RQD		or Fracture Index	Description of Strata	(Thick- ness)	Level	Lege
- /05		% ′	From	То	%	%	%	(mm)		AND COORD TO	(m)		XXX
5/01										MADE GROUND: Tarmacadam.	(0.10) 0.10	13.56	
			_							MADE GROUND: Composed of grey angular coarse gravel of dolomite.	(0.20)		
			0.30-	-0.60	В	1					0.30	13.36	
			-							MADE GROUND: Composed of grey roadstone (Driller's description).	(0.10) 0.40	13.26	
			F							MADE GROUND: Composed of grey and brown	(0.20)		
			0.60-	-0.90	В	2				sandy gravelly angular cobbles of mica	0.60	13.06	
	(0)		 							\ schist. Gravel is angular to subangular fine to coarse mica schist. Sand is	' ‡		
		(0)	-							medium to coarse.	(0.60)		
										MADE GROUND: Composed of grey brown clayey sandy angular and subangular fine	Ŧ `		
			<u> </u>							to coarse gravel of schist. Sand is fine	F		
			<u>[</u>							to coarse.	1.20	12.46	
			-							MADE GROUND: Composed of brown and grey brown silty sandy angular to subrounded	ŀ		
			0.00-2.8	2 00	91	82	fine to coarse gravel of schist and quartz with many subangular cobbles an occasional boulders >200mm. Sand is fi			fine to coarse gravel of schist and	(0.50)		
			- 0.00-	-2.60	91	02		occasional boulders >200mm. Sand is fine	(0.30)				
			Ē							to coarse.	ŧ		
									-	Medium strong fractured thinly foliated	1.70	11.96	
			-							grey and locally grey brown quartz mica	ţ		\sim
			-							SCHIST. Slightly to moderately weathered. Fractures are closely to medium spaced	ţ		\sim
			-							inclined 20 to 30 degrees planar rough. Surfaces stained dark brown and orange	F		\sim
			-							brown penetrating 10mm to 20mm.	ţ		\sim
		(90)	<u>-</u>						_		(1.10)		\sim
			-						6		Ŧ		\sim
			_								Ł		\sim
			-								F		\sim
			-							At 2.70m: Inclined fracture surfaces	ţ		\sim
			-							stained dark grey brown penetrating 60mm to 80mm.	2.80	10.86	\simeq
			- -							Strong locally medium strong fractured	‡		\sim
			_							foliated light grey quartz mica SCHIST. Slightly weathered. Fractures are medium	F		\sim
			[spaced inclined 20 to 30 degrees planar	E		\sim
			_							rough. Surfaces locally stained light brown penetrating <5mm.	Ł		\sim
			ţ								ţ		\sim
			ļ								‡		\sim
		(90)	2.80-	-4.30	100	100	93				F		\sim
											E		\sim
			<u> </u>								t		\sim
			<u>-</u>								ţ		\sim
			<u>-</u>								L		\sim
			-								ţ		\sim
			-								Ŧ		\sim
		(00)	4 32	4 50	100	100	100	ł			F		\sim
		(90)	4.30-	-4.50	100	100	100				E		\sim
			_					1			F		\sim
			-								ţ		\sim
			<u>-</u>								‡		\sim
			<u> </u>								‡		\sim
			-							Below 5.00m: With occasional irregular	ţ		\sim
			F				l			quartzite bands 20mm to 50mm.	T .	Ī	

(See notes & keysheets)

Scale 1:25

- Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection depth and rescanned using the CAT to check for services. Services were not located.
- The borehole was advanced by chiselling methods from 1.60m to 1.70m for one hour.
- Rotary coring was carried out using a core barrel 131mm in diameter.

 The borehole was advanced by cable percussion means to 1.70m and then progressed by rotary coring means to 4
 - An amount of water was added to facilitate boring in granular strata. Groundwater was not apparent during boring.

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR8 (1 of 2)

Equipment Dando 2000 Knebel

Drill Fluid Drill Crew Dates Drilled

Start

Borehole Diameter 200mm to 1.20m 150mm to 8.00m

Casing Diameter 200mm to 1.20m

BOREHOLE No.

Coordinates (National Grid) Ground Level

232148 E 709867 N 13.66 m OD

BH7

Logged by Compiled by Approved by

		End	25/01/2010					LO 25/0				
Data	Cooler	Water Depth (m)	Sample/Co	_	_		SPT Blows /N	Result		Depth		
Date &	Casing Depth	(m)	Depth (m)	Туре	No.		/N		Description of Strata	(Thick-	Level	Legend
Time	(m)	(Flush Return) %		TCR	SCR %	RQD	Core Size (mm)	Fracture Index	Description of Strata	`ness)		
	` ,	%	From To	%	%	%	(mm)	muex		(m)		
			-							t		~~~
			-							t		~~~
		(90)	4.50-6.00	100	100	93				t		~~~
			-							(5.20)		~~~
			<u>.</u>					4		Ŀ		~~~
			_							t		~~~
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			-							‡		$\sim\sim$
			<u> </u>							‡		$\sim\sim$
									Below 6.00m: Becoming green grey.	<u>L</u>		~~~
			ţ							ţ		~~~
										‡		~~~
			-							‡		~~~~
			ţ							ţ		$\sim\sim$
		(90)	<u>-</u>							L		$\sim\sim$
			-						Between 6.65m and 6.75m: Quartz band.	‡		~~~
			ţ						between 0.05m and 0.75m. Quartz band.	‡		~~~
			- 6.00-7.50	100	100	100				‡		~~~
			<u>-</u>							†		~~~~
			<u>-</u>							<u>L</u>		$\sim\sim$
			-							‡		~~~
			- -							†		$\sim\sim$
			- -							‡		~~~
			- -							†		~~~
		(90)	-							‡		~~~
			-							‡		~~~~
			7.50-8.00	100	100	100				‡		$\sim\sim$
			7.30-8.00	100	100	100				Ŧ		$\sim\sim$
25/01		DRY	-							Ŧ		$\sim\sim$
			<u>-</u>						Pud of Pourhals	8.00	5.66	~~~
			-						End of Borehole	‡		
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Remarks (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR8 (2 of 2)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No.** BH8 200mm to 1.60m 200mm to 1.60m Equipment Dando 2000 150mm to 4.50m 150mm to 2.00m Coordinates 232124 F Knebel (National Grid) 709876 N **Drill Fluid** 15.14 m OD Ground Level Water **Drill Crew** Logged by Compiled by Approved by 29/01/2010 **Dates Drilled** Start End 04/02/2010 12/04/2010 12/02/2010 02/02/2010 Water Sample/Core Recovery SPT Depth Depth (m) Blows /N Date Casing No. Level Legend Type (Thick-Depth & Depth (m) **Description of Strata** ness) (Flush Return) % Core Size (mm) Time TCR % SCR RQD (m) From To % (m) 29/01 0.00 DRY MADE GROUND: Tarmacadam (0.10) 15.04 0.10 MADE GROUND: Composed of medium dense grey becoming brown clayey sandy gravelly angular cobbles of mica schist with occasional angular boulders. Gravel is angular to subangular fine to coarse 0.50-0.80 в 1 schist. Sand is medium to coarse. (0.90)0.90-1.20 2 в 1.00 14.14 MADE GROUND: Composed of light brown silty fine to coarse sand and subangular to subrounded fine to coarse gravel of schist and quartz with occasional angular (0.30) DRY 1.20 C C50/ 1.30 13.84 160 to subangular cobbles. 1.20-1.60 3 MADE GROUND: Composed of grey sandy quartz mica schist boulders (Driller's 29/01 1.60 DRY description). C 1.60 11/02 1.60 DRY 30* 1.60-3.10 NA NΑ NA (3.10)3.10-4.50 NA NA NA 11/02 2.00 DRY 4.40 10.74 Possibly quartz mica SCHIST (Driller's Description). (0.10) 12/02 2.00 DRY 4.50 10.64 12/02 DRY End of Borehole Remarks 1 Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m (See notes & keysheets) depth and rescanned using the CAT to check for services. Services were not located. The borehole was advanced by rotary open hole means from 1.60m to 4.50m. The borehole was advanced by cable percussion means to 1.60m and then progressed by rotary open hole means to 4.50m. An amount of water was added to facilitate boring in granular strata. Borehole advanced by chiselling using CH from 1.50m to 1.60m (60 mins). Groundwater was not apparent during boring. Scale 1:25 Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR9 (1 of 1) 308/03

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No.** BH8A 131mm to 8.50m 150mm to 5.30m Equipment Dando 2000 Coordinates 232127 F Knebel Local Grid) 709877 Ν **Drill Fluid** Ground Level 15.16 m OD Water **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** Start 24/02/2010 01/03/2010 04/03/2010 12/04/2010 End 24/02/2010 Water Sample/Core Recovery SPT Depth Depth (m) Blows /N racture Date Casing No. Level Legend Type Index (Thick-Depth **Description of Strata** & Depth (m) (Flush Return) % ness) Core Size (mm) or Time (m) TCR % SCR % RQD % Result From To (m) 0.10 24/02 MADE GROUND: Tarmacadam. 15.06 MADE GROUND: Composed of cobbles and boulders (Drillers description). (5.40)

Remarks 1 (See notes & keysheets)

- Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.
- On completion the borehole was grouted up to ground level.
- Rotary coring was carried out using symmetrix drilling techniques.

 The borehole was advanced by rotary open hole means to 5.50m and progressed by rotary coring means to 8.50m. Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR10 (1 of 2)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No.** BH8A 131mm to 8.50m 150mm to 5.30m Equipment Dando 2000 Coordinates 232127 F Knebel Local Grid) 709877 N **Drill Fluid** 15.16 m OD Water Ground Level **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** 24/02/2010 Start 12/04/2010 End 24/02/2010 01/03/2010 04/03/2010 Water Sample/Core Recovery SPT Depth Depth (m) Blows /N racture Date Casing No. Level Legend Type (Thick-Index Depth & Depth (m) **Description of Strata** (Flush Return) % ness) or Core Size (mm) Time (m) TCR % SCR RQD Result From To % (m) 5.50 9.66 Medium strong fractured thinly foliated Medium strong fractured thinly foliated light grey green, grey and dark grey quartz mica SCHIST. Fractures are subhorizontal to subvertical very closely to closely spaced planar rough and infilled with a fine gravel of schist with a greenish clay matrix.

Between 5.50m and 5.75m: Fracture inclined 70 degrees undulating rough open clean some quartz vening penetrating AZCL clean some quartz veining penetrating <10mm At 5.82m: Induced fracture horizontal (100) undulating rough tight clean.
At 6.04m: Closely spaced fractures inclined 30 to 40 degrees planar smooth 5.50-7.10 75 69 11 6 (3.00) Between 7.10m and 7.70m: Closely spaced fractures inclined 70 degrees undulating slightly rough some gravel infill of schist. Between 7.72m and 8.10m: Fracture inclined 85 degrees planar slightly rough very open with angular fine gravel infill (100) 7.10-8.50 100 92 34 of schist in a silty matrix. 1 At 8.34m: Induced fracture inclined 15 degrees undulating striated tight clean. Between 8.44m and 8.59m: Fracture inclined 85 degrees and 45 degrees

Remarks (See notes & keysheets)

24/02

5.30

1.25

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

stepped rough open clean.

End of Borehole

Contract No. CON103001

Figure No.

FR10 (2 of 2)

8.50

6.66

Equipment Dando 2000

Drill Fluid Drill Crew Dates Drilled Knebel

Start 01/02/2010 End 04/02/2010 **Borehole Diameter**

01/02/2010

200mm to 3.60m 150mm to 7.50m

04/02/2010

Casing Diameter 200mm to 3.60m

12/04/2010

BOREHOLE No. Coordinates (Local Grid) Ground Level

232103 709874 **N** 15.66 m OD

BH9

Compiled by Approved by Logged by

Sacina	Water Depth			re Rec	overy		SPT	Result or		Depth		
Danth	(ṁ)	Depth	(m)			ROD	/N	Blows/ Rec. mm or	Description of Strata	(Thick- ness)	Level	Legen
(,	%	From	То	%	%	%	(mm)	Index		(m)		
		- -							MADE GROUND: Tarmacadam.	(0.12)	15.54	
		- - - -							MADE GROUND: Composed of dark grey slightly sandy gravelly angular cobbles of dolerite. Sand is coarse.	(0.28)	13.34	
		0.50	-0.80	В	1				MADE GROUND: Composed of light brown silty sandy gravelly angular cobbles and	0.40	15.26	
		- - -							boulders of mica schist. Gravel is angular to subangular fine to coarse mica schist. Sand is medium to coarse.	(0.40)	14 06	
		- - - - -							MADE GROUND: Composed of dense brown grey sandy angular fine to coarse gravel of mica schist and quartz with many angular cobbles. Sand is medium to coarse.		14.00	
D	DRY			C B	2					<u> </u>		
		- - - - -								<u> </u>		
		- - - -								(2.00)		
1.90		2.00		D C B	3 4		C33					
		- - - -								<u> </u>		
		- - - -								<u> </u>		
		2.80		D	5				MADE GROUND: Composed of very dense brown	2.80	12.86	
2.90		3.00	-3.50	C B	6		C50/ 190		gravelly medium to coarse micaceous sand with frequent cobbles.	<u> </u>		
		- - - -	3.30	2	ŭ					(0.80)		
3.50	DRY	- - - - 3 60	2 66				C7E /			3 60	12.06	
3.50	(0) (0)	-	3.00				55*		Possibly MADE GROUND: Composed of dark grey angular to subangular cobbles and boulders of quartz mica schist.	3.00	12.00	
Ñ		3.90	-3.90	73	60	0				<u> </u>		
	(10)	3.90	-4.50	63	63	0				<u> </u>		
4.50	3.00	- - -								Ī.		
4.50		-								F		
		-								‡		
	Į	- - -								F		
	ŀ	-				Ī				(2.70)		
	(m) 1.90 2.90 3.50 3.50	Depth (Flush Return) % DRY DRY 1.90 DRY 3.50 DRY (10) (10)	Depth (m) Return % Depth (Flush Return) % From DRY 1.20 1.90	DRY - 1.20-1.65 1.20-1.70 DRY - 1.20-1.65 1.20-1.70 - 1.90 - 2.00-2.45 2.00-2.50 3.50 DRY - 3.60-3.66 (0) (0) - 3.90-3.90 (10) 3.90-3.90 4.50 3.00	DRY - 1.20-1.65 C 1.20-1.70 B 1.90 - 2.00-2.45 C 2.00-2.50 B 2.90 - 3.00 C 3.00-3.50 B 3.50 DRY - 3.60-3.66 (0) (0) (0) - 3.90-3.90 73 (10) 3.90-4.50 63	Depth (m) (Flush (m) (m) (Flush (m) (Flush (m) (Flush (m) (m) (Flush (m) (m) (m) (m) (m) (m) (m) (m) (m) (m)	Depth (m) (m) (Flush (Depth (m) Peth (m) From To Type No. Core Size (mm)	Depth Depth Depth Depth	Depth (m) Dept	Description of Strata Desc	Depth Post Depth Post Depth
Remarks 1 (See notes & keysheets)

Scale 1:25

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.

Rotary coring was carried out using Geobore drilling techniques.

The borehole was advanced by cable percussion means to 3.60m and then progressed by rotary coring means to 3 7.20m.

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 2.40m to 2.70m (45 mins); from 2.80m to 2.90m (45 mins); from 3.50m to 3.60m (60 mins).



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR11 (1 of 2)

Equipment

Dando 2000

Drill Fluid Drill Crew Dates Drilled Knebel

Start End 01/02/2010 04/02/2010 **Borehole Diameter** 200mm to 3.60m 150mm to 7.50m

Casing Diameter 200mm to 3.60m **BOREHOLE No.**

Coordinates (Local Grid) Ground Level

232103 E 709874 **N** 15.66 m OD

BH9

Logged by 01/02/2010

Compiled by Approved by 04/02/2010 12/04/2010

		End	04/02/2010				02/201							
Date	Casing	Water Depth (m)	Sample/Core Recover				SPT Blows /N	Result		Depth				
& Dept	Depth	(m)	Depth (m) Type No.	İ		or Fracture	Description of Strata	(Thick- ness)	Level	Legend				
Time	(m)	(Flush Return) %	From To	TCR %	SCR %	RQD %	Core Size (mm)	Index		(m)				
		(0)	4.50-6.00	27	0	0				-				
									Strong fractured dark grey and light grey quartz mica SCHIST. Moderately weathered. Fractures are closely spaced	6.30	9.36			
		(0)	6.00-7.50	100	83	0		>20	subhorizontal planar smooth clean with orange staining on surfaces.	(1.20)				
04/02	7.50	DRY	- - - - -						End of Borehole	- - 7.50	8.16			
			_											
			- - -											
			-							I. I. I. I. I. I.				
			- - - -											
			_							<u> </u>				

Remarks 6 Groundwater was not apparent during boring. (See notes & keysheets)

Scale 1:25

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR11 (2 of 2)

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No. BH10** 200mm to 4.40m 200mm to 4.30m Coordinates (Local Grid) Ground Level Equipment 150mm to 7.30m 150mm to 5.86m 232087 Dando 2000 F Massenza MI6 709872 N **Drill Fluid** 16.16 m OD Water

Compiled by

Logged by

Approved by

Drill Crew Dates Drilled		Start	20/01/2010			Log	ged by	Com	piled by	Approved by				
		End Water	21/01/2010	D				0 22/0 Result or	1/2010	12/04/2010		1		
Date & Time	Casing Depth (m)	Depth (m)	Sample/Co	Туре	No.		SPT Blows /N Core Size	U100 Blows/ Rec. mm or		Description of	of Strata	Depth (Thick- ness)	Level	Legen
1 11110	(,	Return)	From To	%	%	1%	(mm)	Fracture Index				(m)		
20/01			<u> </u>						MADE GR	OUND: Tarmacadam	n.	(0.10) 0.10	16.06	
			ļ.							OUND: Composed of	of dark grey cobbles of schist.	1 3323		
			E						angurar	to subangular c	comples of schist.	(0.40)		
			<u> -</u>										15 66	
									brown be coarse fine to occasion	ecoming dark bro sand and subangu	ular to subrounded of mica schist with cobbles.	- 0.50	15.66	
			<u> -</u> [F		
			[Ŧ		
	1.90	DRY	- 1.20 - 1.20-1.65	B	1 2		S10					‡		
			1.20-1.70	В	3							Ē		
			<u> </u>									<u> </u>		
			Ē									ŧ		
			Ę.									F		
			- -									‡		
	1.90	DRY	2.00-2.45	С			C40/ 245					F		
			2.00	D B	4 5		243					ţ		
			2.00-2.50	В	5									
			<u> </u>									(3.70)		
			_									L		
			<u> </u>									ŧ		
			[E		
			2.80	D	6							‡		
	2.90	DRY	3.00	С			C50/					Ŧ		
			3.00-3.50	В	7		125					F		
			3.00-3.30	_	′							‡		
			Ē									ŧ		
	2 40			_			-40 /					‡		
	3.40	DRY	_ 3.50-3.95	С	_		C48/ 235					F		
			3.50-4.00	В	8							<u> </u>		
			<u> -</u>									ţ		
			Ė									ŧ		
			<u></u>									 		
			ļ.									4.20	11.96	
			4.20	D	9					y medium strong recovered as ang		(0.20)		~~~
20/01	4.30	DRY	<u>.</u>							gravel sized fra		4.40	11.76	~~~
21/01	4.30		4.40	С			C50/			y medium strong		± ••••	11./6	~~~
			<u> </u>				115		SCHIST	recovered as ang	jurar CODDIES.	E		
			<u> </u>									ţ		
			- -									‡		~~~
			<u>E</u>									E		~~~
												<u> </u>		

Remarks 1 (See notes & keysheets)

Scale 1:25

Drill Crew

Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.

On completion the borehole was grouted up to ground level.

The borehole was advanced by cable percussive drilling to 4.40m and progressed with rotary coring to 7.30m. Rotary coring was carried out using a core barrel 140mm in diameter.

An amount of water was added to facilitate boring in granular strata.

Borehole advanced by chiselling from 2.40m to 2.80m (30 mins); from 3.20m to 3.50m (45 mins); from 4.20m to 4.40m (45 mins).



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR12 (1 of 2)

Equipment Dando 2000

Drill Fluid

Massenza MI6

20/01/2010

Drill Crew

Water

Start End **Dates Drilled**

Borehole Diameter 200mm to 4.40m 150mm to 7.30m

Casing Diameter

200mm to 4.30m 150mm to 5.86m

BOREHOLE No. Coordinates (Local Grid) Ground Level

232087 E 709872 N 16.16 m OD

BH10

Logged by Compiled by Approved by 21/01/2010 22/01/2010

12/04/2010

Dates	Drilled	Start End	20/01/2010 21/01/2010			21/	01/201	.0 22/0	1/2010 12/04/2010			
Date	Casing	Water Depth	Sample/Co	_	_		SPT Blows	Result	•	Depth		
&	Depth	(m)	Depth (m)	Туре		PO.5	Blows /N Core	or Fracture	Description of Strata	(Thick- ness)	Level	Legend
Time	(m)	(Flush Return) %	From To	%	SCR %	%	Core Size (mm)	Index		(m)		
		(0)	4.40-5.80	NA	NA	NA				(1.40)		
												~~~
			<b>-</b> -							‡		~~~
			[ <del>-</del>							<u> </u>		
			-							ŧ		~~~
			-  -							ţ		~~~
									Weak to medium strong grey fractured and	5.80	10.36	
			<u>.</u> —						foliated quartz mica SCHIST. Moderately to slightly weathered. Fractures are	Ļ		~~~
			<u> </u>						subvertical to vertical closely spaced undulating rough and predominantly	Ŧ		~~~
			- -						infilled with gravel in a clayey matrix. Surfaces locally stained orange brown and	‡		~~~
			-						cemented with quartz.	Į.		~~~
		(100)	- 5.80-7.30	100	97	20		6		(1.50)		~~~
		(100)	3.80-7.30	100	31	20		o o		(1.30)		~~~
			<u>-</u> -							Ŧ.		$\sim\sim$
			- -							ŧ		~~~
			_							F		~~~
			<del>-</del>							ŧ		~~~
21/01	5.86	DRY	-							7.30	8.86	~~~~
			-						End of Borehole	ŧ		
			<u> </u>							F		
			-							Ŧ.		
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Remarks 7 (See notes & keysheets)

Groundwater was not apparent during boring.

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR12 (2 of 2)

200mm to 3.60m 200mm to 3.60m Equipment Dando 2000 131mm to 10.00m 151mm to 7.00m Coordinates 232170 F Massenza MI6 Local Grid) 709841 Ν **Drill Fluid** Ground Level 12.05 m OD Water **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** 01/02/2010 Start 04/02/2010 End 01/03/2010 02/02/2010 12/04/2010 Water Sample/Core Recovery SPT Result or Depth U100 Blows/ Rec. mm or Depth (m) Blows /N Date Casing No. Level Legend Type (Thick-Depth & Depth (m) **Description of Strata** (Flush Return) % ness) Core Size (mm) Time TCR % SCR RQD or Fracture Index (m) From To % (m) 01/02 Grass over gravelly TOPSOIL with many (0.10) 11.95 rootlets. 0.10 MADE GROUND: Composed of grey brown silty sandy gravelly angular cobbles of mica schist with occasional angular boulders. Gravel is angular to subangular fine to 0.50-1.00 в 1 coarse mica schist. Sand is fine to coarse. (1.30)01/02 1.20 DRY 02/02 1.20 10.65 1.40 MADE GROUND: Composed of brown becoming grey silty sandy angular to subangular fine to coarse gravel of mica schist with many subangular cobbles and occasional boulders. Sand is medium to coarse. 2.00 в (2.10)3.00 в 3 3.50 8.55 Possibly MADE GROUND: Composed of gravel, cobbles and boulders (Driller's description). 02/02 3.60 DRY 01/03 3.60 (2.90) Remarks 1 Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m (See notes & keysheets) depth and rescanned using the CAT to check for services. Services were not located. On completion the borehole was grouted up to ground level. Rotary coring was carried out using symmetrex coring techniques The borehole was advanced by cable percussion means to 3.60m and then progressed by rotary open hole means to 7.00m and rotary coring to 10.00m. An amount of water was added to facilitate boring in granular strata. Groundwater was encountered at 5.20m during boring. Scale 1:25 Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR13 (1 of 2) 308/03

Drilling Method Cable Percussion & Rotary

**Borehole Diameter** 

**Casing Diameter** 

**BOREHOLE No.** 

**BH11** 

Drilling Method Cable Percussion & Rotary **Borehole Diameter Casing Diameter BOREHOLE No. BH11** 200mm to 3.60m 151mm to 7.00m 200mm to 3.60m Coordinates (Local Grid) Ground Level Equipment 232170 **E** Dando 2000 131mm to 10.00m Massenza MI6 709841 **N** Drill Fluid 12.05 m OD Water **Drill Crew** Logged by Compiled by Approved by Dates Drilled Start 01/02/2010

Dates	Drillea	Start	01/02/2010 01/03/2010			02/	02/201	04/0	2/2010 12/04/2010			
D-1-	0	Water	Sample/Co	re Rec	overy					Depth		
&	Casing Depth	(111)	Depth (m)	Туре	No. SCR	DOD	SPT Blows /N		Description of Strata	(Thick- ness)	Level	Legend
Time	(m)	(Flush Return) %	From To	%	%	%	Core Size (mm)	Index		(m)		
		(70)	3.60-8.50	91	81	52		AZCL 3	Medium strong fractured thinly foliated light grey and dark grey quartz mica SCHIST. Slightly weathered. Fractures are closely to medium spaced subvertical to subhorizontal planar smooth locally infilled with fine gravel of schist. Surfaces locally stained orange brown.  Between 7.45m and 7.49m: Fine gravel infilled fracture. Between 7.60m and 7.62m: Fine gravel infilled fracture.	6.40	5.65	
			<u>-</u>					15		‡		~~~
			- - - - -					AZCL				
			- - - - -					13	Between 9.07m and 9.36m: Orange stained	† - - - -		
		(70)	8.50-10.00	83	55	17		NI	highly fractured zone.			
			- - - -					>20		<u> </u>		
			- - -					1		<u> </u>		
01/03	7.00	1.00	- -						End of Borehole	10.00	2.05	
Pomar												

Remarks (See notes & keysheets)

Borehole advanced by chiselling from 3.20m to 3.50m (45 mins); from 3.50m to 3.60m (60 mins); from 3.60m to 3.60m (30 mins).

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR13 (2 of 2)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No. BH12** 150mm to 35.00m 150mm to 3.80m Equipment Massenza MI6 Coordinates 232152 F Local Grid) 709847 Ν **Drill Fluid** Ground Level 12.38 m OD **Drill Crew** Compiled by Logged by Approved by **Dates Drilled** Start 02/03/2010 04/03/2010 05/03/2010 12/04/2010 End 04/03/2010 Water Sample/Core Recovery SPT Depth Depth (m) Blows /N racture Date Casing No. Level Legend Type Index (Thick-Depth **Description of Strata** & Depth (m) (Flush Return) % ness) or Core Size (mm) Time (m) TCR % SCR RQD Result From To % (m) Grass over MADE GROUND: Composed of gravel, cobbles and boulders (Drillers 02/03 description) (3.60)

Medium strong to strong fractured thinly foliated light grey and dark grey quartz mica SCHIST. Slightly weathered. Fractures are closely to medium spaced subhorizontal to subvertical stepped smooth and planar smooth locally infilled up to 8mm with quartz.

Remarks 1 (See notes & keysheets)

- Prior to boring a Cable Avoidance Tool (CAT) survey was carried out. An inspection pit was hand-dug to 1.20m depth and rescanned using the CAT to check for services. Services were not located.
- 2 Rotary coring was carried out using a core barrel 131mm in diameter.

75

- 3 The borehole was advanced by rotary open hole means to 4.00m and then progressed by rotary coring means to 35.00m.
- 4 Flush loss in overburden at base of symmetrix casing.

95 91

See installation details on final sheet.

Scale 1:25 6 Groundwater was not apparent during boring.

4.00-5.85



(100)

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR14 (1 of 8)

3.60

8.78

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No.** 150mm to 35.00m 150mm to 3.80m

Equipment Massenza MI6 **Drill Fluid Drill Crew** 

Logged by Compiled by Approved by

**BH12** Coordinates (Local Grid) Ground Level 232152 **E** 709847 **N** 12.38 **m OD** 

Drill Cr Dates	rew Drilled	Start	02/03/	2010			Log	ged by	Com	piled by Approved by				
		End	04/03/	2010				03/201		3/2010 12/04/2010				
Date & Time	Casing Depth (m)	Water Depth (m) (Flush Return)	Depth		Type TCR	No.	RQD	Coro	Fracture Index or Result	Description of	of Strata	Depth (Thick- ness)	Level	Legen
		%	From	То	%	%	%	(mm)				(m)		
			[						0			Ŧ l		~~~
			_									Ł		~~~
			_									ŧ l		~~~
			_									ţ l		~~~
			_									<u> </u>		~~~
			-									‡		~~
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			-									‡		~~~
			-									‡		~~~
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			[									E		~~
ļ			_									F		~~
			-									ŧ I		~~~
			_									ţ l		~~~
ļ												ŧ l		$\sim\sim$
		(70)	5.85	-8.05	100	97	85		3			L		$\sim\sim$
			-									‡		$\sim\sim$
ļ			-									‡		$\sim\sim$
ļ			-									‡		$\sim\sim$
			-							Between 7.45m and 7.55m	. Ouartz bande un	‡		~~
			_							to 40mm thick.	Quartz bands up	<b>‡</b>		~~~
			-							At 7.67m: Induced fract	cure inclined 45	‡		~~~
			Ī							degrees undulating smoo		Į.		~~~
			Ē							Between 7.72m and 7.74m	n: Quartz band.	Ŧ I		$\sim\sim$
			<u>[</u> [									Ŧ I		$\sim\sim$
ļ			_									├		$\sim\sim$
ļ			-									ŧ l		$\sim\sim$
			_									<u>t</u>		~~~
			-									‡		~~~
ļ			<u> </u>		1							‡		~~~
ļ			-									<b> </b>		~~~
ļ			- -									‡		~~~
ļ				0	1.00					At 8.75m: Induced fract		‡		$\sim$
		(0)	8.05	-9.55	100	99	69			degrees undulating smoo	otn moderately open	‡		$\sim\sim$
ļ			F		1							‡		~~~
ļ			-		1				0	At 9.10m: Fracture incl		F		~~~
ļ			_							undulating smooth moder	rately open clean.	<u> </u>		~~~
ļ			E									F		~~~
ļ			Ē									<u> </u>		~~~
					1							<u> </u>		$\sim$
ļ					1							F		$\sim\sim$
ļ			<u>.</u>		1							t l		~~~
ļ			}		1					At 9.77m: Induced fract degrees undulating slig		<u> </u>		~~~
ļ			-							clean.	,	<u>†</u>		~~~
			<u>L</u>									<u>t</u>		~~~
									<u> </u>					

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR14 (2 of 8)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No. BH12** 150mm to 35.00m 150mm to 3.80m Equipment Coordinates (Local Grid) Ground Level 232152 **E** 709847 **N** 12.38 **m OD** Massenza MI6 **Drill Fluid** Approved by **Drill Crew** Logged by Compiled by

Drill Control	rew Drilled	Start	02/03/2010			Log	ged by	Com	piled by Approved by				
		End	04/03/2010					05/0	3/2010 12/04/2010		T	1	
&	Casing Depth	(m) (Flush	Sample/Co	Туре	No.		/N Core	Fracture Index or	Description o	of Strata	Depth (Thick- ness)	Level	Legend
Time	(m)	Return) %	From To	TCR %	SCR %	RQD %	Size (mm)	Result			(m)		
			-								- ` ′		~~~
									At 10.20m: Induced frac	ture inclined 10	E		~~~
									degrees slightly undula clean.	ating smooth tight	F		~~~
			- -						Clean.		<u> </u>		~~~
			<u>-</u>								<u>L</u>		~~~
			- -								‡		~~~
		(0)		100	0.0	<b>61</b>					‡		~~~
		(0)	- 9.55-12.00	100	96	61					F		~~~
			-								Ŧ		~~~
			_						Between 11.00m and 11.1		F		~~~
									inclined 70 degrees and undulating striated ver		<u> </u>		~~~
			= =-						veins on surfaces.		Ł		~~~
			-						At 11.40m: Fracture inc	lined 60 degrees	‡		~~~
			<u>-</u>						undulating rough open s	stained orange	‡		$\sim\sim$
			<del>-</del>						brown on surfaces penet Between 11.55m and 11.6		<b>†</b>		~~~
			<del>-</del>						inclined 25 degrees und open stained orange and		‡		~~~
			<del>-</del>						grey silt <5mm.		‡		~~~
			<del>-</del> -						Between 11.68m and 11.8 inclined 60 degrees und		‡		~~~
02/03	4.00	4.20	<del>-</del> <del>-</del>						very open locally stair	ned orange.	‡		~~~
03/03	4.00		-					3	Between 11.91m and 11.9 Between 12.04m and 12.1		F		~~~
			-						fractures inclined 60 c smooth open clean.	legrees planar	‡		~~~
			<del>-</del>						smooth open clean.		‡		~~~
			-						Between 12.43m and 12.7	4m: Induced	ļ.		~~~
			-						fractures inclined 45 of smooth moderately open		ŧ		~~~
			<del>-</del>						smooth moderatery open	Clean.	F		~~~
			-								Ŧ		~~~
			_								F		~~~
											F		$\sim\sim$
			<u>-</u>						At 12.98m: Induced fract degrees planar slightly		Ł		~~~
			-						open clean.	<b>3</b>	<u> </u>		~~~
			-								ţ		~~~
			<u>-</u>								<u>F</u>		~~~
			- -								‡		~~~
			<u>-</u>								<b>L</b>		~~~
		(0)	12.00-15.15	100	98	67					‡		~~~
			<del>-</del>						24 12 FF Tudosad for		‡		~~~
			-						At 13.77m: Induced fract degrees slightly undula		‡		~~~
			-						moderately open clean.		‡		~~~
			<del>-</del>						1. 14 10 1. 1. 5	-t	‡		~~~
			-						At 14.10m: Induced fract degrees undulating smooth	oth open some	‡		~~~
			-						angular gravel infill <		‡		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			-						quartz.		Ŧ		~~~
			-						At 14.47m: Induced frac	cture inclined 30	‡		~~~
			<del>-</del>						degrees slightly undula		F		~~~
			[						rough tight clean. Between 14.66m and 14.9		<del>I</del>		~~~
									inclined 30 and 80 degr very open quartz veins		E		~~~
			E								F		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			[								Ŧ.		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			<u> </u>						At 15.00m: Induced fraction degrees slightly stepped		<b>H</b>		
_							Щ_		degrees strantly steppe	a smooth tight to			

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR14 (3 of 8)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No. BH12** 150mm to 35.00m 150mm to 3.80m Equipment Coordinates (Local Grid) Ground Level 232152 **E** 709847 **N** 12.38 **m OD** Massenza MI6 **Drill Fluid** 

Compiled by

Approved by

Logged by

Drill C Dates	rew Drilled	Start	02/03/	2010			Log	ged by	Com	piled by	Approved by				
		End Water	04/03/	2010					05/0		12/04/2010			ı	
Date & Time	Casing Depth	Depth (m)	Depth	mple/Co (m)	Туре	No.		/N Core	Fracture Index or		Description of	of Strata	Depth (Thick- ness)	Level	Legen
Time	(m)	Return)	From	То	%	%	RQD %	Size (mm)	Result				(m)		
			-							moderate	ely open clean.	75m: Quartz bands	ţ		~~~
			-								m thick.	Jim. Quartz Danus	ţ		~~~
			-										Ŧ		~~
													Ŧ		~~~
			_										<u> </u>		~~~
			-							A+ 15.70	m. Induced frac	cture inclined 60	‡		~~
										degrees		moderately open	Ŧ		$\sim\sim$
										clean.			E		~~
			_										<u> </u>		$\sim\sim$
			-										‡		$\sim\sim$
			-										Ŧ		$\sim\sim$
			[										E		~~~
			-										<b>‡</b>		$\sim\sim$
		(0)	_										<b>†</b>		$\sim\sim$
			15.15	-18.15	100	100	95						Ŧ		$\sim\sim$
			[										F		$\sim\sim$
			_										ŧ		$\sim\sim$
			_										<u>‡</u>		$\sim\sim$
			-										ļ		$\sim$
			_										E		$\sim$
			-							λ+ 17 <i>Δ</i> (	m. Induced frac	cture inclined 30	ŧ		~~~
			-								undulating roug		<b>†</b>		~~~
			_										<b>†</b>		~~~
			_							At 17.74	m: Induced frac	cture inclined 30	Ŧ		~~~
			_							degrees clean.	planar smooth m	moderately open	Ŧ.		~~~
			-							0_000			<b>†</b>		~~
			-										<b> </b>		~~
			_										Ē		~~
													Ł		~~~
			-										‡		~~
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			<u> </u>										‡		~~
			Ē										E		$\sim\sim$
			[										E		$\sim\sim$
			-								im: Induced frac planar smooth t	cture inclined 60	(31.40)		$\sim\sim$
			-									aline band (250mm	‡		~~~
		(0)								thick).		IIIIe Dana (230mm	F		$\sim\sim$
			18.15	-21.15	100	100	90						ţ		$\sim\sim$
			F										ţ		$\sim\sim$
			-										‡		~~~
			<u> </u>										<b>F</b>		~~
	<u> </u>				Щ_		Щ_								

Remarks (See notes & keysheets)

**Drill Crew** 

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No. FR14 (4 of 8)

304/03

Drilling Method Rotary Open Hole/Coring

Borehole Diameter
150mm to 35.00m
Casing Diameter
150mm to 3.80m
BOREHOLE No.

Equipment Massenza MI6

Drill Fluid
Drill Crew

Dates Drilled Start 02/03/2010 End 04/03/2010 Logged by Compiled by Approved by
04/03/2010 05/03/2010 12/04/2010

	Dillieu	End	04/03/				04/		LO 05/0				
Date	Casing	Water Depth	Sa	mple/Co	_			SPT Blows	Fracture Index		Depth		
&	Depth	(m)	Depti	n (m)	Туре				Index or	Description of Strata	(Thick- ness)	Level	Leger
Γime	(m)	(Flush Return) %	From	То	TCR %	SCR %	RQD %	Core Size (mm)	Result		(m)		
			_								- '		~~~
			-								‡		$\sim\sim$
			Ē								Į.		$\sim\sim$
			-							At 20.35m: Induced fracture inclined 10 degrees undulating smooth tight clean.	Į.		~~~
										At 20.44m: Induced fracture inclined 35	‡		$\sim\sim$
			-						1	degrees planar striated. At 20.45m: Induced fracture subhorizontal	<b>.</b>		$\sim\sim$
			-						_	planar smooth open some gravel infill of	‡		$\sim$
			_							schist <20mm. At 20.75m: Two induced fractures inclined	ţ		~~~
										30 degrees planar smooth clean.	ţ		$\sim$
			_								t		$\sim$
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			-								Ŧ		$\sim\sim$
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			_								<u>†</u>		~~
			-								ł		$\sim$
			-								Ŧ		~~
			_								F		~~
			-								‡		$\sim$
										At 22.30m: Induced fracture inclined 15	‡		$\sim$
			-							degrees slightly undulating rough tight clean.	ţ		~~
										Clean.	ţ		~~
			_								F		$\sim$
			_								ł		~~
		(0)	21.15	-24.15	100	100	79				F		~~
			Ī								Ŧ		$\sim$
			-							At 22.98m: Induced fracture inclined 60	‡		$\sim$
			_							degrees stepped smooth moderately open	Ļ		~~
			-							clean.	‡		~~
			-								‡		$\sim$
			-								ţ .		~~
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			<u>-</u>								Ł		$\sim$
			_								ł		$\sim$
			-								Ŧ		$\sim$
			Ī							At 23.80m: Induced fracture inclined 25	Į.		~~
			-							degrees undulating smooth tight clean.	Ŧ		$\sim$
			-								‡		$\sim$
			_								<b>F</b>		~~
					1		<u> </u>				ţ .		$\sim$
			_								ţ		$\sim$
			<u> </u>		1						t		<u>~</u>
			-								Ŧ		~~
			<u> </u>		1					Between 24.50m and 24.70m: Quartz band.	F		$\sim$
			-								Ŧ		$\sim$
			ļ		1						ţ		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			-								‡		$\sim$
			<u>-</u>		1						ţ .		$\sim$
			<u> </u>		1						Ł		~
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Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No. FR14 (5 of 8)

5 of 8)

Drilling Method Rotary Open Hole/Coring Borehole Diameter 150mm to 35.00m Casing Diameter 150mm to 3.80m BOREHOLE No.

Equipment Massenza MI6

Drill Fluid Drill Crew Dates Drilled

Start 02/03/2010 End 04/03/2010 Logged by Compiled by Approved by

BOREHOLE No. BH12

Coordinates (Local Grid) 709847 N
Ground Level 12.38 m OD

Dates	Drilled	Start End	02/03/2010 04/03/2010			04/	03/201	.0 05/0	3/2010 12/04/2010			
_		Water	Sample/Co	re Rec	overy		CDT			Depth		
Date &	Casing Depth	Depth (m)	Depth (m)	Туре	No.		/N	Fracture Index	Description of Strata	(Thick-	Level	Legend
Time	(m)	(Flush Return) %		TCR %	SCR %	RQD %	Core Size (mm)	or Result		ness) (m)		
		/0	-	,,,	,,,		()			(''')		~~~
			-							‡		~~~
			<u>-</u>							‡		~~~
			-						At 25.40m: Induced fracture inclined 30	ţ		$\sim\sim$
			-						degrees stepped rough tight clean.	ţ		~~~
			_							<u> </u>		~~~
			_							ł		~~~
		(0)	24.15-27.15	100	97	88				ł		$\sim\sim$
			F							F		~~~
			-						Between 25.85m and 25.96m: Quartz band.	Ī		~~~
			<u>L</u>							Į.		~~~
			-							‡		$\sim\sim$
										‡		~~~
			-							ţ.		~~~
			-							t		~~~
			-							ŧ		~~~
			F							F		~~~
			-							Ŧ		$\sim\sim$
			Ē							Į.		$\sim\sim$
			Ī						At 26.85m: Fracture inclined 30 degrees planar smooth locally infilled brown fine	‡		~~~
			-						to medium sand <2mm.	‡		~~~
			<u>-</u>							<b>-</b>		$\sim\sim$
03/03	4.00	1.80								ţ		$\sim\sim$
04/03	4.00		_						At 27.30m: Fracture inclined 30 degrees	ţ		~~~
			-						stepped to undulating smooth open clean.	ł		~~~~
			-						Between 27.46m and 27.65m: Fracture	Ŧ		$\sim\sim$
			-						inclined 70 degrees stepped smooth tight	F		~~~
			-						clean.	Ī		~~~
			Ī							‡		$\sim\sim$
			<del>-</del>							‡		$\sim\sim$
		(0)	27.15-28.65	100	96	63			At 27.89m: Induced fracture inclined 30	‡		$\sim\sim$
		(0)	_ 27.15-26.65	100	96	0.3		2	degrees undulating rough moderately open clean.	L		~~~~
			_							ŧ		$\sim\sim$
			-							ł		~~~
			ŀ							ł		
			-							Ŧ		~~~
			F							F		~~~
			F							F		~~~
			<del>-</del>	1	1					Į.		
			<del>-</del>					AZCL		‡		~~~
			-						At 28.83m: Fractures subhorizontal stepped striated open clean.	‡		~~~
			‡							<b>‡</b>		~~~
			F						At 29.00m: Induced fracture subhorizontal planar striated tight clean.	F		~~~~
			ŀ						F Dollacoa olgato olean.	t		~~~
			<u> </u>							Ł		~~~
			F							F		
		(0)	28.65-30.15	99	91	67		2		Ŧ		$\sim\sim$
		, , ,			-	-				ļ.		~~~
			-							‡		~~~
			ţ							t		~~~
			-							t		~~~
			ŀ							t		~~~
			-	1					Between 30.05m and 30.15m: Highly	ł		~~~~
									fractured zone.			
Remar	ke											

Remarks (See notes & keysheets)

Scale 1:25

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SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR14 (6 of 8)

Drilling Method Rotary Open Hole/Coring **Borehole Diameter Casing Diameter BOREHOLE No. BH12** 150mm to 35.00m 150mm to 3.80m Equipment Massenza MI6

**Drill Fluid** Logged by **Drill Crew** Compiled by Approved by **Dates Drilled** 02/03/2010

Coordinates (Local Grid) Ground Level 232152 **E** 709847 **N** 12.38 **m OD** 

		End	04/03/2010			04/	03/201	.0 05/0	3/2010 12/04/2010			
D-1-	0	Water	Sample/Co	re Rec	overy		SPT	Fracture		Depth		
	Casing	(111)	Danth (m)	Туре	No.		/N	Fracture Index	Description of Strate	(Thick-	Level	Legend
& Time	Depth (m)	(Flush Return)	Depth (m)	TCP	SCR	POD	Core	_or	Description of Strata	`ness)		
Tille	(111)	Return) %	From To	%	%	<b>%</b>	Core Size (mm)	Result		(m)		
			-									~~~
			- -					NI		ļ.		~~~
			<b>-</b> -						At 30.29m: Fracture inclined 10 degrees	_		~~~
			-						undulating smooth open clean.			$\sim\sim$
			-					1		-		~~~
			<u>-</u>							Ļ		$\sim\sim$
			-						At 30.58m: Fracture horizontal stepped			~~~
			=						rough open clean.	-		~~~~
			_							-		$\sim\sim$
												~~~
			-									$\sim\sim$
			-							-		$\sim\sim$
			_							ŀ		~~~
			=						At 31.25m: Induced fracture inclined 20	-		$\sim\sim$
1			-						degrees slightly undulating smooth	F		~~~
			-						moderately open clean.	ļ.		
			<u>-</u>							L		$\sim\sim$
			-							-		~~~
		(0)	30.15-33.15	100	96	84				-		~~~~
		(0)	-	100		0.1				Ļ		$\sim\sim$
			-									~~~
			_							ŀ		$\sim\sim$
			_							_		$\sim\sim$
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			_					1	At 32.85m: Quartz band 5mm thick.	ŀ		~~~
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			_						At 33.00m: Induced fracture inclined 25	_		$\sim\sim$
			= =						degrees undulating slightly rough tight clean.	ļ.		~~~~
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			-						At 22 56m. Induged fracture inclined 20	F		~~~
			-						At 33.56m: Induced fracture inclined 30 degrees planar smooth moderately open	Γ.		~~~
			- -						clean.	ţ		$\sim\sim$
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			F						At 33.96m: Induced fracture inclined 28	F		~~~
1			<u>-</u>						degrees planar smooth tight clean.	<u>L</u>		~~~
		(0)	33.15-35.00	100	100	98				t		
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			Ē						At 34.40m: Induced fracture subhorizontal	F		~~~~
			-						undulating rough open clean.	ţ		$\sim\sim$
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04/03	4.00	4.10	-						At 34.92m: Induced fracture inclined 10 degrees planar rough tight clean.	ţ		$\sim\sim$
04/03	4.00	4.10								_35.00	-22.62	~~~
Pomari	<u> </u>				Щ_		<u> </u>		End of Borehole			<u> </u>

Remarks (See notes & keysheets)

Scale 1:25



SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No. CON103001

Figure No.

FR14 (7 of 8)

Drilling Method Rotary Open Hole/Coring **BOREHOLE No. Borehole Diameter BH12** Coordinates (Local Grid) Ground Level 232152 **E** 709847 **N** Equipment Massenza MI6 12.38 m OD **Drill Crew** Logged by Compiled by Approved by **Dates Drilled** 02/03/2010 Start 04/03/2010 05/03/2010 12/04/2010 End 04/03/2010 Installation Water Strata Level **Installation Details** Strata Details m OD Strikes Depth (m) Depth (m) Bentonite Grout Coarse/Medium grained Instrumentation: Metamorphic (SCHIST) 19mm standpipe piezometer tip at 29.50m 22.00 -9.62 Bentonite Seal 23.00 -10.62 Gravel Filter 30.00 -17.62 Bentonite Seal 31.00 -18.62 Bentonite Grout 35.00 -22.62 35.00 Base of Hole Remarks (See notes & keysheets) Water Strike ▼ Water Rise Upstanding cover. Pipe diameter 19mm to 30.00m. Project Contract No. CON103001 SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Figure No. FR14 (8 of 8)

Drilling	Metho	Concr	ete Core			Boreho	ole Diamo	eter Casing Diameter	BOREHOLE No.		CH0	1
Equipn		Concr	ete Corer						Coordinates (Local Grid) Ground Level	232141 709825 10.98		
Drill Cr Dates	ew Drilled	Start End				Logged		Approved by 12/04/2010 12/04/2010				
Data	Casing	Depth	Sample D	otaile		SPT	U100			Depth		
& Time	Depth	to Water	Depth (m)	Г		Blows/N Drive mm	Blows/ Recovery mm	Description of	f Strata	(Thick- ness)	Level	Legend
Time	(m)	(m)	From To	Туре	NO.	Test	Result			(m)		
Remark								Medium strong grey CONCRI 60-80% angular to subrour aggregate of mixed lithol coarse sand and cement may be upto 3mm. At 1.10m: Orange yellow morigin unknown.	ehole	3.28	7.70	
(See note & keyshe		Ground	water was not	appa	rent (auring	poring.					

Scale 1:25

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SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No. CON103001

Figure No.

FR15 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP01** Plan 0.60m x 0.60m 08/03/2010 Start End Date Excavated Coordinates (Local Grid) Ground Level 232215 E 710127 N 08/03/2010 ➤ 000 32.86 m OD Logged by Compiled by Approved by

8/03/2010	1	0/03/2010		1/2010					
	tu Testi		Sam			l	Depth		
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	(Thick- ness) (m)	Level	Legen
(,			· · · · · · · · · · · · · · · · · · ·			Dark grey organic silty gravelly fine to coarse SAND with occasional subangular cobbles and abundant organic fragments, wood and roots. Gravel is subangular to subrounded fine to medium of schist.	(0.15)	32.71	
						Light brown yellow and orange sandy subangular fine to coarse GRAVEL of schist with many angular to subangular cobbles and occasional boulders. Sand is medium to coarse.	(1.05)		
			- - - - - -				1.20	31.66	
			- - - - -			End of Trial Pit			
			-						
			-						
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			-				- - - -		

Remarks ₁

The walls of the pit were stable to a depth of 1.20m during excavation. (See notes & keysheets) 2

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
On completion the trial pit was backfilled with compacted arisings.
Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

Groundwater was not apparent during excavation.

Scale 1:25

Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR16 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP02 Surface Dimensions** 0.60m x 0.60m Start Coordinates (Local Grid) Ground Level Date Excavated 08/03/2010 232231 E End 08/03/2010 710120 N 000 28.74 m OD Logged by Compiled by Approved by 10/03/2010 08/03/2010 12/04/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Туре Type No. Result (m) (m) (m) Dark brown fibrous to pseudofibrous spongy PEAT with occasional subrounded schist N/Z 21/2 cobbles. VZ. M/2 (0.70) Mz. 21/2 31/2 M/z 0.70 28.04 At 0.70m: Cobble/boulder obstruction End of Trial Pit

Remarks

(See notes & keysheets)

- The walls of the pit were stable to a depth of 0.70m during excavation.
- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.
 - Cobbles/ boulders encountered at 0.70m.
 - Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately. Groundwater was encountered at 0.40m during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR17 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP03** Plan 0.60m x 0.60m 08/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232240 E 08/03/2010 710114 **N** ➤ 000 28.81 m OD Logged by Compiled by Approved by

	Ĭ		Дрио						
08/03/2010		.0/03/2010	12/04				15.0		
In-sit Depth (m)	Type	ing Result	Samp Depth (m)	Type	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Leger
(,			- ()			Dark brown organic gravelly medium to coarse SAND with frequent roots and organic fragments. Gravel is subrounded fine to medium schist.	(0.30)		
			- - - - -			Brown grey silty medium to coarse SAND and subangular to subrounded fine to medium GRAVEL of schist with many subrounded cobbles and occasional roots.	0.30	28.51	
			[- -			Light brown gravelly medium to coarse SAND with rare subrounded cobbles. Gravel is	0.70	28.11	a
			- - - -			subangular to subrounded fine to medium schist. End of Trial Pit	0.90	27.91	<u> </u>
			- - - -						
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 0.90m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.40m during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR18 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP04** Plan 0.60m x 0.60m 08/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232211 E 08/03/2010 710110 N ➤ 000 30.06 m OD Logged by Compiled by Approved by

10gged by 108/03/2010		L0/03/2010							
In-si	tu Test	ing	Samp	les	1	Description of Strata	Depth (Thick- ness)	Level	Legen
Depth (m)	Туре	Result	Depth (m)	Туре	No.		(m)		
			- - - -			Dark grey to grey organic silty gravelly medium to coarse SAND with frequent roots and organic fragments. Gravel is subangular to subrounded fine to medium schist.	(0.30)		
			[- - -			Yellow brown silty gravelly medium to coarse SAND. Gravel is subrounded fine to medium schist.	0.30	29.76	, , ,
			- - -				(0.90)		
			<u>-</u>				-		0 0
			- - - -			End of Trial Pit	1.20	28.86	
			<u>-</u> -				<u>-</u> -		
			[
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.20m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 On completion the trial pit was backfilled with compacted arisings.
 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.
- Groundwater was not apparent during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR19 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP05** Plan 0.60m x 0.60m 08/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232224 E 08/03/2010 710103 N ➤ 000 29.56 m OD Logged by Compiled by Approved by

8/03/2010 In-s	03/2010 10/03/2010 : In-situ Testing		12/0	4/2010 ples			Depth		
Depth (m)	Type	Result	Depth (m)	Туре	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
						Dark brown pseudofibrous spongy PEAT with occasional schist cobbles. Moderate organic			311/2 31/2 :
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			- -				-		2/1/2
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			- -				F (1 00)		Mz.
			ļ.				(1.20)		, N/2, N/2,
			-				F		N/2
			E				E		31/2 23/2
			-				-		M/.
			- -				1.20	28.36	, N/. , N/.
			F			End of Trial Pit	1.20	20.30	
			E				E		
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Remarks ₁

The walls of the pit were stable to a depth of 1.20m during excavation. (See notes & keysheets)

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.35m during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR20 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP06** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 08/03/2010 232239 E 710097 N 08/03/2010 ➤ 000 29.20 m OD Logged by Compiled by Approved by

08/03/2010		08/03/2010	12/0	4/2010		<u> </u>			
	tu Test Type	ing		Type	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legend
			-			Dark brown organic silty gravelly medium to coarse SAND with frequent organic fragments, wood and roots. Gravel is subangular to subrounded fine to coarse schist. Light brown and grey slightly sandy gravelly SILT. Gravel is subrounded fine to medium schist. Sand is medium to coarse. Dark brown organic subrounded fine to medium GRAVEL of schist.	(0.15) - (0.15) - (0.15) - (0.30	29.05 28.90	
			- - - - - -			End of Trial Pit	0.95	28.25	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 0.95m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.55m during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR21 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP07 Surface Dimensions** 0.60m x 0.60m Start Date Excavated 08/03/2010 Coordinates 232205 E End 08/03/2010 (Local Grid) 710094 N Ground Level 000 31.59 m OD Logged by Compiled by Approved by 10/03/2010 08/03/2010 12/04/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend Depth Depth ness) Type Result Type No. (m) (m) (m) Dark brown organic silty medium to coarse SAND with frequent roots and leaves. (0.10) 0.10 (0.05) 31.49 Light grey silty gravelly medium to coarse SAND. Gravel is subrounded fine to medium 0.15 31.44 Brown orange and yellow gravelly medium to coarse SAND with many subangular cobbles. Gravel is subangular to subrounded fine to medium schist. (0.90)30.54 1.05 End of Trial Pit

Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.05m during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

Groundwater was not apparent during excavation.

Scale 1:25

Project

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Contract No.

CON103001

Figure No.

FR22 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP08** Plan 0.60m x 0.60m 08/03/2010 Start End Date Excavated Coordinates (Local Grid) Ground Level 232214 **E** 710087 **N** 09/03/2010 **→** 000 30.07 m OD Logged by Compiled by Approved by

8/03/2010		.0/03/2010		4/2010					
	tu Testi	ing		ples	7	Description of Strata	Depth (Thick-	Level	Legen
Depth (m)	Туре	Result	Depth (m)	Туре	No.		ness) (m)		
			<u> </u>			Dark brown fibrous spongy PEAT with occasional cobbles. Slight organic odour.	Ė		
			-			-	- (0.45)		
			[0.45	20. 62	.\\\. .\\
			<u> </u>			Light grey silty gravelly medium to coarse SAND with many subangular cobbles. Gravel is	- 0.45	29.62	٥
			- - -			subangular to subrounded fine to coarse schist.	Ė		
			_				(0.65)		σ.
			<u>-</u>				<u>E</u>		, , ,
			-				1.10	28.97	
			<u>-</u>			End of Trial Pit	-		
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Remarks ₁ (See notes & keysheets)

- The walls of the pit were stable to a depth of 1.10m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.50m during excavation.

Scale 1:25



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Contract No.

CON103001

Figure No.

FR23 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP09** Surface Dimensions 0.60m x 0.60m Start Coordinates (Local Grid) Date Excavated 08/03/2010 232235 E End 08/03/2010 710082 N Ground Level 000 29.41 m OD Logged by Compiled by Approved by 10/03/2010 12/04/2010 08/03/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Туре Type No. Result (m) (m) (m) Dark brown organic silty gravelly medium to coarse SAND with frequent roots, wood and organic fragments. Gravel is subrounded fine (0.20)0.20 29.21 to medium schist. Dark brown organic sandy subangular to subrounded fine to medium GRAVEL of schist with many subangular cobbles and occasional roots. Sand is medium to coarse. (0.70) 28.51 0.90 End of Trial Pit

Remarks ₁

(See notes & keysheets) 2

The walls of the pit were stable to a depth of 0.9m during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

- 3 On completion the trial pit was backfilled with compacted arisings.
- 4 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

5 Groundwater was encountered at a depth of 0.20m during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd Contract No.

CON103001

Figure No.

FR24 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP10** Plan 0.60m x 0.60m 04/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232194 **E** 710077 **N** 04/03/2010 ➤ 000 33.25 m OD Logged by Compiled by Approved by

/03/2010		0/03/2010	12/0	4/2010					
In-si	tu Testi		Sam	ples	,	Description of Strata	Depth (Thick-	Level	Leger
Depth (m)	Туре	Result	Depth (m)	Туре	No.	bessipion of ortale	ness) (m)	20101	Logo
			- - -			Dark brown organic silty fine to coarse SAND with frequent roots, wood and organic fragments.	(0.20)	33.05	×
			- - -			Grey mottled orange brown silty gravelly fine to coarse SAND with occasional subrounded cobbles. Gravel is subangular to subrounded fine to coarse schist.	0.35	32.90	a
			- - - -			Grey fine to coarse SAND and subrounded fine to coarse GRAVEL of schist with many subrounded cobbles and rare schist boulders.	(0.70)		
			-				1.05	32.20	0 0 0 0
			- - - - -			End of Trial Pit	-	32.20	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.05m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at a depth of 0.40m during excavation.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

Figure No.

FR25 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP11** Plan 0.60m x 0.60m 09/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232205 E 710067 N 09/03/2010 ➤ 000 30.41 m OD Logged by Compiled by Approved by

Logged by		omplied by							
09/03/2010 In-sit	tu Test	ina	12/04, Samp				Depth		
Depth (m)	Туре		Depth (m)	Туре	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
			- ` ´			Soft dark brown pseudofibrous PEAT with slight organic odour.			
			Ė			slight organic odour.	Ę		2/1/2
			E				E		2/1/2
			<u> </u>				- (0.85)		
			ļ.			At 0.60m: 2 No. boulders encountered.	<u> </u>		31/2 . 31/2
			<u> </u>				E		Mz .
			<u> </u>				0.85	29.56	7/1/2
			<u>-</u>			End of Trial Pit	<u> </u>	23.30	
			<u> </u>				<u> </u>		
			<u> </u>				E		
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Remarks ₁

(See notes & keysheets)

- The walls of the pit were stable to a depth of 0.85m during excavation.
- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.
- Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately. Groundwater was encountered at a depth of 0.40m during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR26 (1 of 1)

Method of Excavation Surface Dimensions ons 0.60m x 0.60m Start 09/03/2010 End 09/03/202 TRIAL PIT No. **MP12** Plan Date Excavated Coordinates (Local Grid) Ground Level 232225 E 710058 N **→** 000 31.34 m OD Logged by Compiled by Approved by

2/03/2010		0 / 03 / 2010		1/2010]			
9/03/2010 In-site	u Testi	0/03/2010	Sam	1/2010 plac			Depth		I
Depth (m)	Type	Result	Depth (m)	Туре	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Leger
			- ` ′			Dark brown fibrous spongy PEAT with moderate organic odour.			.N/. .N/.
			<u> </u>			organic odour.	(0.35)		31/2 31/2
			- -				, , , , , ,		Mz.
			- -			Grey and brown clavey gravelly medium to	0.35	30.99	o
						Grey and brown clayey gravelly medium to coarse SAND with many angular to subangular cobbles and frequent wood and organic fragments. Gravel is subangular to subrounded fine to coarse schist.	(0.55)		y 0
			_ -				-		o d
			- -				0.90	30.44	., . ,
						End of Trial Pit	- 0.90	30.11	
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Remarks ₁

(See notes & keysheets)

- The walls of the pit were stable to a depth of 0.90m during excavation.
- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.45m during excavation.

Scale 1:25

Project

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Contract No.

CON103001

Figure No.

FR27 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP13** Surface Dimensions 0.60m x 0.60m Start 09/03/2010 Date Excavated Coordinates (Local Grid) 232181 E End 09/03/2010 710058 N Ground Level 000 33.57 m OD Logged by Compiled by Approved by 10/03/2010 09/03/2010 12/04/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Type No. Type Result (m) (m) (m) Dark brown organic silty medium to coarse SAND with frequent plant remains and roots. (0.20)0.20 33.37 Dark brown orange sandy subangular to subrounded fine to coarse GRAVEL of schist with occasional subrounded cobbles and (0.55)roots. 0.75 32.82 At 0.75m: Schist boulder/ bedrock. End of Trial Pit

Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 0.75m during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

- Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.
- Groundwater was not apparent during excavation.

Scale 1:25



Project

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Figure No.

FR28 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP14** Plan 0.60m x 0.60m 09/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232191 E 09/03/2010 710049 N ➤ 000 30.64 m OD Logged by Compiled by Approved by

/03/2010		0/03/2010		4/2010					
	itu Testi			ples			Depth (Thick-		
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Level	Legen
. ,						Dark brown pseudofibrous spongy PEAT with moderate organic odour.	-		
			<u>-</u>			moderate organic odour.	- -		
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			Ė			End of Trial Pit	1.20	29.44	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.20m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.40m during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR29 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP15** Plan 0.60m x 0.60m 09/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232196 E 09/03/2010 710041 N ➤ 000 30.05 m OD Logged by Compiled by Approved by

9/03/2010	1	10/03/2010		1/2010					
	itu Test		Sam				Depth (Thick-		
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Level	Legei
. ,						Dark brown fibrous to pseudofibrous spongy PEAT with strong organic odour.	- ` ` `		31/2 31/2
			<u> </u>			FERT WICH SCIONS OF SAME ORDER.	Ė		21/2 21/2
			Ė				Ė		71/5 71/5
			<u> </u>				<u>-</u>		71/2
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			F				<u> </u>		M/.
			Ė			At 1.10m: Schist cobbles and boulders.		28.95	\/\/
			F			End of Trial Pit	<u>-</u>		
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.10m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.30m during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR30 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP16** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 09/03/2010 232215 E 09/03/2010 710040 N ➤ 000 30.60 m OD Logged by Compiled by Approved by

/03/2010	1	0/03/2010	12/04	/2010					
	tu Testi Type		Samp Depth (m)		No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
(111)	+		- (111)	-		Dark brown pseudofibrous PEAT with rare subrounded cobbles of schist. Moderate	- (111)		S/I/2
			= = =			subrounded cobbles of schist. Moderate organic odour.	Ē		31/2 : 31/2
			- -				F		31/2, 3 31/2,
			- - -				-		Mz. :
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			- - -				(1.20)		.\\/ .\\/ :
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			- -				-		Alz Alz.
			_ -				 -		31/2. 31/2.
			- -				1.20	29.40	Mz
			= = =			End of Trial Pit			
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Remarks ₁

The walls of the pit were stable to a depth of 1.20m during excavation. (See notes & keysheets)

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.40m during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR31 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP17** Plan 0.60m x 0.60m 10/03/2010 Date Excavated Start End Coordinates (Local Grid) Ground Level 232167 E 10/03/2010 710036 N ➤ 000 31.95 m OD Logged by Compiled by Approved by 10/03/2010 12/04/2010 10/03/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend Depth (m) Depth ness) Type Result Type No. (m) (m) Dark grey sandy SILT with frequent roots and organic fragments. Sand is fine to coarse. (0.20) 0.20 31.75 Brown sandy subrounded fine to coarse GRAVEL of schist with many subrounded cobbles. Sand is medium to coarse.

End of Trial Pit

(0.90)

1.10

Contract No.

Figure No.

CON103001

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FR32 (1 of 1)

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Remarks ₁	The	walls of t	he pit were s	 table	to a	depth of 1.10m during excavation.		j	
(See notes & keysheets) 3	Pric	or to excav	ration a Cable	Avoi	dance	Tool (CAT) survey was carried out.			
3	On	completion	the trial pit	was 1	backf	illed with compacted arisings.			
4						se of the trial pit to refusal. Results presented sep	arately.		
5	Grou	ındwater wa	s not apparen	t dur	ing e	xcavation.			

Project

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Scale 1:25

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Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP18** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 10/03/2010 232182 E 10/03/2010 710034 N ➤ 000 30.00 m OD Logged by Compiled by Approved by

703/2010		1/03/2010	12/04	/2010					
In-si Depth (m)	tu Testi Type	ng Result	Sam Depth (m)	oles Type	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
(,			(,			Dark brown pseudofibrous spongy PEAT with slight organic odour.	- (,		alka a
			- -			slight organic odour.	-		N/Z
			<u></u>				Ē		31/2 : 31/2
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			- -				1.20	28.80	Mz
			<u> </u>			End of Trial Pit	E		
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.20m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.20m during excavation.

Scale 1:25



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Figure No.

FR33 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP19** Plan 0.60m x 0.60m 10/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232193 E 710027 N 10/03/2010 ➤ 000 30.48 m OD Logged by Compiled by Approved by

Logged by			10/04						
	u Test		12/04/ Samp	les	1	Description of Strata	Depth (Thick-	Level	Legen
Depth (m)	Туре	Result	Depth (m)	Туре	No.		ness) (m)		
			-			Dark brown fibrous spongy PEAT.	-		31/2 31
			- -				-		2/1/2
			- -				(0.60)		31/2 s 31/2
			- -				-		alla s alla
							F [29.88	N//.
			<u>-</u>			Soft grey sandy gravelly CLAY with	0.60	29.00	
			- -			Soft grey sandy gravelly CLAY with occasional pockets of yellow medium to coarse sand. Gravel is subangular to subrounded fine to medium schist. Sand is	(0.40)		
			- -			fine to coarse.	1.00	29.48	
			- -			End of Trial Pit	- 1.00	29.40	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.00m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at 0.50m during excavation.

Scale 1:25

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Figure No.

FR34 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP20** Plan 0.60m x 0.60m 09/03/2010 Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232159 E 09/03/2010 710030 N ➤ 000 31.51 m OD Logged by Compiled by Approved by

9/03/2010		0/03/2010	12/04	/2010					
In-sit	u Testi		Samp		,	Description of Strata	Depth (Thick-	Level	Legen
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Levei	Legen
			- - - -			Dark grey black organic silty slightly gravelly medium to coarse SAND with frequent roots and organic fragments.	(0.15)	31.36	· · · · · · · · · · · · · · · · · · ·
			- - - - -			Orange brown gravelly medium to coarse SAND with many subangular cobbles. Gravel is subangular to subrounded fine to medium schist.	- (0.60)		
			- - - -			End of Trial Pit	0.75	30.76	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 0.75m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 On completion the trial pit was backfilled with compacted arisings.
 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.
- Groundwater was not apparent during excavation.

Scale 1:25



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Contract No.

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Figure No.

FR35 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP21** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 09/03/2010 232177 E 09/03/2010 710020 N ➤ 000 29.54 m OD Logged by Compiled by Approved by

9/03/2010	1	.0/03/2010	12/04	/2010					
	tu Test	ing	Sam Depth (m)		No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
(m)	+		(m)	+		Dark brown naoudofilmous DEAT	(m)		2/1/2
			<u>-</u>			Dark brown pseudofibrous PEAT.	- -		
			<u> </u>				-		Mz .
			<u>-</u>				-		31/2 31/2
			<u>-</u>				<u> </u>		2/1/2
			Ē				(1.10)		31/2 31/2
			<u>-</u>				-		MZ.
			-				‡		31/2 31/2
							<u> </u>		31/2.
			-				1.10	28.44	71/2
			F			End of Trial Pit	- 1.10	20.44	
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Remarks ₁

The walls of the pit were stable to a depth of 1.10m during excavation. (See notes & keysheets)

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was encountered at ground level during excavation.

Scale 1:25



Project

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Figure No.

FR36 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP22** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 10/03/2010 232176 E 10/03/2010 710015 N **→** 000 29.10 m OD Logged by Compiled by Approved by 10/03/2010 11/03/2010 12/04/2010

0/03/2010 11/03/2010 In-situ Testing		12/04	1/2010						
		Samples			Description of Strata	Depth (Thick-	Level	Legen	
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Ottata	ness) (m)	20001	
			<u>-</u>			Dark brown fibrous spongy PEAT.	- -		31/2 31/2
			_				Ē		2/1/2 2/1/2
			-				-		. N/z
			<u>-</u>				<u> </u>		N/A
							(1.20)		71/2 21/2
			<u> </u>				Ė		71/2 71/2
			-				-		71/2 71/2
			<u></u>			At 1.00m: Schist boulder.	<u>-</u>		N//.
			<u>-</u> 				1.20	27.90	71/5 71/5
			<u></u>			End of Trial Pit	-		
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Remarks ₁ (See notes & keysheets)

- The walls of the pit were stable to a depth of 1.20m during excavation.
- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

 On completion the trial pit was backfilled with compacted arisings.

 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

 Groundwater was enountered at 0.35m during excavation.

Scale 1:25



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Figure No.

FR37 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan MP23 Surface Dimensions 0.60m x 0.60m Start Date Excavated 10/03/2010 Coordinates 232163 E End 10/03/2010 (Local Grid) 710003 N Ground Level 000 29.95 m OD Logged by Compiled by Approved by 11/03/2010 10/03/2010 12/04/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Type Result Type No. (m) (m) (m) Dark brown organic silty fine to coarse SAND with frequent roots and organic fragments. (0.20)0.20 29.75 MADE GROUND: Composed of brown micaceous gravelly medium to coarse sand with many angular to subangular cobbles and occasional brick fragments. Gravel is subangular to subrounded fine to coarse schist. (1.00)28.75 1,20 End of Trial Pit

Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.20m during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

- Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

Groundwater was not apparent during excavation.

Scale 1:25



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Figure No.

FR38 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP24** Surface Dimensions 0.60m x 0.60m Start Coordinates (Local Grid) Ground Level Date Excavated 10/03/2010 232165 E End 10/03/2010 709900 N 000 28.46 m OD Logged by Compiled by Approved by 11/03/2010 10/03/2010 12/04/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Туре Type No. Result (m) (m) (m) Dark grey black sandy SILT with frequent roots and organic fragments. Sand is medium (0.10) 0.10 28.36 to coarse. Light brown sandy subangular to subrounded fine to coarse GRAVEL of schist with occasional subrounded cobbles. (1.10)27.26 1,20 End of Trial Pit

- Remarks ₁
- (See notes & keysheets)
- The walls of the pit were stable to a depth of 1.20m during excavation.
 - Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

 - Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.
 - Groundwater was not apparent during excavation.

Scale 1:25



Project

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Figure No.

FR39 (1 of 1)

Method of Excavation Hand dug TRIAL PIT No. Plan **MP25** Surface Dimensions 0.60m x 0.60m Start Coordinates (Local Grid) Ground Level Date Excavated 10/03/2010 232167 E End 709979 N 10/03/2010 000 28.40 m OD Logged by Compiled by Approved by 11/03/2010 12/04/2010 10/03/2010 Depth (Thick-In-situ Testing Samples **Description of Strata** Level Legend ness) Depth Depth Туре Type No. Result (m) (m) (m) Dark brown black sandy organic SILT with (0.10) 0.10 28.30 frequent roots. Brown sandy subangular to subrounded fine to coarse GRAVEL of schist with many subangular cobbles. Sand is medium to coarse. (1.10)27.20 1,20 End of Trial Pit Remarks ₁ The walls of the pit were stable to a depth of 1.20m during excavation. (See notes & keysheets) Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

- - Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.
 - Groundwater was encountered at 0.55m during excavation.

Scale 1:25



Project

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Figure No.

FR40 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. **MP26** Plan 0.60m x 0.60m Start End **Date Excavated** Coordinates (Local Grid) Ground Level 232174 E 10/03/2010 10/03/2010 709969 N ➤ 000 28.42 m OD Logged by Compiled by Approved by

10/03/2010 11/03/2010		12/04/2010							
In-s Depth (m)	Type	ing Result	Sam Depth (m)	Type	No.	Description of Strata	Depth (Thick- ness) (m)	Level	Legen
			-			Dark grey black organic sandy SILT with frequent roots. Sand is fine to coarse.	(0.10)	28.32	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
			<u>-</u> - - -			Brown sandy subangular to subrounded fine to medium GRAVEL of schist with occasional subrounded cobbles and rare boulders.	(0.55)		
			<u>-</u> -				0.65	27.77	, ,
			- - -			Dark brown organic sandy SILT with frequent roots. Sand is fine to coarse.	(0.10)	27.67	a
			<u>-</u> - -			Brown medium to coarse SAND and subangular to subrounded fine to coarse GRAVEL of schist with occasional subrounded cobbles.	(0.45)		0 · 0 · 0 · 0 · 0 · 0 · 0 · 0 · 0 · 0 ·
			<u> </u>			End of Trial Pit	1.20	27.22	
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Remarks ₁

(See notes & keysheets)

The walls of the pit were stable to a depth of 1.20m during excavation.

- Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.
 On completion the trial pit was backfilled with compacted arisings.
 Mackintosh probe advanced from the base of the trial pit to refusal. Results presented separately.

Groundwater was not apparent during excavation.

Scale 1:25



Project

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Contract No.

CON103001

Figure No.

FR41 (1 of 1)

Method of Excavation Surface Dimensions Hand dug TRIAL PIT No. TP1 Plan 0.60m x 0.80m 02/01/2010 Coordinates (National Grid) Ground Level **Date Excavated** Start 232080 E End 02/01/2010 709866 N ➤ 000 13.72 m OD Logged by Compiled by Approved by

2/01/1910	2	7/04/1910							
In-s	itu Testi		Sam	ples	,	Description of Strate	Depth (Thick-		Laman
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Level	Legen
			-			TOPSOIL	(0.10)		
			0.20	D	1	MADE GROUND: Composed of grey brown gravelly micaceous fine to coarse sand. Gravel is predominantly subangular and subrounded fine, locally fine and medium, of mixed lithologies including brick and sandstone.	(0.60)	13.62	
			-				<u> </u>		
			0.60	D	2	End of Trial Pit	0.70	13.02	
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Remarks ₁ (See notes & keysheets) 2 3

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings and reinstated.

Groundwater was not apparent during excavation.

Scale 1:25

Project

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Figure No.

FR42 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP2 Plan $0.60m \times 1.20m$ Start End **Date Excavated** Coordinates (Local Grid) Ground Level 24/02/2010 232128 E 24/02/2010 709859 N ➤ 000 12.60 m OD Logged by Compiled by Approved by

24/02/2010	2	26/02/2010	12/04	/2010		}			
In-si	tu Test	ing	Samp	oles	í	Description of Strata	Depth (Thick- ness) Level		Legend
Depth (m)	Type	Result	Depth (m)	Туре	No.	·	(m)		
			0.20-0.40 0.20-0.40 0.40-0.40	D	2 3 1	Grass over MADE GROUND: Composed of brown gravelly organic medium and coarse sand with frequent roots. MADE GROUND: Composed of light brown slightly gravelly medium and coarse sand with occasional roots. Gravel is subangular fine to coarse of schist. At 0.50m: Concrete plinth.	(0.10)	12.50	
						End of Trial Pit			

Remarks ₁ (See notes & keysheets)

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

Trial pit terminated at 0.50m on encountering concrete plinth.

On completion the trial pit was backfilled with compacted arisings. Groundwater was not apparent during excavation.

Scale 1:25

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Figure No.

FR43 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP2A Plan $0.60m \times 1.50m$ Start End **Date Excavated** Coordinates (Local Grid) Ground Level 24/02/2010 232136 E 24/02/2010 709862 N ➤ 000 13.09 m OD Logged by Compiled by Approved by

4/02/2010	2	6/02/2010	12/04/	2010					
	itu Testi	ng	Sampl Depth	es	i	Description of Strata	Depth (Thick- ness)	Level	Legen
(m)	Туре	Result	(m)	Type ES	No.	Grass over MADE GROUND: Composed of dark	(m)		
			0.00-0.20	B D	2	brown organic sandy subangular fine to coarse gravel of schist with frequent roots.	(0.20)	12.89	
			- - - - -			MADE GROUND: Composed of brown and grey sandy angular to subangular fine to coarse gravel of schist, concrete and brick with many angular cobbles. Sand is fine to coarse.	(0.40)		
			[- - -			MADE GROUND: Composed of grey angular medium to coarse gravel.	0.60 (0.20)	12.49	
			- - - -			MADE GROUND: Composed of brown sandy angular to subangular fine to coarse gravel of	0.80	12.29	
			1.10-1.13 1.10-1.13	B ES	4 5	schist and concrete with many subangular cobbles and occasional boulders. Faint diesel odour.	(0.60)		
			- - - -			End of Trial Pit	1.40	11.69	
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Remarks ₁

The walls of the pit were stable during excavation.

(See notes & keysheets) 2 3 Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

Groundwater was not apparent during excavation.

Scale 1:25



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Contract No.

CON103001

Figure No.

FR44 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP3 Plan $0.60m \times 1.50m$ Date Excavated Start End Coordinates (Local Grid) Ground Level 24/02/2010 232159 E 24/02/2010 709837 N ➤ 000 12.01 m OD Logged by Compiled by Approved by

24/02/2010		6/02/2010		1/2010					
In-si	tu Testi		Sam		,	Description of Strata	Depth (Thick-	Level	Legen
Depth (m)	Type	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	200	Legei
			- - - -			Grass over MADE GROUND: Composed of dark brown organic sandy angular fine to medium gravel with frequent roots.	(0.20)	11.81	
			- - - -			MADE GROUND: Composed of grey and dark brown sandy angular fine to coarse gravel of schist, brick, concrete and wood with many subangular cobbles. Sand is fine to coarse.			
							(1.30)		
			<u>-</u> - - - - -						
			- - - 1.50	ES	1	End of Trial Pit	1.50	10.51	
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Remarks ₁ (See notes & keysheets) 2 3

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

Groundwater was not apparent during excavation.

Scale 1:25



Project

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Contract No.

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Figure No.

FR45 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP4 Plan $0.60m \times 1.50m$ Start End Coordinates (Local Grid) Ground Level **Date Excavated** 24/02/2010 232151 E 24/02/2010 709848 N ➤ 000 12.38 m OD Approved by Logged by Compiled by

24/02/2010	2	6/02/2010	12/0	4/2010					
In-situ Depth		_	Sam Depth	1	ſ .	Description of Strata	Depth (Thick- ness)	Level	Legen
(m)	Гуре	Result	(m)	Туре	NO.	Grass over MADE GROUND: Composed of dark brown organic sandy angular to subangular fine to coarse gravel with frequent roots.	(m)		
			0.30	ES B D	1 2 3	MADE GROUND: Composed of light brown and dark brown sandy angular to subangular fine to coarse gravel of schist, concrete and unknown cemented construction material with many angular and subangular cobbles and frequent boulders up to 1.00m. At 0.70m: Large boulder (pit extended eastwards).	0.30	12.08	
							[- (1.40) - - - - - -		
			1.70 1.70 1.70	B ES B	4 5 6	MADE GROUND: Composed of light brown gravelly fine to coarse sand with many angular to subangular cobbles and frequent boulders up to 1.20m. Gravel is angular to subrounded fine to coarse schist, concrete, metal, plastic and brick. At 2.00m: Concrete plinth encountered (Pit extended westwards).	1.70	10.68	
			- - - - - - - - -			extended westwards).	(1.10)		
			-			End of Trial Pit	2.80	9.58	***
			-				- - - - - - - - - -		
			— : : : : : :				- - - - - - - - -		
			- - - - - - -				- - - - - - -		

Remarks ₁ (See notes & keysheets) 2 3

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

Groundwater was not apparent during excavation.

Scale 1:25



Project

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Contract No.

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Figure No.

FR46 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP5 Plan $0.60m \times 1.50m$ Start End **Date Excavated** Coordinates (Local Grid) Ground Level 24/02/2010 232121 E 24/02/2010 709858 N ➤ 000 11.97 m OD Logged by Compiled by Approved by

24/02/2010	2	6/02/2010	12/04	/2010		}			
In-si Depth (m)	tu Testi Type		Samp Depth		No	Description of Strata	Depth (Thick- ness)	Level	Legen
(m)	Турс	resuit	(m)	Турс	110.	MADE GROOUND: Tarmacadam.	(m) [']		
			<u> </u>			MADE GROUND: Composed of grey angular medium to coarse gravel.	0.10	11.87	
			0.30 0.30 - 0.30	ES B D	1 2 3	MADE GROUND: Composed of light brown sandy angular to subangular fine to coarse gravel of schist, brick, concrete, wood and metal with many subangular cobbles and occasional boulders.	0.30	11.67	
			- - - - -			At 0.70m: 2.00m by 0.20m by 0.20 wood.	(0.80)		
			— - - -			End of Trial Pit	1.10	10.87	
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Remarks ₁ (See notes & keysheets)

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out.

Trial pit terminated at 1.10m due to possible bedrock.

On completion the trial pit was backfilled with compacted arisings. Groundwater was not apparent during excavation.

Scale 1:25



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Contract No.

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Figure No.

FR47 (1 of 1)

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP6 Plan $0.60m \times 1.50m$ Start End Coordinates (Local Grid) Ground Level **Date Excavated** 24/02/2010 232130 E 24/02/2010 709839 N ➤ 000 11.80 m OD Logged by Compiled by Approved by

24/02/2010	2	6/02/2010	12/04/	2010					
In-s	itu Testi	ing	Sampl	es			Depth (Thick-		
Depth (m)	Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Level	Legend
			0.30-0.40 0.30-0.40 0.30-0.40	ES B D	1 2 3	Grass over MADE GROUND: Composed of brown organic sandy angular to subrounded fine to coarse gravel. MADE GROUND: Composed of grey slightly silty sandy angular to subangular fine to coarse gravel of schist, concrete, brick and metal with many angular to subangular cobbles and occasional boulders up to 1.00m.	0.30	11.50	
			-				(1.20)		
			1.40	В	4	MADE GROUND: Composed of grey angular cobbles and boulders of schist.	1.50	10.30	
			2.40-3.40	В	5				
							- (2.20) - (2.20) 		
			- - - - - -			End of Trial Pit	3.70	8.10	
			- - - - - - -				-		
							- - - -		

Remarks ₁ (See notes & keysheets) 2 3

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings.

Groundwater was not apparent during excavation.

Scale 1:25

Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

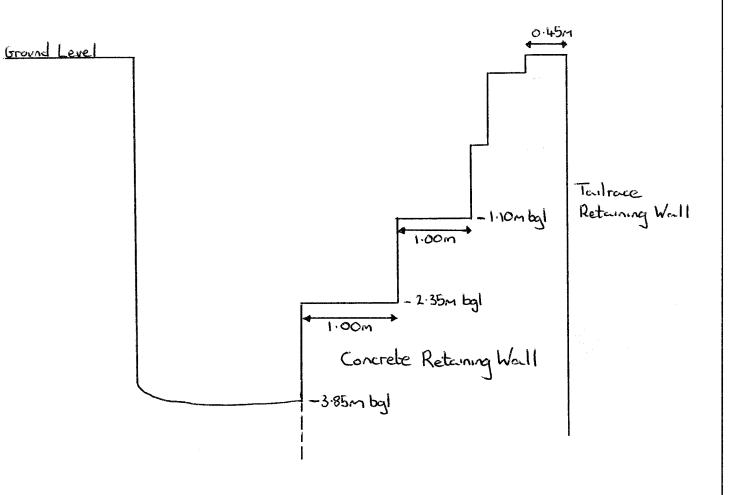
CON103001

Figure No.

FR48 (1 of 1)



TP6



NOT TO SCALE

Method of Excavation Surface Dimensions JCB 3CX TRIAL PIT No. TP7 Plan $0.60m \times 1.40m$ Start 24/02/2010 End 24/02/2010 **Date Excavated** Coordinates (Local Grid) Ground Level 232159 E 24/02/2010 709826 N ➤ 000 11.60 m OD Logged by Compiled by Approved by

2	6/02/2010	12/04/	2010		l l				
itu Testi	ing	Samp	les		•	Depth			
Туре	Result	Depth (m)	Туре	No.	Description of Strata	ness) (m)	Level	Legen	
		0.00-0.10 - 0.20-0.50 0.20-0.50	ES B D	2 3	Grass over MADE GROUND: Composed of brown organic sandy angular fine to coarse gravel with many cobbles and frequent roots. Sand is medium to coarse. MADE GROUND: Composed of grey and brown silty sandy angular to subangular fine to coarse gravel of schist, wood, concrete, metal sheeting, cloth, plastic and coal fragments with many subangular cobbles and occasional boulders up to 1.20m.	(0.10)			
		0.95	ES	4	At 0.95m: Moderate diesel odour.	- - - - -			
		- - - - - 1.50	В	5	At 1.25m: Corrugated metal roofing and wire rope encountered.	- - - -			
		-				- - (3.30)			
		- - - - - - -			At 2.00m: Reinforced concrete beam encountered with dimensions of 2.00m by 0.20m by 0.20m.				
		- - 2.50 - -	В	6		-			
						-			
		3.40	В	7	End of Trial Dit	3.40	8.20		
		-				-			
	itu Testi	itu Testing	Type Result Depth (m) - 0.00-0.10 - 0.20-0.50 - 0.20-0.50	Type Result Depth (m) Type 0.000-0.10 ES 0.20-0.50 D 0.20-0.50 D D D D D D D D D	Type Result Depth (m) Type No. 0.00-0.10	Type Result Depth (m) Type No. 0.00-0.10 ES	Type Result Depth (m) Type No. Cond-0.10 ES 1 Grass over MADE GROUND: Composed of brown organic sandy angular fine to coarse gravel via medium to coarse. Sand is medium to coarse. Sand is medium to coarse gravel of shist, wood, concrete, metal sheeting, cloth, plastic and coal fragments with many cobbles and frequent roots. Sand coarse gravel of schist, wood, concrete, metal sheeting, cloth, plastic and coal fragments with many subangular cobbles and occasional boulders up to 1.20m. At 0.95m: Moderate diesel odour. At 0.95m: Moderate diesel odour. At 1.25m: Corrugated metal roofing and wire rope encountered. At 2.00m: Reinforced concrete beam encountered with dimensions of 2.00m by 0.20m by 0.20m. 3.40	Type	

Remarks ₁ (See notes & keysheets) 2 3

The walls of the pit were stable during excavation.

Prior to excavation a Cable Avoidance Tool (CAT) survey was carried out. On completion the trial pit was backfilled with compacted arisings. Groundwater was encountered at 0.50m during excavation as a rapid water ingress.

Scale 1:25



Project

SLOY PUMPING STATION Scottish and Southern Energy Jacobs Engineering UK Ltd

Contract No.

CON103001

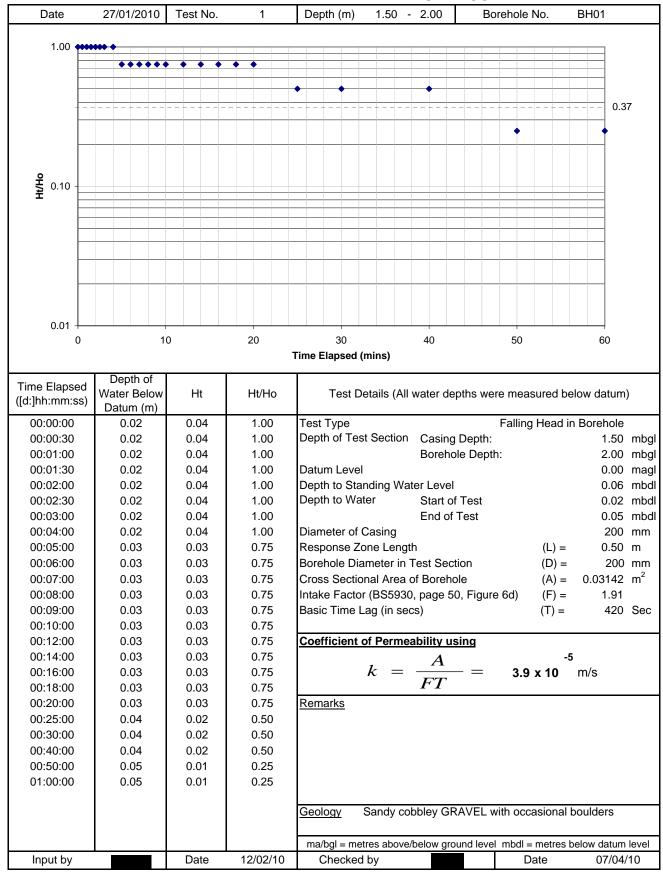
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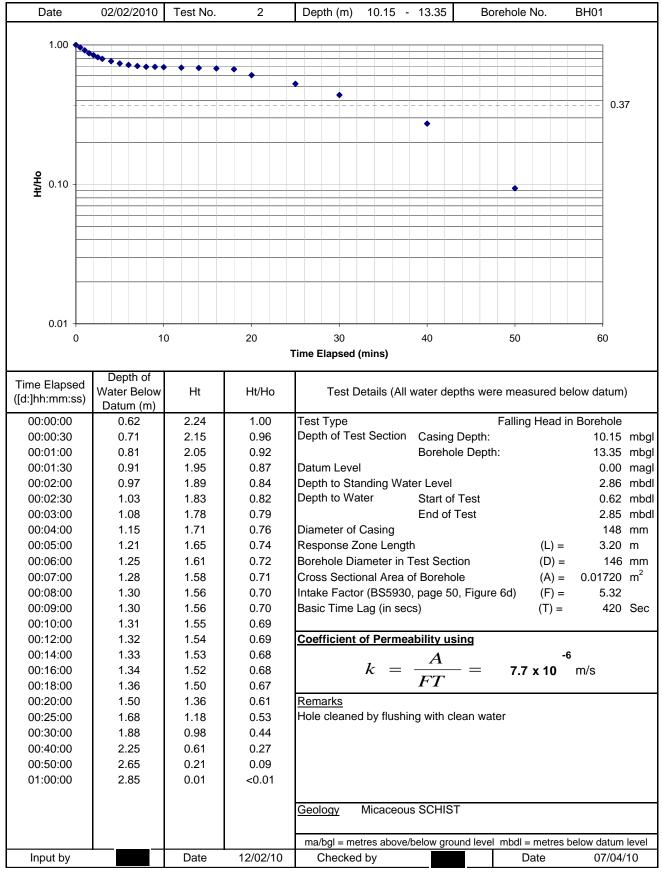
FR49 (1 of 1)

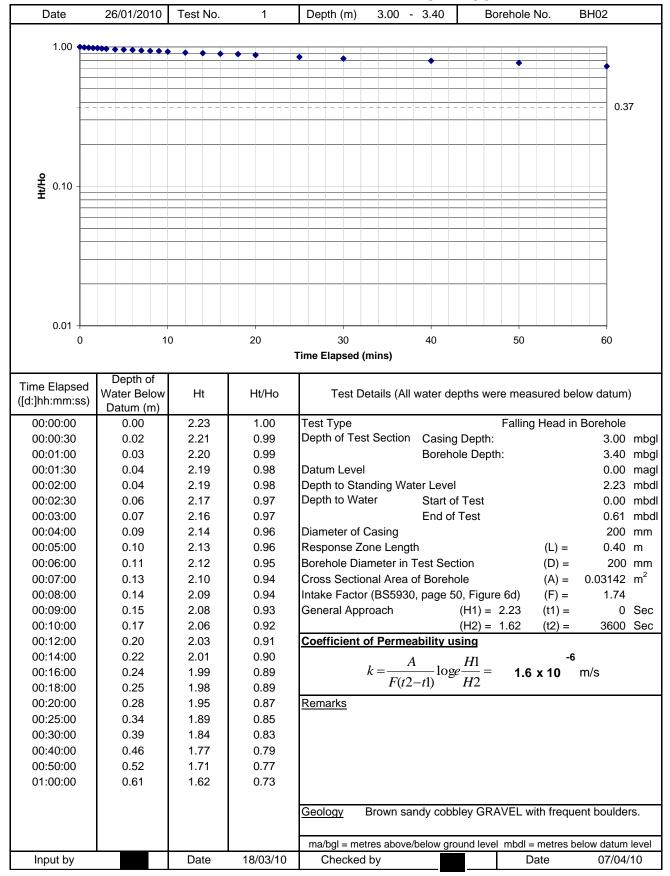


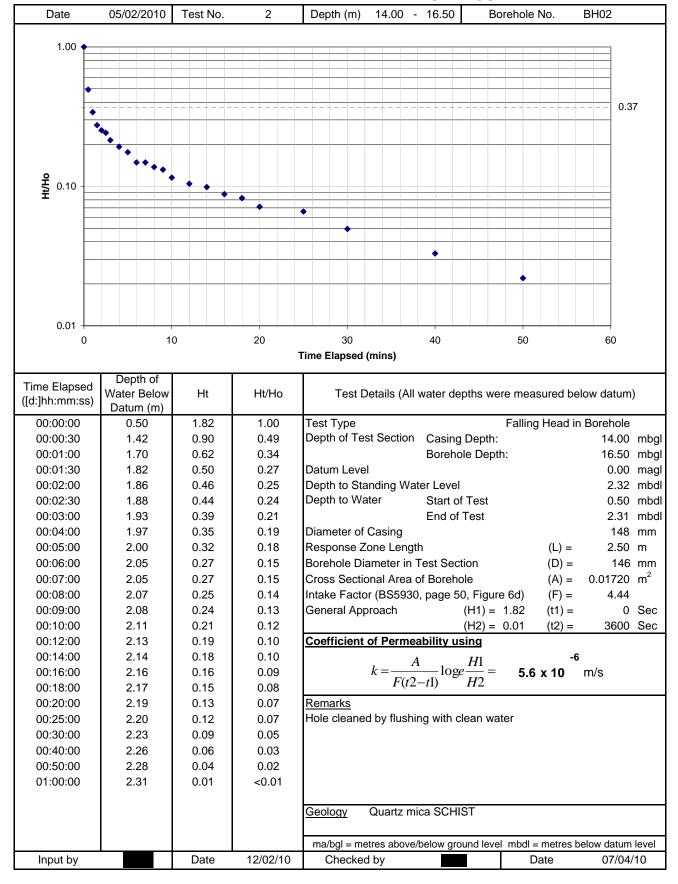
APPENDIX B Field Test Results

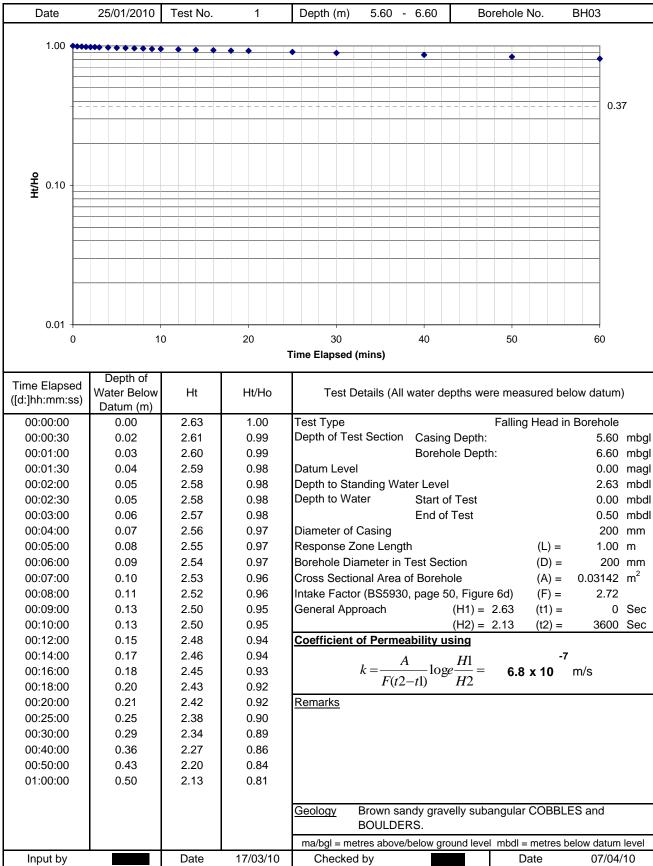
Variable Head Permeability Test Results Packer Permeability Test Results Mackintosh Probe Results Record of Water Levels in Standpipes Figures FT1 to FT11
Figures FT12 to FT29
Figure MP1
Figures GM1 to GM5

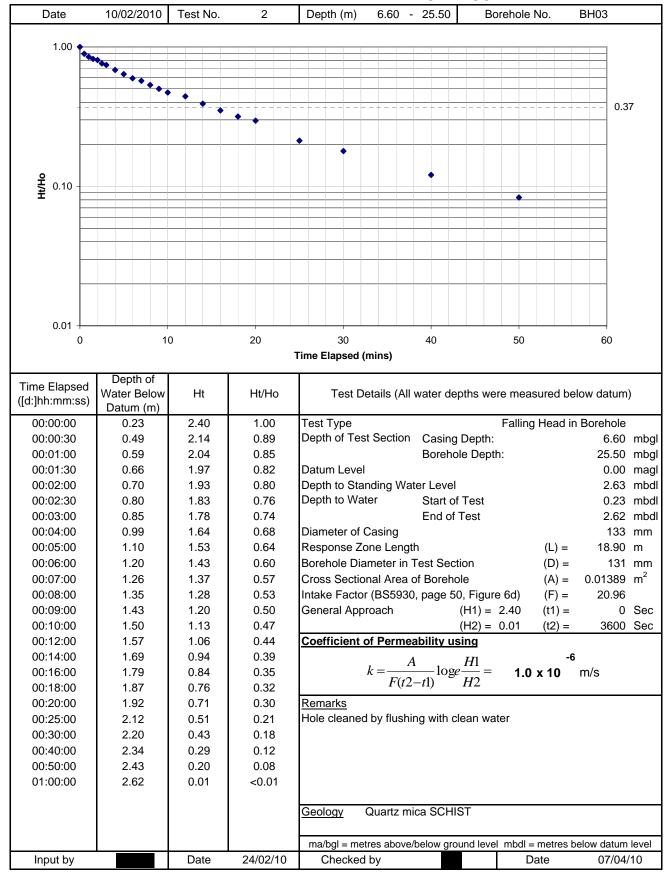


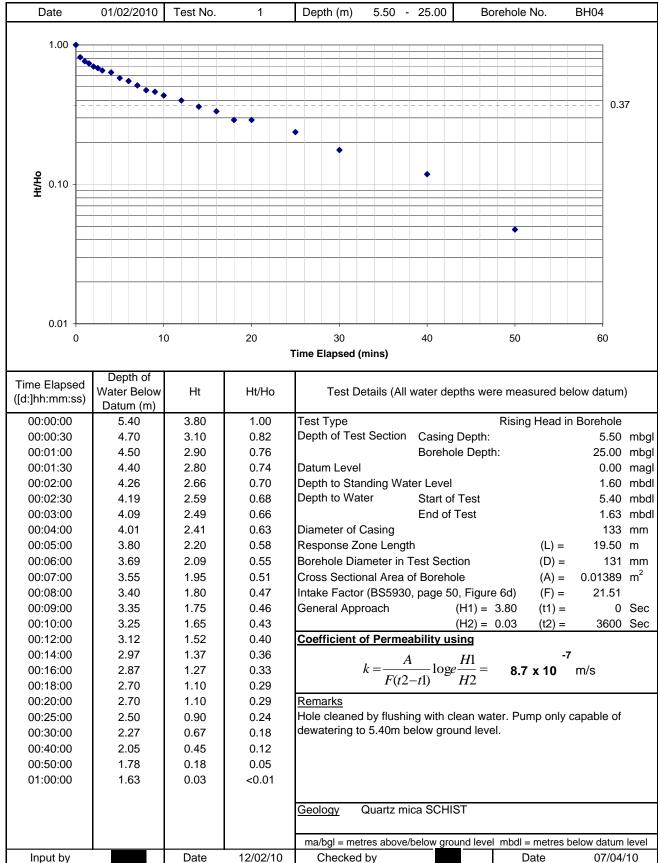


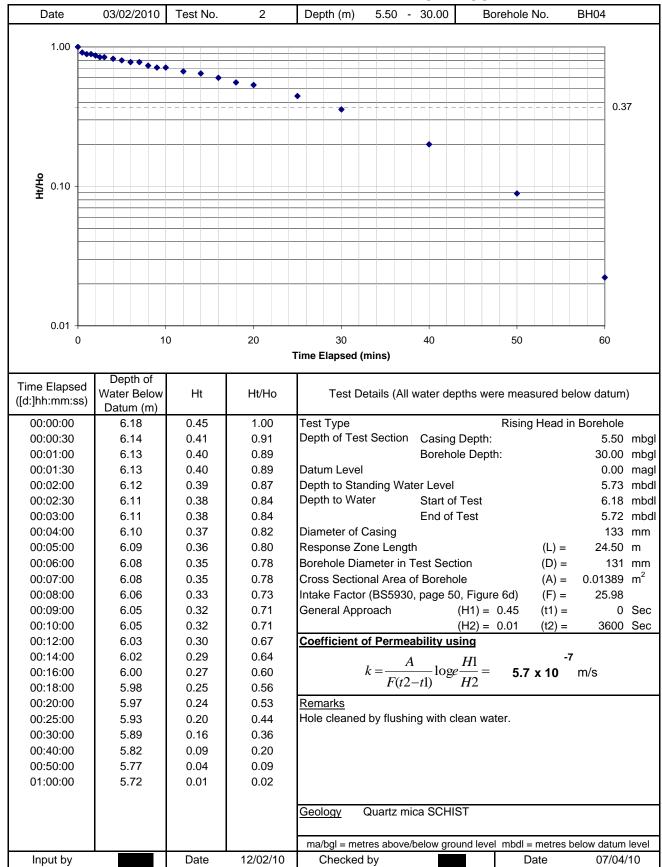


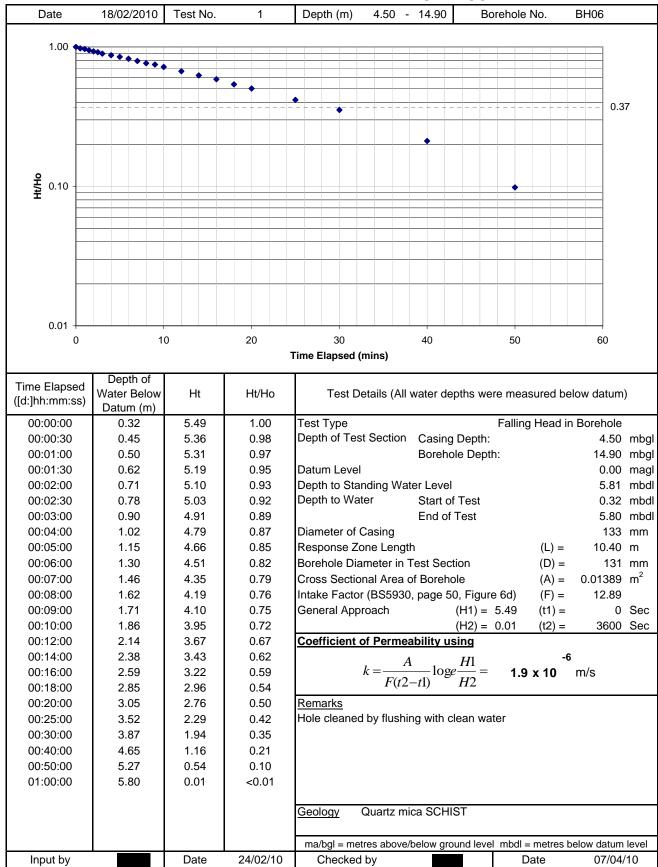


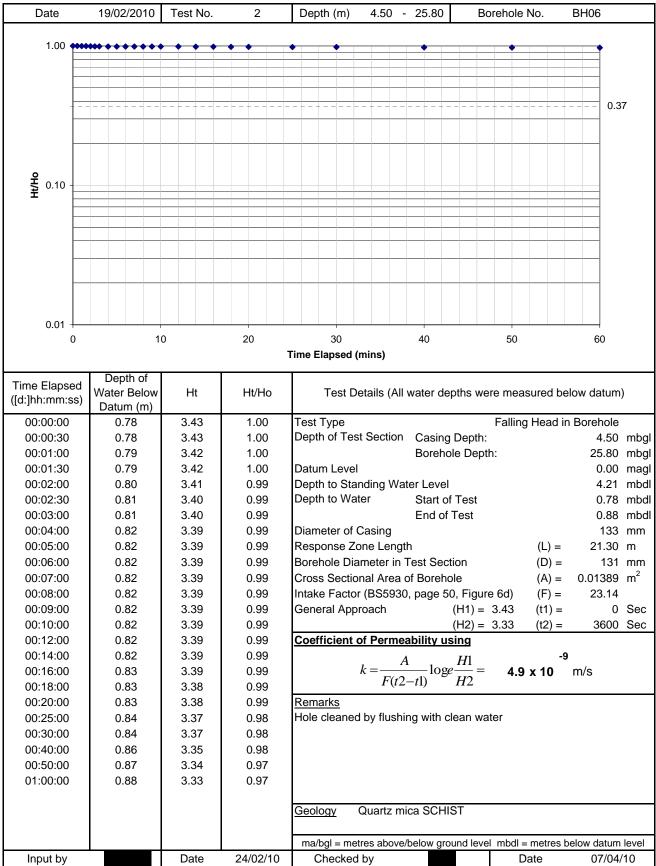


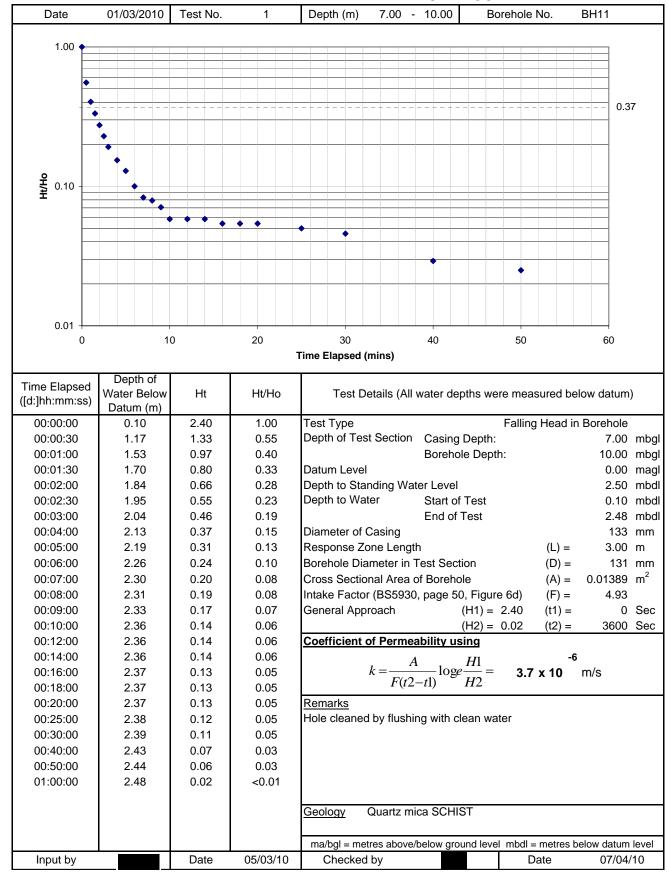














Quartz Mica Schist

PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below ground Level:

Base of Casing: 10.15 Base of Hole: 13.35 Top of Test Section: 12.35

Base of Test Section : 13.35

Initial Groundwater 2.80 m Below Datum Datum Top of Casing

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions N/A Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level 2.80 mbdl Borehole: BH01 Test No: 1

Date: 02/02/2010 Start Time: 12:00pm

2.80

Casing Diameter mm Section Diameter mm 131 Section Length 1.00

Rock Type

Water level after test

Datum Level Site Engineer

Gauge used: 77361-2-09 Flow meter used: 1281900

Number of Rods Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure	J				reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.85		8.7	0	39.185			
			8.7	3	39.185	0	0.0	
			8.7	6	39.184	-1	-0.3	
			8.7	9	39.183	-1	-0.3	
			8.7	12	39.183	0	0.0	
			8.7	15	39.183	0	0.0	0.3
2/200	2.85		17.4	0	39.184			
			17.4	3	39.188	4	1.3	
			17.4	6	39.194	6	2.0	
			17.4	9	39.200	6	2.0	
			17.4	12	39.204	4	1.3	
			17.4	15	39.210	6	2.0	1.7
3/200	2.85		26.1	0	39.246			
			26.1	3	39.251	5	1.7	
			26.1	5	39.258	7	3.5	
			26.1	9	39.265	7	1.7	
			26.1	12	39.272	7	2.3	
			26.1	15	39.279	7	2.3	2.3
4/200	2.85		17.4	0	39.282			
			17.4	3	39.283	1	0.3	
			17.4	5	39.286	3	1.5	
			17.4	9	39.286	0	0.0	
			17.4	12	39.287	1	0.3	
			17.4	15	39.287	0	0.0	0.4
5/200	2.85		8.7	0	39.288			
			8.7	3	39.294	6	2.0	
			8.7	6	39.297	3	1.0	
			8.7	9	39.300	3	1.0	
			8.7	12	39.302	2	0.7	
			8.7	15	39.306	4	1.3	1



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 12.35
Base of Test Section 13.35
Test Section Centre 12.85
Initial Water Depth 2.80
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH01 Date: 02/02/2010 Site Engineer Calculated By Checked By Rock Type

Number of Rods Test Section Length Test No: 1 Start Time: 12:00pm



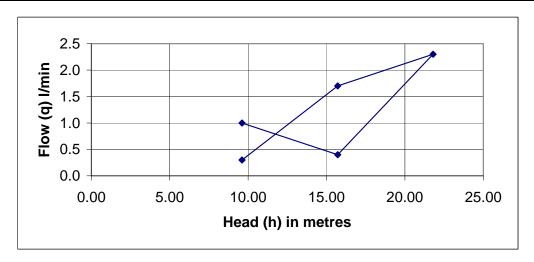
Quartz Mica Schist

5

1.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.3	8.7	6.12	0.70	2.80	0.00	0.00	9.62
2	1.7	17.4	12.23	0.70	2.80	0.02	0.00	15.71
3	2.3	26.1	18.35	0.70	2.80	0.05	0.00	21.80
4	0.4	17.4	12.23	0.70	2.80	0.02	0.00	15.71
5	1.0	8.7	6.12	0.70	2.80	0.00	0.00	9.62



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 8.80 Base of Hole: 12.00 Top of Test Section: 10.00 Base of Test Section: 12.00

Initial Groundwater 3.00

Datum 1.26

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions N/A Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level

3.00 mbdl

m Below Datum

Borehole: BH02 Test No: 01 Date: 05/02/2010 Start Time: 12:00

Casing Diameter mm 150 Section Diameter mm 131 Section Length 2.00

Rock Type Quartz Mica Schist 2.64

Water level after test

Datum Level Site Engineer

Gauge used: 77361-02-09 Flow meter used: 1281900 Number of Rods

Potable Water Quality

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.52		7.3	0	39.840			
			7.3	3	39.860	20	6.7	
			7.3	6	39.870	10	3.3	
			7.3	9	39.879	9	3.0	
			7.3	12	39.891	12	4.0	
			7.3	15	39.899	8	2.7	3.9
2/200	2.52		14.5	0	39.900			
			14.5	3	39.916	16	5.3	
			14.5	6	39.927	11	3.7	
			14.5	9	39.935	8	2.7	
			14.5	12	39.942	7	2.3	
			14.5	15	39.957	15	5.0	3.8
3/200	2.52		21.8	0	39.959			
			21.8	3	39.961	2	0.7	
			21.8	5	39.970	9	4.5	
			21.8	9	39.980	10	2.5	
			21.8	12	39.990	10	3.3	
			21.8	15	39.998	8	2.7	2.7
4/200	2.52		14.5	0	39.999			
			14.5	3	40.004	5	1.7	
			14.5	6	40.010	6	2.0	
			14.5	9	40.012	2	0.7	
			14.5	12	40.015	3	1.0	
			14.5	15	40.016	1	0.3	1.1
5/200	2.52		14.5	0	40.015			
			14.5	3	40.016	1	0.3	
			14.5	6	40.016	0	0.0	
			14.5	9	40.016	0	0.0	
			14.5	12	40.017	1	0.3	
			14.5	15	40.018	1	0.3	0.3



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 10.00
Base of Test Section 12.00
Test Section Centre 11.00
Initial Water Depth 3.00
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH02 Date: 05/02/2010 Site Engineer Calculated By Checked By Rock Type Number of Rods

Start Time: 12:00

Quartz Mica Schist

4

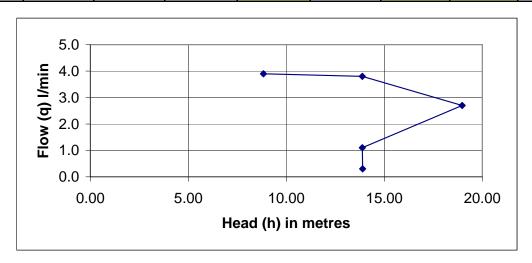
Test No: 01

ber of Rods 4

Test Section Length 2.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	3.9	7.3	5.13	0.70	3.00	0.00	0.00	8.83
2	3.8	14.5	10.19	0.70	3.00	0.02	0.00	13.87
3	2.7	21.8	15.33	0.70	3.00	0.05	0.00	18.98
4	1.1	14.5	10.19	0.70	3.00	0.02	0.00	13.87
5	0.3	14.5	10.19	0.70	3.00	0.00	0.00	13.89



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



Quartz Mica Schist

PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below ground Level:

Base of Casing: 15.80 Base of Hole: 19.50 Top of Test Section: 17.50

Base of Test Section : 19.50

Initial Groundwater 2.19 m Below Datum Datum Top of Casing

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions N/A Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level

2.19 mbdl Borehole: BH02 Test No: 2

Date: 08/02/2010 Start Time: 12:00pm

2.15

Casing Diameter mm Section Diameter mm 131 Section Length 2.00

Rock Type

Water level after test

Datum Level Site Engineer

Gauge used: 77361-2-09 Flow meter used: 1281900

Number of Rods Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1			14.5	0	40.298			
200			14.5	3	40.298	0	0.0	
			14.5	6	40.298	0	0.0	
			14.5	9	40.298	0	0.0	
			14.5	12	40.298	0	0.0	
			14.5	15	40.298	0	0.0	0
2			29.0	0	40.298			
200			29.0	3	40.298	0	0.0	
			29.0	6	40.298	0	0.0	
			29.0	9	40.298	0	0.0	
			29.0	12	40.298	0	0.0	
			29.0	15	40.298	0	0.0	0
3			43.5	0	40.298			
200			43.5	3	40.298	0	0.0	
			43.5	5	40.298	0	0.0	
			43.5	9	40.298	0	0.0	
			43.5	12	40.298	0	0.0	
			43.5	15	40.298	0	0.0	0
4			29.0	0				
200			29.0	3		0	0.0	
			29.0	5		0	0.0	
			29.0	9		0	0.0	
			29.0	12		0	0.0	
			29.0	15		0	0.0	0
5			14.5	0				
200			14.5	3		0	0.0	
			14.5	6		0	0.0	
			14.5	9		0	0.0	
			14.5	12		0	0.0	
			14.5	15		0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level: Top of Test Section 17.50 Base of Test Section 19.50 **Test Section Centre** 18.50 **Initial Water Depth** 2.19 Datum to Ground/Bed 0.00 Datum to Gauge 0.70

Borehole: BH02 Date: 08/02/2010 Site Engineer Calculated By Checked By Rock Type

Number of Rods

Test No: 2

Start Time: 12:00pm

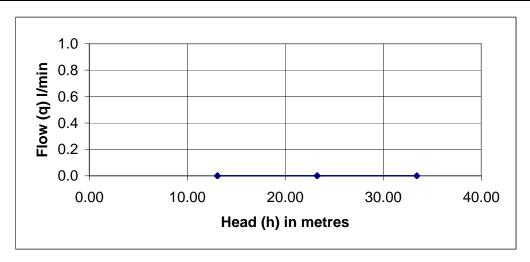


Quartz Mica Schist

Test Section Length 2.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	14.5	10.19	0.70	2.19	0.00	0.00	13.08
2	0.0	29.0	20.39	0.70	2.19	0.02	0.00	23.26
3	0.0	43.5	30.58	0.70	2.19	0.05	0.00	33.42
4	0.0	29.0	20.39	0.70	2.19	0.02	0.00	23.26
5	0.0	14.5	10.19	0.70	2.19	0.00	0.00	13.08



Note

mbdl - metres below datum/deck level

mabl - metres above bed level

PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 0.50 Base of Hole: 25.50 Top of Test Section: 24.00 Base of Test Section:

25.50 **Initial Groundwater**

2.60 Datum 1.40

m Below Datum

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions N/A Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level 2.60 mbdl

Borehole: BH02 Test No: 03 Start Time: 12:00 Date: 09/02/2010

Casing Diameter mm 150 Section Diameter mm 131 Section Length 1.50

Rock Type Quartz Mica Schist 2.63

Water level after test

Datum Level Site Engineer Gauge used: Flow meter used: Number of Rods

77361-02-09

1281900

Water Quality Potable

	•	1		Ī	T			1
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.60		18.1	0	40.330			
			18.1	3	40.340	10	3.3	
			18.1	6	40.345	5	1.7	
			18.1	9	40.351	6	2.0	
			18.1	12	40.354	3	1.0	
			18.1	15	40.357	3	1.0	1
2/200	2.60		36.2	0	40.385			
			36.2	3	40.386	1	0.3	
			36.2	6	40.391	5	1.7	
			36.2	9	40.392	1	0.3	
			36.2	12	40.394	2	0.7	
			36.2	15	40.395	1	0.3	0.7
3/200	2.60		54.4	0	40.397			
			54.4	3	40.402	5	1.7	
			54.4	5	40.407	5	2.5	
			54.4	9	40.414	7	1.8	
			54.4	12	40.418	4	1.3	
			54.4	15	40.420	2	0.7	1.6
4/200	2.60		36.2	0	40.420			
			36.2	3	40.426	6	2.0	
			36.2	6	40.423	-3	-1.0	
			36.2	9	40.426	3	1.0	
			36.2	12	40.429	3	1.0	
			36.2	15	40.433	4	1.3	1.3
5/200	2.60		18.1	0	40.434			
			18.1	3	40.437	3	1.0	
			18.1	6	40.442	5	1.7	
			18.1	9	40.443	1	0.3	
			18.1	12	40.445	2	0.7	
			18.1	15	40.446	1	0.3	1



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 24.00
Base of Test Section 25.50
Test Section Centre 24.75
Initial Water Depth 2.60
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH02 Date: 09/02/2010 Site Engineer Calculated By Checked By Rock Type Number of Rods

Start Time: 12:00

Quartz Mica Schist

Test No: 03

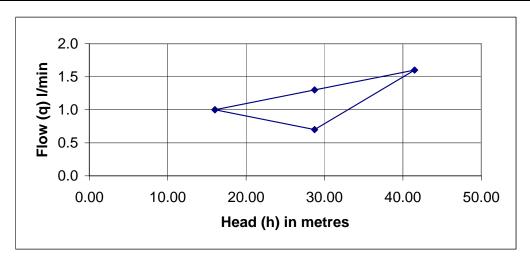
Quartz Mica Sc

4

Test Section Length 1.50 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	1.0	18.1	12.72	0.70	2.60	0.00	0.00	16.02
2	0.7	36.2	25.45	0.70	2.60	0.02	0.00	28.73
3	1.6	54.4	38.24	0.70	2.60	0.05	0.00	41.49
4	1.3	36.2	25.45	0.70	2.60	0.02	0.00	28.73
5	1.0	18.1	12.72	0.70	2.60	0.00	0.00	16.02



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



PACKER TEST FIELD RECORDS

Test Type: Double Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.50
Base of Hole: 35.00
Top of Test Section: 7.00
Base of Test Section: 10.00

Initial Groundwater 1.70

Datum 1.16

m Below Datum

Datum to Bed Level :

Gauge Height Above Datum 0.70

Tank Dimensions
Type of Drill Rods
Packer Type / Ref
Pneumatic

Assumed Standing Water Level 1.70 mbdl

Borehole: BH02 Test No: 4
Date: 10/02/10 Start Time:

Date: 10/02/10 Start Time: 12:00
Casing Diameter mm
150

Section Diameter mm Section Length 3.00

Rock Type Quartz Mica Schist

77361-2-09

Water level after test 2.00

Datum Level Site Engineer Gauge used :

Flow meter used : 1281900 Number of Rods 3

Water Quality Potable

					_			ı
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	1.70		7.3	0	40.701			
			7.3	3	40.702	1	0.3	
			7.3	6	40.702	0	0.0	
			7.3	9	40.702	0	0.0	
			7.3	12	40.702	0	0.0	
			7.3	15	40.702	0	0.0	0
2/200	1.70		14.5	0	40.703			
			14.5	3	40.703	0	0.0	
			14.5	6	40.703	0	0.0	
			14.5	9	40.703	0	0.0	
			14.5	12	40.703	0	0.0	
			14.5	15	40.703	0	0.0	0
3/200	1.70		21.8	0	40.704			
			21.8	3	40.704	0	0.0	
			21.8	5	40.705	1	0.5	
			21.8	9	40.705	0	0.0	
			21.8	12	40.706	1	0.3	
			21.8	15	40.706	0	0.0	0.2
4/200	1.70		14.5	0	40.706			
			14.5	3	40.706	0	0.0	
			14.5	6	40.706	0	0.0	
			14.5	9	40.706	0	0.0	
			14.5	12	40.706	0	0.0	
			14.5	15	40.706	0	0.0	0
5/200	1.70		7.3	0	40.706			
			7.3	3	40.706	0	0.0	
			7.3	6	40.706	0	0.0	
			7.3	9	40.706	0	0.0	
			7.3	12	40.706	0	0.0	
								0
			7.3	15	40.706	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Double Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 7.00
Base of Test Section 10.00
Test Section Centre 8.50
Initial Water Depth 1.70
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH02 Date: 10/02/10 Site Engineer Calculated By Checked By Rock Type Number of Rods

Start Time: 12:00

Test No: 4

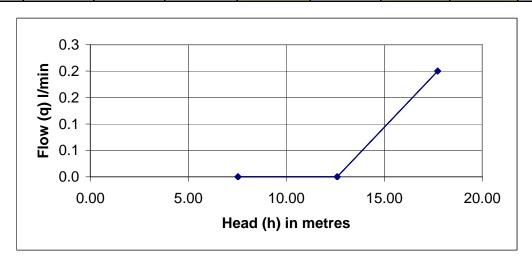
Quartz Mica Schist

3

Test Section Length 3.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	7.3	5.13	0.70	1.70	0.00	0.00	7.53
2	0.0	14.5	10.19	0.70	1.70	0.00	0.00	12.59
3	0.2	21.8	15.33	0.70	1.70	0.00	0.00	17.73
4	0.0	14.5	10.19	0.70	1.70	0.00	0.00	12.59
5	0.0	7.3	5.13	0.70	1.70	0.00	0.00	7.53



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



PACKER TEST FIELD RECORDS

Test Type: Double Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.50 Base of Hole: 12.00 Top of Test Section: 10.00 Base of Test Section:

12.00

Initial Groundwater 2.62 m Below Datum

Datum 0.92

Datum to Bed Level: Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level 2.62 mbdl

Borehole: BH03 Test No: 1 Date: 05/02/10

Start Time: 12:00 Casing Diameter mm 150

Section Diameter mm 131 Section Length 2.00

Rock Type Quartz Mica Schist

Water level after test 2.62

Datum Level Site Engineer Gauge used:

77361-2-09 Flow meter used: 1281900 Number of Rods

Potable Water Quality

F.	•	T .			•			
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.62		7.3	0	40.298			
			7.3	3	40.298	0	0.0	
			7.3	6	40.298	0	0.0	
			7.3	9	40.298	0	0.0	
			7.3	12	40.298	0	0.0	
			7.3	15	40.298	0	0.0	0
2/200	2.62		14.5	0	40.298			
			14.5	3	40.298	0	0.0	
			14.5	6	40.298	0	0.0	
			14.5	9	40.298	0	0.0	
			14.5	12	40.298	0	0.0	
			14.5	15	40.298	0	0.0	0
3/200	2.62		21.3	0	40.298			
			21.3	3	40.298	0	0.0	
			21.3	5	40.298	0	0.0	
			21.3	9	40.298	0	0.0	
			21.3	12	40.298	0	0.0	
			21.3	15	40.298	0	0.0	0
4/200	2.62		14.5	0	40.298			
			14.5	3	40.298	0	0.0	
			14.5	6	40.298	0	0.0	
			14.5	9	40.298	0	0.0	
			14.5	12	40.298	0	0.0	
			14.5	15	40.298	0	0.0	0
5/200	2.62		7.3	0	40.298			
			7.3	3	40.298	0	0.0	
			7.3	6	40.298	0	0.0	
			7.3	9	40.298	0	0.0	
			7.3	12	40.298	0	0.0	
			7.3	15	40.298	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Double Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 10.00
Base of Test Section 12.00
Test Section Centre 11.00
Initial Water Depth 2.62
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH03 Date: 05/02/10 Site Engineer Calculated By Checked By Rock Type

Number of Rods Test Section Length Test No: 1 Start Time: 12:00

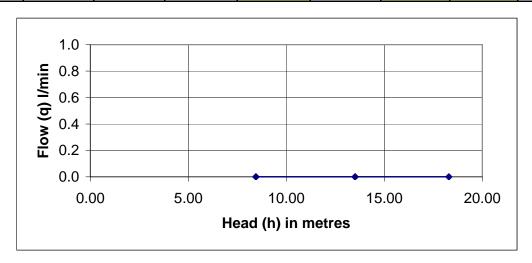
Quartz Mica Schist

5

2.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	7.3	5.13	0.70	2.62	0.00	0.00	8.45
2	0.0	14.5	10.19	0.70	2.62	0.00	0.00	13.51
3	0.0	21.3	14.97	0.70	2.62	0.00	0.00	18.29
4	0.0	14.5	10.19	0.70	2.62	0.00	0.00	13.51
5	0.0	7.3	5.13	0.70	2.62	0.00	0.00	8.45



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.50 Base of Hole: 20.00 Top of Test Section: 18.00 Base of Test Section: 20.00

Initial Groundwater 2.27

Datum 0.25

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref

Pneumatic

Assumed Standing Water Level 2.27 mbdl

m Below Datum

Borehole: BH03 Test No: 2

Start Time: 12:00 Date: 09/02/10 Casing Diameter mm 150

Section Diameter mm 131 Section Length 2.00

Rock Type Quartz Mica Schist

Water level after test 2.26

Datum Level Site Engineer

Gauge used: 77361-2-09 Flow meter used: 1281900

Number of Rods Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.27		14.5	0	40.472			
			14.5	3	40.473	1	0.3	
			14.5	6	40.496	23	7.7	
			14.5	9	40.502	6	2.0	
			14.5	12	40.510	8	2.7	
			14.5	15	40.518	8	2.7	3.1
2/200	2.27		29.0	0	40.521			
			29.0	3	40.529	8	2.7	
			29.0	6	40.539	10	3.3	
			29.0	9	40.548	9	3.0	
			29.0	12	40.558	10	3.3	
			29.0	15	40.564	6	2.0	2.8
3/200	2.27		56.5	0	40.683			
			56.5	3	40.684	1	0.3	
			56.5	5	40.686	2	1.0	
			56.5	9	40.686	0	0.0	
			56.5	12	40.687	1	0.3	
			56.5	15	40.688	1	0.3	0.9
4/200	2.27		29.0	0	40.688			
			29.0	3	40.689	1	0.3	
			29.0	6	40.689	0	0.0	
			29.0	9	40.690	1	0.3	
			29.0	12	40.690	0	0.0	
			29.0	15	40.691	1	0.3	0.2
5/200	2.27		14.5	0	40.691			
			14.5	3	40.690	-1	-0.3	
			14.5	6	40.690	0	0.0	
			14.5	9	40.690	0	0.0	
			14.5	12	40.691	1	0.3	
			14.5	15	40.691	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 18.00
Base of Test Section 20.00
Test Section Centre 19.00
Initial Water Depth 2.27
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH03 Date: 09/02/10 Site Engineer Calculated By Checked By Rock Type

Rock Type
Number of Rods
Test Section Length

Test No: 2
Start Time: 12:00

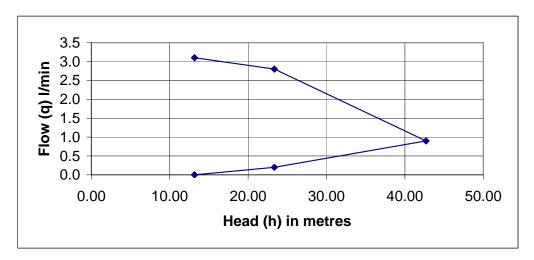
Quartz Mica Schist

6

2.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	3.1	14.5	10.19	0.70	2.27	0.00	0.00	13.16
2	2.8	29.0	20.39	0.70	2.27	0.00	0.00	23.36
3	0.9	56.5	39.72	0.70	2.27	0.00	0.00	42.69
4	0.2	29.0	20.39	0.70	2.27	0.00	0.00	23.36
5	0.0	14.5	10.19	0.70	2.27	0.00	0.00	13.16



Note

mbdl - metres below datum/deck level

mabl - metres above bed level



PACKER TEST FIELD RECORDS

Test Type: Double Water Injection Packer

Depth Below Bed Level:

Base of Casing: 6.60 Base of Hole: 30.00 Top of Test Section: 20.00 Base of Test Section: 23.00

Initial Groundwater 0.22

Datum

0.90

m Below Datum

Datum to Bed Level: Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm

Pneumatic Packer Type / Ref

Assumed Standing Water Level 0.22 mbdl Borehole: BH03 Test No: 3

Date: 11/02/10 Start Time: 12:00 Casing Diameter mm 150

Section Diameter mm 131 Section Length 3.00

Rock Type Quartz Mica Schist

Water level after test 0.23

Datum Level Site Engineer

Gauge used: 77361-2-09 Flow meter used: 1281900

Number of Rods 7

Water Quality Potable

					-			-
Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	0.22		14.5	0	104.849			
			14.5	3	104.849	0	0.0	
			14.5	6	104.849	0	0.0	
			14.5	9	104.849	0	0.0	
			14.5	12	104.849	0	0.0	
			14.5	15	104.849	0	0.0	0
2/200	0.22		29.0	0	104.849			
			29.0	3	104.849	0	0.0	
			29.0	6	104.849	0	0.0	
			29.0	9	104.849	0	0.0	
			29.0	12	104.849	0	0.0	
			29.0	15	104.849	0	0.0	0
3/200	0.22		43.5	0	104.849			
			43.5	3	104.849	0	0.0	
			43.5	5	104.849	0	0.0	
			43.5	9	104.849	0	0.0	
			43.5	12	104.849	0	0.0	
			43.5	15	104.849	0	0.0	0
4/200	0.22		29.0	0	104.849			
			29.0	3	104.849	0	0.0	
			29.0	6	104.849	0	0.0	
			29.0	9	104.849	0	0.0	
			29.0	12	104.849	0	0.0	
			29.0	15	104.849	0	0.0	0
5/200	0.22		14.5	0	104.849			
			14.5	3	104.849	0	0.0	
			14.5	6	104.849	0	0.0	
			14.5	9	104.849	0	0.0	
			14.5	12	104.849	0	0.0	
			14.5	15	104.849	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Double Water Injection Packer

Depth Below Ground/Bed Level: Top of Test Section 20.00 Base of Test Section 23.00 **Test Section Centre** 21.50 **Initial Water Depth** 0.22 Datum to Ground/Bed 0.00 Datum to Gauge 0.70

Borehole: BH03 Date: 11/02/10 Site Engineer Calculated By Checked By Rock Type

Number of Rods

Test No: 3 Start Time: 12:00



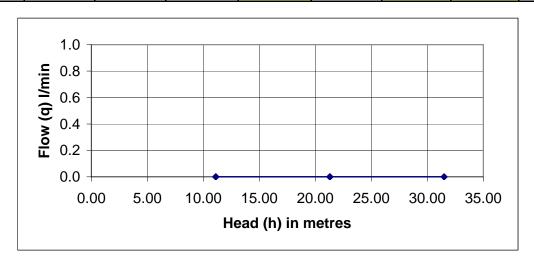
Quartz Mica Schist

Test Section Length

3.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	14.5	10.19	0.70	0.22	0.00	0.00	11.11
2	0.0	29.0	20.39	0.70	0.22	0.00	0.00	21.31
3	0.0	43.5	30.58	0.70	0.22	0.00	0.00	31.50
4	0.0	29.0	20.39	0.70	0.22	0.00	0.00	21.31
5	0.0	14.5	10.19	0.70	0.22	0.00	0.00	11.11



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 5.50
Base of Hole: 11.10
Top of Test Section: 9.50

Base of Test Section: 11.10

Initial Groundwater 2.20 m Below Datum
Datum Top of Casing

Datum Top of Casing
Datum to Bed Level:
Gauge Height Above Datum
6.00
0.70

Tank Dimensions N/A
Type of Drill Rods 25mm
Packer Type / Ref Pneumatic

Assumed Standing Water Level 2.20 mbdl

Borehole: BH04 Test No: 1

Date: 26/01/2010 Start Time: 12:00pm
Casing Diameter mm 150

77361-2-09

Section Diameter mm 131
Section Length 1.60

Rock Type Quartz Mica Schist

Water level after test 2.20

Datum Level Site Engineer Gauge used :

Flow meter used : 1281900 Number of Rods 3

Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.20		7.2	0	103.208			
			7.2	3	103.222	14	4.7	
			7.2	6	103.335	113	37.7	
			7.2	9	103.365	30	10.0	
			7.2	12	103.390	25	8.3	
			7.2	15	103.336	-54	-18.0	16
2/200	2.20		14.5	0	103.365			
			14.5	3	103.376	11	3.7	
			14.5	6	103.381	5	1.7	
			14.5	9	103.399	18	6.0	
			14.5	12	103.408	9	3.0	
			14.5	15	103.416	8	2.7	4
3/200	2.20		29.0	0	103.318			
			29.0	3	103.318	0	0.0	
			29.0	5	103.318	0	0.0	
			29.0	9	103.318	0	0.0	
			29.0	12	103.318	0	0.0	
			29.0	15	103.318	0	0.0	0
4/200	2.20		14.5	0	103.319			
			14.5	3	103.326	7	2.3	
			14.5	5	103.329	3	1.5	
			14.5	9	103.335	6	1.5	
			14.5	12	103.338	3	1.0	
			14.5	15	103.339	1	0.3	2
5/200	2.20		7.2	0	103.331			
			7.2	3	103.337	6	2.0	
			7.2	6	103.341	4	1.3	
			7.2	9	103.354	13	4.3	
			7.2	12	103.359	5	1.7	
			7.2	15	103.459	100	33.3	9



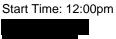
PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 9.50
Base of Test Section 11.10
Test Section Centre 10.30
Initial Water Depth 2.20
Datum to Ground/Bed 6.00
Datum to Gauge 0.70

Borehole: BH04 Date: 26/01/2010 Site Engineer Calculated By Checked By Rock Type

Rock Type Number of Rods Test Section Length Test No: 1



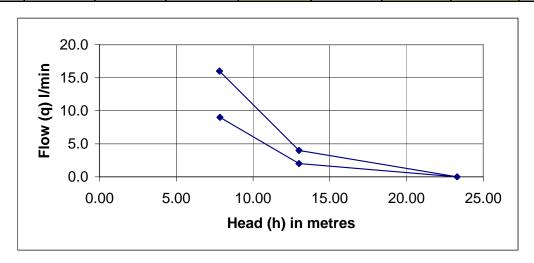
Quartz Mica Schist

3

1.60 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	16.0	7.2	5.06	0.70	2.20	0.15	0.00	7.81
2	4.0	14.5	10.19	0.70	2.20	0.10	0.00	12.99
3	0.0	29.0	20.39	0.70	2.20	0.00	0.00	23.29
4	2.0	14.5	10.19	0.70	2.20	0.10	0.00	12.99
5	9.0	7.2	5.06	0.70	2.20	0.11	0.00	7.85



Note

mbdl - metres below datum/deck level



Quartz Mica Schist

1.98

PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 5.50 Base of Hole: 20.50 Top of Test Section: 16.50 Base of Test Section :

20.50

Initial Groundwater 1.99 m Below Datum Datum Top of Casing

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions N/A Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level

1.99 mbdl Borehole: BH04 Test No: 2

Date: 29/01/2010 Start Time: 12:00pm

Casing Diameter mm Section Diameter mm 131 Section Length 4.00

Rock Type

Water level after test

Datum Level Site Engineer

Gauge used: 77361-2-09 Flow meter used: 1281900

Number of Rods Water Quality Potable

Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.00		14.5	0	39.229			
			14.5	3	39.331	102	34.0	
			14.5	6	39.333	2	0.7	
			14.5	9	39.334	1	0.3	
			14.5	12	39.336	2	0.7	
			14.5	15	39.337	1	0.3	0.5
2/200	2.00		29.1	0	39.238			
			29.1	3	39.240	2	0.7	
			29.1	6	39.242	2	0.7	
			29.1	9	39.244	2	0.7	
			29.1	12	39.246	2	0.7	
			29.1	15	39.248	2	0.7	0.7
3/200	2.00		58.0	0	39.268			
			58.0	3	39.273	5	1.7	
			58.0	5	39.274	1	0.5	
			58.0	9	39.280	6	1.5	
			58.0	12	39.284	4	1.3	
			58.0	15	39.288	4	1.3	1.3
4/200	2.00		29.1	0	39.305			
			29.1	3	39.307	2	0.7	
			29.1	5	39.310	3	1.5	
			29.1	9	39.312	2	0.5	
			29.1	12	39.315	3	1.0	
			29.1	15	39.317	2	0.7	0.7
5/200	2.00		14.5	0	39.318			
			14.5	3	39.320	2	0.7	
			14.5	6	39.321	1	0.3	
			14.5	9	39.323	2	0.7	
			14.5	12	39.325	2	0.7	
			14.5	15	39.326	1	0.3	0.5



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level: Top of Test Section 16.50 Base of Test Section 20.50 **Test Section Centre** 18.50 **Initial Water Depth** 1.99 Datum to Ground/Bed 0.00 Datum to Gauge 0.70

Borehole: BH04 Date: 29/01/2010 Site Engineer Calculated By Checked By Rock Type Number of Rods

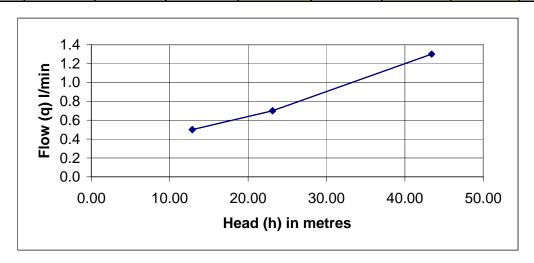
Start Time: 12:00pm Quartz Mica Schist

Test No: 2

Test Section Length 4.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.5	14.5	10.19	0.70	1.99	0.00	0.00	12.88
2	0.7	29.1	20.46	0.70	1.99	0.02	0.00	23.13
3	1.3	58.0	40.77	0.70	1.99	0.05	0.00	43.41
4	0.7	29.1	20.46	0.70	1.99	0.02	0.00	23.13
5	0.5	14.5	10.19	0.70	1.99	0.00	0.00	12.88



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Double Water Injection Packer

Depth Below Bed Level:

Base of Casing: 5.50
Base of Hole: 35.00
Top of Test Section: 22.00
Base of Test Section: 25.00

Base of Test Section: 25.00 Initial Groundwater 0.89

Initial Groundwater 0.89 Datum 0.45

0.45

m Below Datum

Datum to Bed Level :
Gauge Height Above Datum

0.70

Tank Dimensions NA
Type of Drill Rods 25mm
Packer Type / Ref Pneumatic

Assumed Standing Water Level 0.89 mbdl

Borehole: BH04 Test No: 3
Date: 12/02/10 Start Time: 12:00

Casing Diameter mm 150
Section Diameter mm 131

Section Length 3.00

Rock Type Quartz Mica Schist

Water level after test 0.96

Datum Level Site Engineer Gauge used :

Gauge used : 77361-2-09
Flow meter used : 1281900

Number of Rods 8 Water Quality Potable

	_				•			
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	0.89		18.1	0	105.952			
			18.1	3	105.964	12	4.0	
			18.1	6	105.977	13	4.3	
			18.1	9	106.004	27	9.0	
			18.1	12	106.020	16	5.3	
			18.1	15	106.042	22	7.3	6
2/200	0.89		36.2	0	106.061			
			36.2	3	106.068	7	2.3	
			36.2	6	106.072	4	1.3	
			36.2	9	106.073	1	0.3	
			36.2	12	106.073	0	0.0	
			36.2	15	106.073	0	0.0	0.8
3/200	0.89		54.4	0	106.085			
			54.4	3	106.085	0	0.0	
			54.4	5	106.085	0	0.0	
			54.4	9	106.085	0	0.0	
			54.4	12	106.085	0	0.0	
			54.4	15	106.085	0	0.0	0
4/200	0.89		36.2	0	106.086			
			36.2	3	106.086	0	0.0	
			36.2	6	106.086	0	0.0	
			36.2	9	106.086	0	0.0	
			36.2	12	106.086	0	0.0	
			36.2	15	106.086	0	0.0	0
5/200	0.89		18.1	0	106.086			
5.200	2.00		18.1	3	106.087	1	0.3	
			18.1	6	106.087	0	0.0	
			18.1	9	106.088	1	0.3	
			18.1	12	106.088	0	0.0	
			18.1	15	106.088	0	0.0	0
			10.1	10	100.000	J	0.0	U



PACKER TEST CALCULATIONS

Test Type: Double Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 22.00
Base of Test Section 25.00
Test Section Centre 23.50
Initial Water Depth 0.89
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH04 Date: 12/02/10 Site Engineer Calculated By Checked By Rock Type

Quartz Mica Schist

Start Time: 12:00

Test No: 3

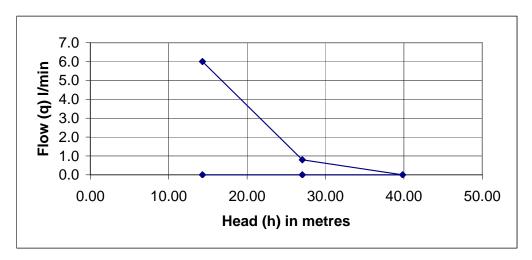
Quartz Mica Sci

Number of Rods 8

Test Section Length 3.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	6.0	18.1	12.72	0.70	0.89	0.00	0.00	14.31
2	0.8	36.2	25.45	0.70	0.89	0.00	0.00	27.04
3	0.0	54.4	38.24	0.70	0.89	0.00	0.00	39.83
4	0.0	36.2	25.45	0.70	0.89	0.00	0.00	27.04
5	0.0	18.1	12.72	0.70	0.89	0.00	0.00	14.31



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 5.50
Base of Hole: 8.80
Top of Test Section: 6.00

Base of Test Section: 8.80
Initial Groundwater 3.20

Initial Groundwater 3.20 Datum 1.20

Datum to Bed Level :

Gauge Height Above Datum 0.70

Tank Dimensions
Type of Drill Rods
Packer Type / Ref
Pneumatic

Assumed Standing Water Level 3.20 mbdl

m Below Datum

Borehole: BH06 Test No: 1

Date: 17/02/10 Start Time: 12:00
Casing Diameter mm 150

Section Diameter mm 131
Section Length 2.80

Rock Type Quartz Mica Schist

Water level after test 3.21

Datum Level Site Engineer

Gauge used : 77361-1-00
Flow meter used : 1281900

Number of Rods 2 Water Quality Potable

	1					1		
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	3.20		7.0	0	105.101			
			7.0	3	105.101	0	0.0	
			7.0	6	105.101	0	0.0	
			7.0	9	105.101	0	0.0	
			7.0	12	105.101	0	0.0	
			7.0	15	105.101	0	0.0	0
2/200	3.20		14.5	0	105.112			
			14.5	3	105.115	3	1.0	
			14.5	6	105.119	4	1.3	
			14.5	9	105.121	2	0.7	
			14.5	12	105.125	4	1.3	
			14.5	15	105.128	3	1.0	1
3/200	3.20		22.5	0	105.158			
			22.5	3	105.160	2	0.7	
			22.5	5	105.164	4	2.0	
			22.5	9	105.174	10	2.5	
			22.5	12	105.180	6	2.0	
			22.5	15	105.199	19	6.3	2
4/200	3.20		14.5	0	105.193			
	0.00		14.5	3	105.196	3	1.0	
			14.5	6	105.198	2	0.7	
			14.5	9	105.203	5	1.7	
			14.5	12	105.203	0	0.0	
			14.5	15	105.204	1	0.3	1
					. 55.25 7	·		
5/200	3.20		7.0	0	105.207			
0,200	0.20		7.0	3	105.207	0	0.0	
			7.0	6	105.207	0	0.0	
			7.0	9	105.207	0	0.0	
			7.0	12	105.207	0	0.0	
			7.0	15	105.207	0	0.0	0
			7.0	10	103.207	U	0.0	U



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 6.00
Base of Test Section 8.80
Test Section Centre 7.40
Initial Water Depth 3.20
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH06 Date: 17/02/10 Site Engineer Calculated By Checked By Rock Type Number of Rods

Start Time: 12:00

Test No: 1

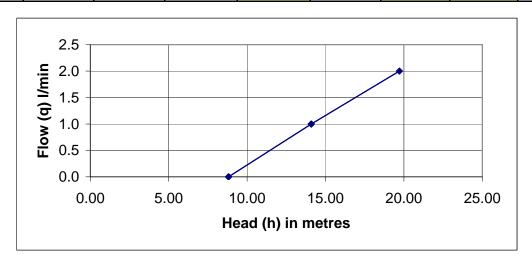
Quartz Mica Schist

2

Test Section Length 2.80 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	7.0	4.92	0.70	3.20	0.00	0.00	8.82
2	1.0	14.5	10.19	0.70	3.20	0.00	0.00	14.09
3	2.0	22.5	15.82	0.70	3.20	0.00	0.00	19.72
4	1.0	14.5	10.19	0.70	3.20	0.00	0.00	14.09
5	0.0	7.0	4.92	0.70	3.20	0.00	0.00	8.82



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 5.50 Base of Hole: 19.60 Top of Test Section: 13.00 Base of Test Section: 19.60

Initial Groundwater m Below Datum 3.41

Datum 1.20

Datum to Bed Level: Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref Pneumatic

Assumed Standing Water Level 3.41 mbdl

Borehole: BH06 Test No: 2 Date: 18/02/10 Start Time: 12:00

Casing Diameter mm 150 Section Diameter mm 131 Section Length 6.60

Rock Type **Quartz Mica Schist** 3.41

Water level after test

Datum Level Site Engineer

Gauge used: 77361-1-00 Flow meter used: 1281900

Number of Rods Water Quality Potable

01/	Day-t1	DA7-411	0		FI.	1147-1 T-1	F1.	
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure					reading	Proces	Programme describe	Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	3.41		14.5	0	105.213			
			14.5	3	105.213	0	0.0	
			14.5	6	105.213	0	0.0	
			14.5	9	105.213	0	0.0	
			14.5	12	105.213	0	0.0	_
			14.5	15	105.213	0	0.0	0
2/200	3.41		29.0	0	105.217			
			29.0	3	105.217	0	0.0	
			29.0	6	105.217	0	0.0	
			29.0	9	105.218	1	0.3	
			29.0	12	105.218	0	0.0	
			29.0	15	105.218	0	0.0	0
3/200	3.41		43.5	0	105.211			
			43.5	3	105.212	1	0.3	
			43.5	5	105.214	2	1.0	
			43.5	9	105.217	3	0.8	
			43.5	12	105.219	2	0.7	
			43.5	15	105.222	3	1.0	0.8
4/200	3.41		29.0	0	105.224			
			29.0	3	105.228	4	1.3	
			29.0	6	105.233	5	1.7	
			29.0	9	105.237	4	1.3	
			29.0	12	105.242	5	1.7	
			29.0	15	105.244	2	0.7	1.3
5/200	3.41		14.5	0	105.244			
			14.5	3	105.239	-5	-1.7	
			14.5	6	105.233	-6	-2.0	
			14.5	9	105.230	-3	-1.0	
			14.5	12	105.229	-1	-0.3	
			14.5	15	105.228	-1	-0.3	0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 13.00
Base of Test Section 19.60
Test Section Centre 16.30
Initial Water Depth 3.41
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole: BH06 Date: 18/02/10 Site Engineer Calculated By Checked By Rock Type

Number of Rods Test Section Length Test No: 2 Start Time: 12:00

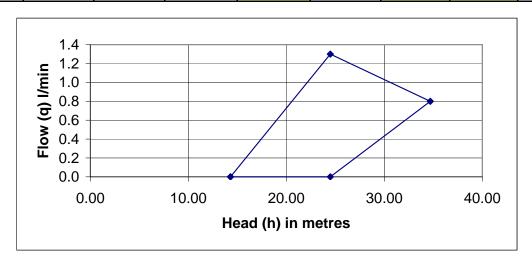
Quartz Mica Schist

4

6.60 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	14.5	10.19	0.70	3.41	0.00	0.00	14.30
2	0.0	29.0	20.39	0.70	3.41	0.00	0.00	24.50
3	0.8	43.5	30.58	0.70	3.41	0.00	0.00	34.69
4	1.3	29.0	20.39	0.70	3.41	0.00	0.00	24.50
5	0.0	14.5	10.19	0.70	3.41	0.00	0.00	14.30



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 7.00 Base of Hole: 10.00 Top of Test Section: 8.00 Base of Test Section: 10.00

Initial Groundwater 1.43

Datum 0.50

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref Pneumatic

Assumed Standing Water Level 3.46 mbdl

m Below Datum

Borehole:BH11 Test No: 1 Date: 01/03/2010 Start Time:1510

Casing Diameter mm 150 Section Diameter mm 131 Section Length 2.00

Rock Type Mica SCHIST 3.46

Water level after test

Datum Level Site Engineer Gauge used: Flow meter used:

1281900 Number of Rods 3 Water Quality

Potable

77361-1-00

Store /	Motor Loval	Water Level	Cours	Florood	Flow	Water Take	Flow	Assessed
Stage / Packer	in Casing	in River	Gauge Pressure	Elapsed Time	(meter)	Litres	FIOW	Assessed
Pressure	in Casing	III KIVEI	Flessule	Time	reading	Lilles		Flow
	mbdl	mabl	nci	mins	m3	litres	litres/min	litres/min
psi 1/200	3.50	IIIabi	psi 7,3	0	105.254	illes	111162/111111	IIII ES/IIIIII
1/200	3.50		1.3	3	105.254	19	6.2	
				6	105.273	18	6.3 6.0	
				9				
					105.317	26	8.7	
				12	105.324	7	2.3	5.0
				15	105.338	14	4.7	5.6
0/000	0.50		445	0	405.045			
2/200	3.50		14.5	0	105.345	0.4	10.0	
				3	105.376	31	10.3	
				6	105.411	35	11.7	
				9	105.446	35	11.7	
				12	105.483	37	12.3	
				15	105.512	29	9.7	11.1
3/200	3.50		21.3	0	106.000			
				3	106.285	285	95.0	
				5	106.498	213	106.5	
				9	106.713	215	53.7	
				12	106.994	281	93.7	
				15	107.184	190	63.3	82
4/200	3.50		14.5	0	107.226			
				3	107.405	179	59.7	
				6	107.656	251	83.7	
				9	107.904	248	82.7	
				12	108.169	265	88.3	
				15	108.351	182	60.7	75
5/200	3.50		7.3	0	108.367			
				3	108.419	52	17.3	
				6	108.701	282	94.0	
				9	108.848	147	49.0	
				12	109.023	175	58.3	
				15	109.156	133	44.3	20.5
				10	100.100	100	77.0	20.0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level: Top of Test Section 8.00 Base of Test Section 10.00 **Test Section Centre** 9.00 **Initial Water Depth** 1.43 Datum to Ground/Bed 0.00 Datum to Gauge 0.70

Borehole:BH11 Date: 01/03/2010 Site Engineer Calculated By Checked By Rock Type Number of Rods

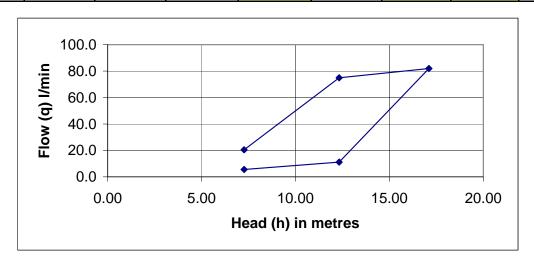
Test No: 1 Start Time:1510



3 **Test Section Length** 2.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	5.6	7.3	5.13	0.70	1.43	0.00	0.00	7.26
2	11.1	14.5	10.19	0.70	1.43	0.00	0.00	12.32
3	82.0	21.3	14.97	0.70	1.43	0.00	0.00	17.10
4	75.0	14.5	10.19	0.70	1.43	0.00	0.00	12.32
5	20.5	7.3	5.13	0.70	1.43	0.00	0.00	7.26



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.00 Base of Hole: 12.00 Top of Test Section: 8.00 Base of Test Section: 12.00

Initial Groundwater 2.38

Datum 0.69

m Below Datum

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref Pneumatic

Assumed Standing Water Level 2.83 mbdl

Borehole:BH12 Test No: 1 Date: 02/03/2010 Start Time:1620

Casing Diameter mm 150 Section Diameter mm 131 Section Length 4.00

Rock Type Mica SCHIST 2.83

Water level after test

Datum Level Site Engineer Gauge used: Flow meter used: Number of Rods

77361-1-00 1281900

3 Water Quality Potable

0, ,	Ivar e i i	Day () (DA		I
Stage /		Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure			_		reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	2.80		7.5	0	108.514			
				3	108.515	1	0.3	
				6	108.515	0	0.0	
				9	108.515	0	0.0	
				12	108.515	0	0.0	
				15	108.515	0	0.0	0
2/200	2.80		14.5	0	108.515			
				3	108.516	1	0.2	
				6	108.517	1	0.3	
				9	108.517	1	0.2	
				12	108.517	0	0.0	
				15	108.518	1	0.2	0
3/200	2.80		21.3	0	108.518			
				3	108.518	0	0.0	
				5	108.518	0	0.0	
				9	108.519	1	0.3	
				12	108.519	0	0.0	
				15	108.520	1	0.3	0.2
4/200	2.80		14.5	0	108.520			
				3	108.520	0	0.0	
				6	108.520	0	0.0	
				9	108.520	0	0.0	
				12	108.520	0	0.0	
				15	108.520	0	0.0	0
						j	0.0	
5/200	2.80		7.3	0	108.520			
0,200	2.00		7.0	3	108.520	0	0.0	
				6	108.520	0	0.0	1
				9	108.520	0	0.0	
				12	108.520	0	0.0	
								0
				12 15	108.520	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level: Top of Test Section 8.00 Base of Test Section 12.00 **Test Section Centre** 10.00 **Initial Water Depth** 2.38 Datum to Ground/Bed 0.00 Datum to Gauge 0.70

Borehole:BH12 Date: 02/03/2010 Site Engineer Calculated By Checked By Rock Type

Start Time:1620

(L)

Mica SCHIST

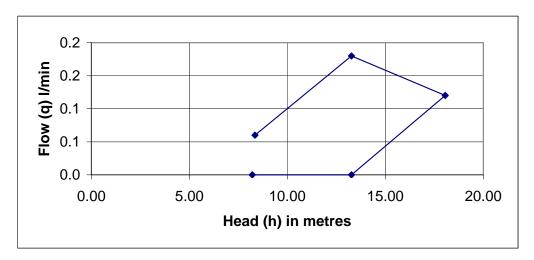
Test No: 1

3

Number of Rods **Test Section Length** 4.00

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.1	7.5	5.27	0.70	2.38	0.00	0.00	8.35
2	0.2	14.5	10.19	0.70	2.38	0.00	0.00	13.27
3	0.1	21.3	14.97	0.70	2.38	0.00	0.00	18.05
4	0.0	14.5	10.19	0.70	2.38	0.00	0.00	13.27
5	0.0	7.3	5.13	0.70	2.38	0.00	0.00	8.21



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.00 Base of Hole: 21.00 Top of Test Section: 16.00 Base of Test Section:

21.00 **Initial Groundwater**

2.03 Datum 0.69

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref Pneumatic

Assumed Standing Water Level mbdl

m Below Datum

Borehole:BH12 Test No: 2 Date: 03/03/2010 Start Time:1240

Casing Diameter mm 150 Section Diameter mm 131 Section Length 5.00

Rock Type Mica SCHIST 2.12

Water level after test

Datum Level Site Engineer Gauge used:

77361-1-00 Flow meter used: 1281900 Number of Rods

Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure	J				reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200			14.5	0	108.537			
				3	108.536	-1	-0.3	
				6	108.536	0	0.0	
				9	108.536	0	0.0	
				12	108.536	0	0.0	
				15	108.536	0	0.0	0
2/200			29.1	0	108.537			
				3	108.537	0	0.0	
				6	108.537	0	0.0	
				9	108.537	0	0.0	
				12	108.537	0	0.0	
				15	108.537	0	0.0	0
3/200			43.6	0	108.537			
				3	108.537	0	0.0	
				5	108.537	0	0.0	
				9	108.537	0	0.0	
				12	108.537	0	0.0	
				15	108.537	0	0.0	0
4/200			29.1	0	108.537			
				3	108.537	0	0.0	
				6	108.537	0	0.0	
				9	108.537	0	0.0	
				12	108.537	0	0.0	
				15	108.537	0	0.0	0
5/200			14.5	0	108.537			
				3	108.537	0	0.0	
				6	108.537	0	0.0	
				9	108.537	0	0.0	
				12	108.537	0	0.0	
				15	108.537	0	0.0	0



(L)

PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 16.00
Base of Test Section 21.00
Test Section Centre 18.50
Initial Water Depth 2.03
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole:BH12
Date: 03/03/2010
Site Engineer
Calculated By
Checked By
Rock Type

Mica SCHIST

Test No: 2

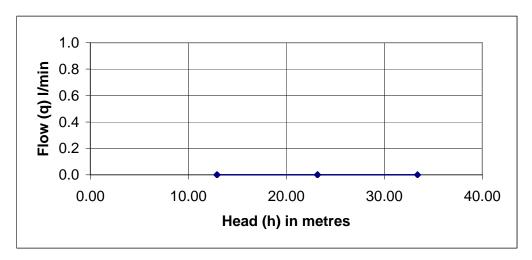
Start Time:1240

Number of Rods

Test Section Length 5.00

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(q)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	14.5	10.19	0.70	2.03	0.00	0.00	12.92
2	0.0	29.1	20.46	0.70	2.03	0.00	0.00	23.19
3	0.0	43.6	30.65	0.70	2.03	0.00	0.00	33.38
4	0.0	29.1	20.46	0.70	2.03	0.00	0.00	23.19
5	0.0	14.5	10.19	0.70	2.03	0.00	0.00	12.92



Note

mbdl - metres below datum/deck level



PACKER TEST FIELD RECORDS

Test Type: Single Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.00 Base of Hole: 35.00 Top of Test Section: 25.00 Base of Test Section: 35.00

Initial Groundwater 1.70

Datum 0.69

Datum to Bed Level: Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Pneumatic Packer Type / Ref

Assumed Standing Water Level 1.74 mbdl

m Below Datum

Borehole:BH12 Test No: 3 Date: 04/03/2010 Start Time:1625

Casing Diameter mm 150 Section Diameter mm 131 Section Length 10.00

Rock Type Mica SCHIST

Water level after test 1.74

Datum Level Site Engineer Gauge used: Flow meter used:

1281900 Number of Rods Water Quality

Potable

77361-1-00

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure	J				reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	1.74		22.0	0	108.589			
				3	108.589	0	0.0	
				6	108.589	0	0.0	
				9	108.589	0	0.0	
				12	108.590	1	0.3	
				15	108.590	0	0.0	0
2/200	1.74		43.5	0	108.590			
				3	108.591	1	0.3	
				6	108.591	0	0.0	
				9	108.591	0	0.0	
				12	108.591	0	0.0	
				15	108.591	0	0.0	0
3/200	1.74		65.5	0	108.591			
				3	108.591	0	0.0	
				5	108.591	0	0.0	
				9	108.591	0	0.0	
				12	108.591	0	0.0	
				15	108.591	0	0.0	0
4/200	1.74		43.5	0	108.591			
				3	108.591	0	0.0	
				6	108.591	0	0.0	
				9	108.591	0	0.0	
				12	108.591	0	0.0	
				15	108.591	0	0.0	0
5/200	1.74		22.0	0	108.591			
				3	108.591	0	0.0	
				6	108.591	0	0.0	
				9	108.591	0	0.0	
				12	108.591	0	0.0	
				15	108.591	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Single Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 25.00
Base of Test Section 35.00
Test Section Centre 30.00
Initial Water Depth 1.70
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole:BH12 Date: 04/03/2010 Site Engineer Calculated By Checked By Rock Type

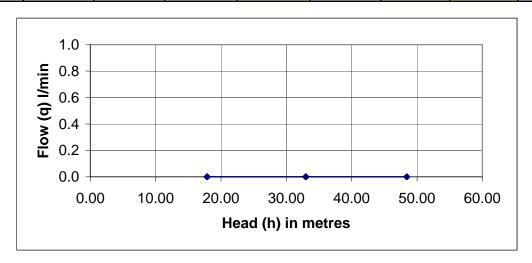
Number of Rods Test Section Length Test No: 3 Start Time:1625



8 10.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	22.0	15.47	0.70	1.70		0.00	17.87
2	0.0	43.5	30.58	0.70	1.70		0.00	32.98
3	0.0	65.5	46.05	0.70	1.70		0.00	48.45
4	0.0	43.5	30.58	0.70	1.70		0.00	32.98
5	0.0	22.0	15.47	0.70	1.70	_	0.00	17.87



Note

mbdl - metres below datum/deck level

PACKER TEST FIELD RECORDS

Test Type: Double Water Injection Packer

Depth Below Bed Level:

Base of Casing: 4.00 Base of Hole: 35.00 Top of Test Section: 11.00 Base of Test Section: 14.00

Initial Groundwater

Datum

Datum to Bed Level:

Gauge Height Above Datum 0.70

Tank Dimensions NA Type of Drill Rods 25mm Packer Type / Ref Pneumatic

Assumed Standing Water Level 1.79 mbdl

1.72

m Below Datum

Borehole:BH12 Test No: 4 Date: 04/03/2010 Start Time:16:00

Casing Diameter mm 150 Section Diameter mm 131 Section Length 3.00

Rock Type Mica SCHIST 1.79

Water level after test

Datum Level Site Engineer Gauge used:

77361-1-00 Flow meter used: 1281900

Number of Rods Water Quality Potable

Stage /	Water Level	Water Level	Gauge	Elapsed	Flow	Water Take	Flow	Assessed
Packer	in Casing	in River	Pressure	Time	(meter)	Litres		Average
Pressure	J				reading			Flow
psi	mbdl	mabl	psi	mins	m3	litres	litres/min	litres/min
1/200	1.60		14.5	0	108.667			
				3	108.667	0	0.0	
				6	108.667	0	0.0	
				9	108.667	0	0.0	
				12	108.667	0	0.0	
				15	108.667	0	0.0	0
2/200	1.60		29.1	0	108.667			
				3	108.667	0	0.0	
				6	108.667	0	0.0	
				9	108.667	0	0.0	
				12	108.667	0	0.0	
				15	108.667	0	0.0	0
3/200	1.60		43.5	0	108.667			
				3	108.667	0	0.0	
				5	108.667	0	0.0	
				9	108.667	0	0.0	
				12	108.667	0	0.0	
				15	108.667	0	0.0	0
4/200	1.60		29.1	0	108.667			
				3	108.667	0	0.0	
				6	108.667	0	0.0	
				9	108.667	0	0.0	
				12	108.667	0	0.0	
				15	108.667	0	0.0	0
5/200	1.60		14.5	0	108.667			
				3	108.667	0	0.0	
				6	108.667	0	0.0	
				9	108.667	0	0.0	
				12	108.667	0	0.0	
				15	108.667	0	0.0	0



PACKER TEST CALCULATIONS

Test Type: Double Water Injection Packer

Depth Below Ground/Bed Level:
Top of Test Section 11.00
Base of Test Section 14.00
Test Section Centre 12.50
Initial Water Depth 1.72
Datum to Ground/Bed 0.00
Datum to Gauge 0.70

Borehole:BH12 Date: 04/03/2010 Site Engineer Calculated By Checked By Rock Type

Number of Rods Test Section Length Test No: 4 Start Time:16:00

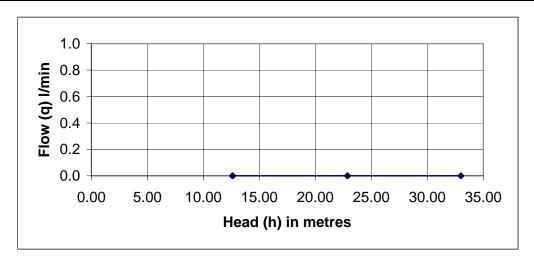
Mica SCHIST

5

3.00 (L)

See field data sheet for test and section data Initial water level in casing used as standing water level Total head calculated as (1 + 2 + 3) - (4 + 5)

Stage	Assessed	Gauge	Equivalent	Gauge	Datum to	Head loss	Head loss -	Total Head
	Flow I/min	Pressure	Head of	Height to	Initial Water	in basic	other (5)	(h)
	(p)	psi	Water on	Datum (2)	Level (3)	pipework		
			Gauge (1)			and rods		
						(4)		
	l/min	psi	m	m	m	m	m	m
1	0.0	14.5	10.19	0.70	1.72	0.00	0.00	12.61
2	0.0	29.1	20.46	0.70	1.72	0.00	0.00	22.88
3	0.0	43.5	30.58	0.70	1.72	0.00	0.00	33.00
4	0.0	29.1	20.46	0.70	1.72	0.00	0.00	22.88
5	0.0	14.5	10.19	0.70	1.72	0.00	0.00	12.61



Note

mbdl - metres below datum/deck level

Scottish & Southern Energy Sloy Pumping Station Mackintosh Probe Data

Position	Depth (mbgl)	Blow Count
MP01	1.20-1.27	100
MP02	0.70-0.80	23
	0.80-0.90	60
	0.90-0.90	100
MP03	0.90-1.00	85
	1.00-1.05	100
MP04	1.20-1.30	32
	1.30-1.40	80
	1.40-1.40	100
MP05	1.20-1.30	9
	1.30-1.40	11
	1.40-1.48	100
MP06	0.95-1.05	30
00	1.05-1.15	100
MP07	1.05-1.15	50
IVII O7	1.15-1.21	100
MP08	1.10-1.20	14
IVIFUO		64
	1.20-1.30	
MDOO	1.30-1.36	100
MP09 MP10	0.90-0.98	100
MP10	1.05-1.15	70
	1.15-1.20	100
MP11	0.85-0.95	37
	0.95-1.05	18
	1.05-1.15	16
	1.15-1.25	36
	1.25-1.27	100
MP12	0.90-1.00	19
	1.00-1.10	24
	1.10-1.20	11
	1.20-1.30	50
	1.30-1.31	100
MP13	0.75-0.75	100
MP14	1.20-1.30	2
	1.30-1.40	3
	1.40-1.50	2
	1.50-1.60	4
	1.60-1.70	7
	1.70-1.80	19
	1.80-1.90	20
	1.90-1.95	100
MP15	1.10-1.15	100
MP16	1.20-1.30	12
	1.30-1.40	21
	1.40-1.50	12
	1.50-1.60	43
	1.60-1.70	48
	1.70-1.80	24
	1.80-1.90	33
	1.90-1.93	100
MP17	1.10-1.14	100
	10 1.14	.00

- ···	Is	D. 0 .
Position	Depth (mbgl)	
MP18	1.90-2.00	3
	2.00-2.10	3
	2.10-2.20	3
	2.20-2.30	2
	2.30-2.40	2
	2.40-2.50	2
	2.50-2.60	7
	2.60-2.70	12
	2.70-2.80	7
	2.80-2.90	7
	2.90-3.00	5
	3.00-3.08	100
MP19	1.00-1.10	9
	1.10-1.20	7
	1.20-1.30	1
	1.30-1.40	9
	1.40-1.50	43
	1.50-1.60	52
	1.60-1.64	100
MP20	0.75-0.85	88
	0.85-0.91	100
MP21	1.10-1.20	3
	1.20-1.30	3
	1.30-1.40	6
	1.40-1.50	7
	1.50-1.60	10
	1.60-1.70	6
	1.70-1.80	3
	1.80-1.90	3
	1.90-1.94	100
MP22	1.20-1.30	15
	1.30-1.40	42
	1.40-1.50	10
	1.50-1.58	100
MP23	1.20-1.30	16
	1.30-1.40	27
	1.40-1.50	39
	1.50-1.60	31
	1.60-1.70	24
	1.70-1.78	100
MP24	1.20-1.30	9
	1.30-1.40	13
	1.40-1.50	14
	1.50-1.60	39
	1.60-1.70	28
	1.70-1.75	100

Position	Depth (mbgl)	Blow Count
MP25	1.20-1.30	20
	1.30-1.40	21
	1.40-1.50	20
	1.50-1.60	9
	1.60-1.70	19
	1.70-1.80	9
	1.80-1.90	6
	1.90-2.00	7
	2.00-2.10	6
	2.10-2.20	9
	2.20-2.20	100
MP26	1.20-1.30	10
	1.30-1.40	61
	1.40-1.43	100

CON103001 Figure MP1



			Installat	ion Details					
Remarks			Type 19mm Standpipe piezometer Response Zone 6.00-10.00m Installation Date 15/03/2010		Borehole No Tip/Pipe Depth Datum Datum Elevation	BH2 10.00m Ground Level 15.46m OD			
	Reading Details								
Date	Time	Operator	Depth to Water (m below Datum)	Water Level m OD	Remarks	and Samples Taken			
15/03/2010 17/03/2010 30/03/2010 16/04/2010 11/05/2010	010 12:00		Reading Details Depth to Water Water Level		Remarks and Samples Taken				
					Approved by				



Installation Details									
Remarks			Type 50	mm Standpipe	Borehole No	ВН3			
					Tip/Pipe Depth	4.76m			
			Response Zone 2.0		Datum	Ground Level			
			Installation Date 15	/03/2010	Datum Elevation	15.62m OD			
	Reading Details								
Date	Time	Operator	Depth to Water (m below Datum)	Water Level m OD	Remarks and Samples Taken				
17/03/2010	12:00		3.57	12.05					
30/03/2010	12:00		3.42	12.20					
16/04/2010	12:00		3.13	12.49					
11/05/2010	13:00		3.68	11.94					
					Approved by				



			Installat	ion Details			
Remarks			Type 50mm Standpipe Response Zone 2.00-4.00m Installation Date 12/03/2010		Borehole No Tip/Pipe Depth Datum Datum Elevation	BH4 4.00m 16.73m OD	
			Readir	ng Details			
Date	Time	Operator	Depth to Water (m below Datum) Water Level m OD		Remarks and Samples Taken		
17/03/2010 30/03/2010 16/04/2010 11/05/2010	10 12:00		2.86 2.80 2.70 3.26	13.87 13.93 14.03 13.47	Remarks and Samples Taken		
					Approved by		



Installation Details										
Remarks			Type 19mm Standpipe piezometer Response Zone 10.00-20.00m Installation Date 17/03/2010		Borehole No Tip/Pipe Depth Datum Datum Elevation	BH6 19.50m Ground Level 15.91m OD				
	Reading Details									
Date	Time	Operator	Depth to Water (m below Datum)	Water Level m OD	Remarks	and Samples Taken				
17/03/2010 30/03/2010 16/04/2010 11/05/2010	110 12:00		0.90 0.49 0.61 0.69	15.01 15.42 15.30 15.22	Tip/Pipe Depth 19.50m Datum Ground Level Datum Elevation 15.91m OD Remarks and Samples Taken					
					Approved by					

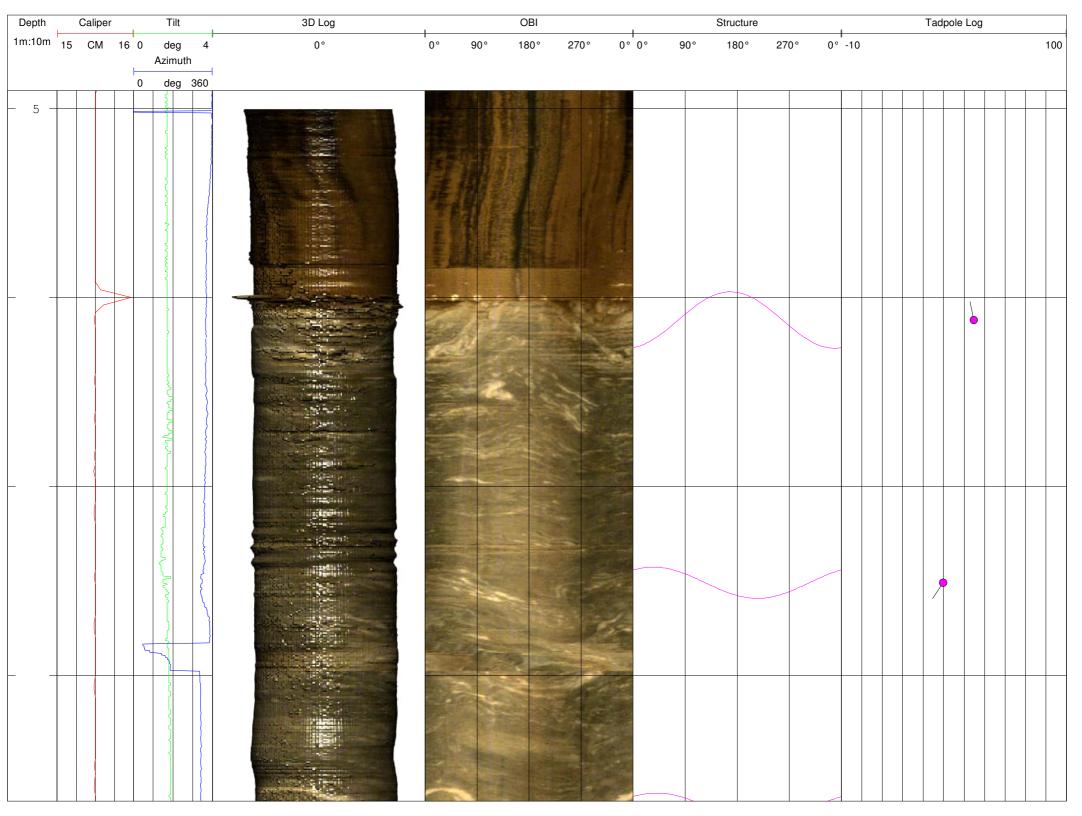


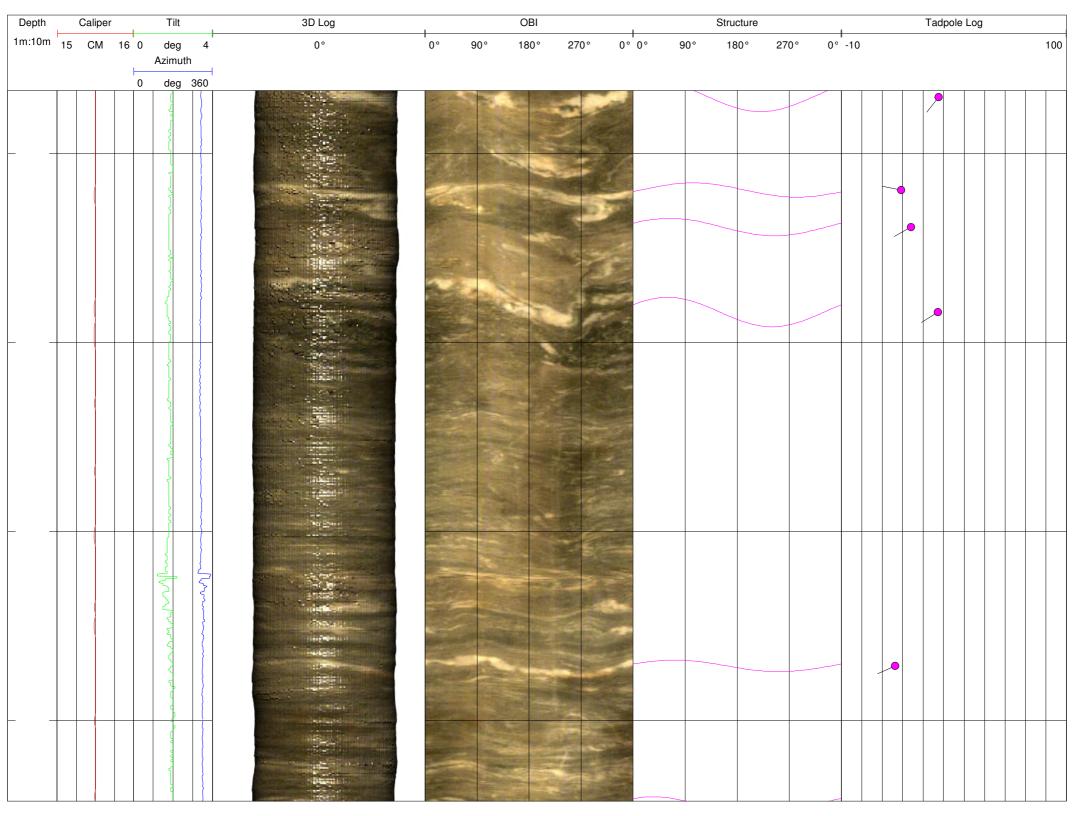
			Installat	ion Details						
Remarks			Type 19mm Standpipe piezometer Response Zone 23.00-30.00m Installation Date 17/03/2010		Borehole No Tip/Pipe Depth Datum Datum Elevation	BH12 29.50m Ground Level 15.96m OD				
	Reading Details									
Date	Time	Operator	Depth to Water (m below Datum)	Water Level m OD	Remarks	and Samples Taken				
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					Approved by	l				

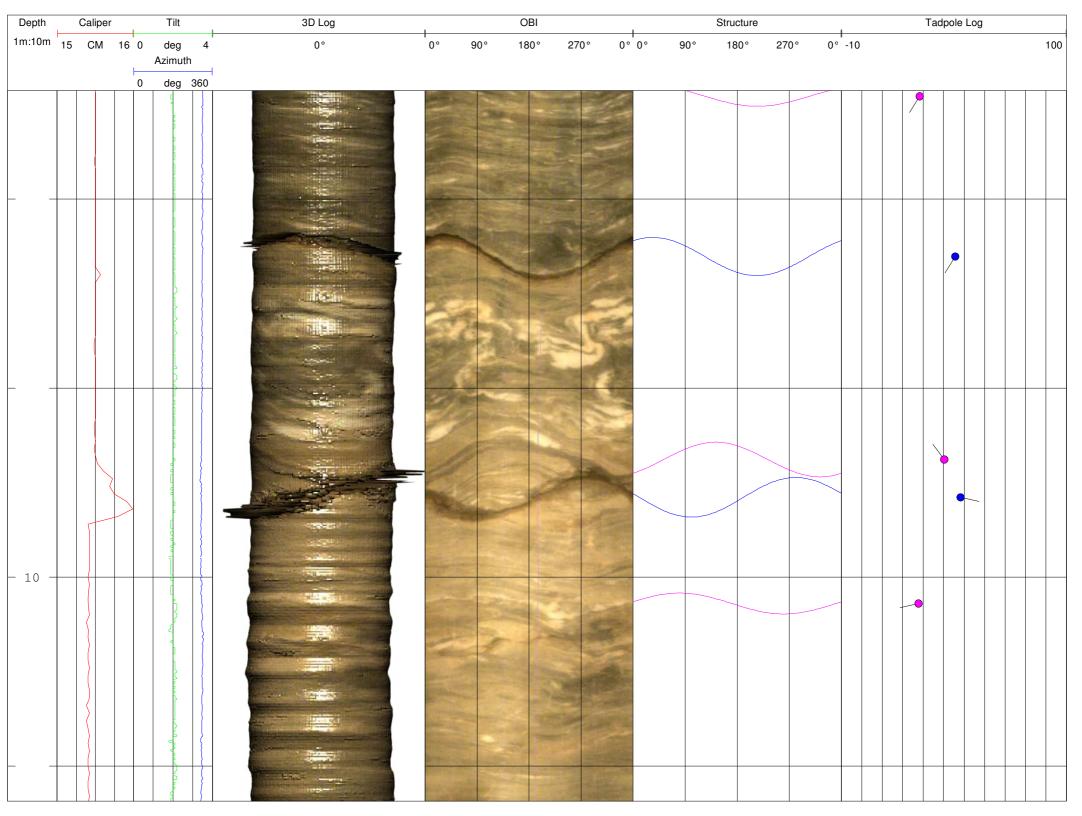


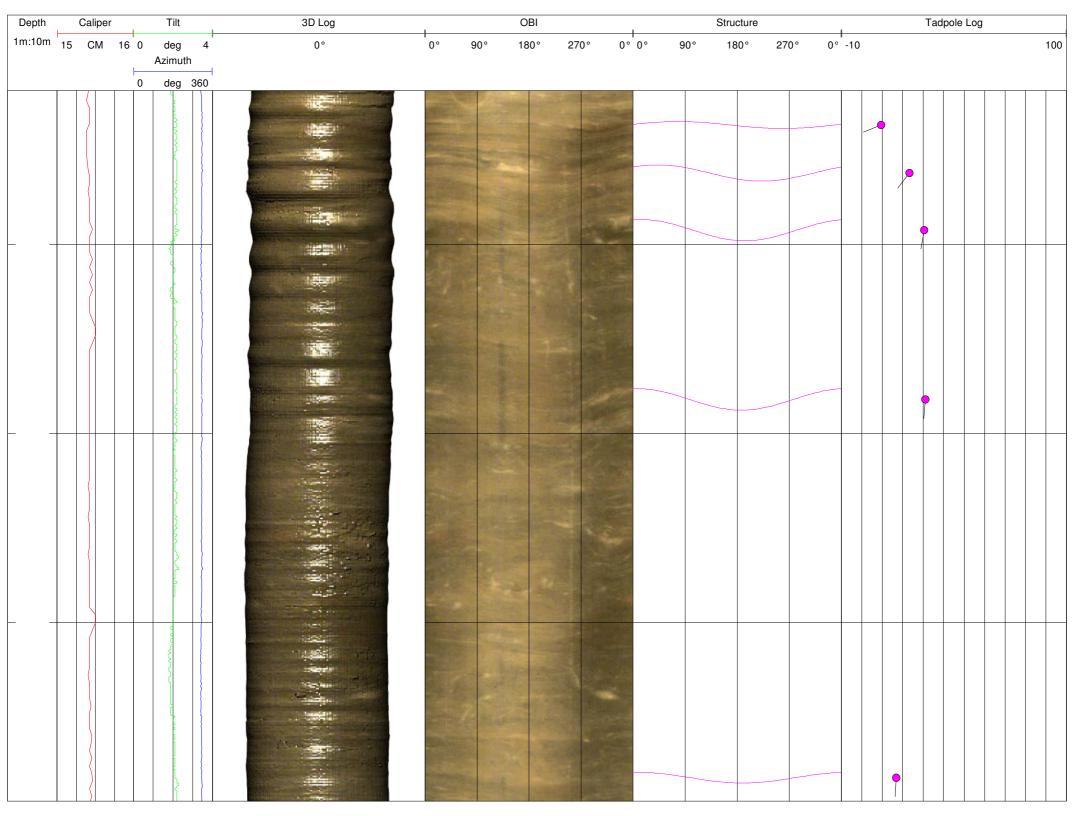
APPENDIX C Downhole Logging Results

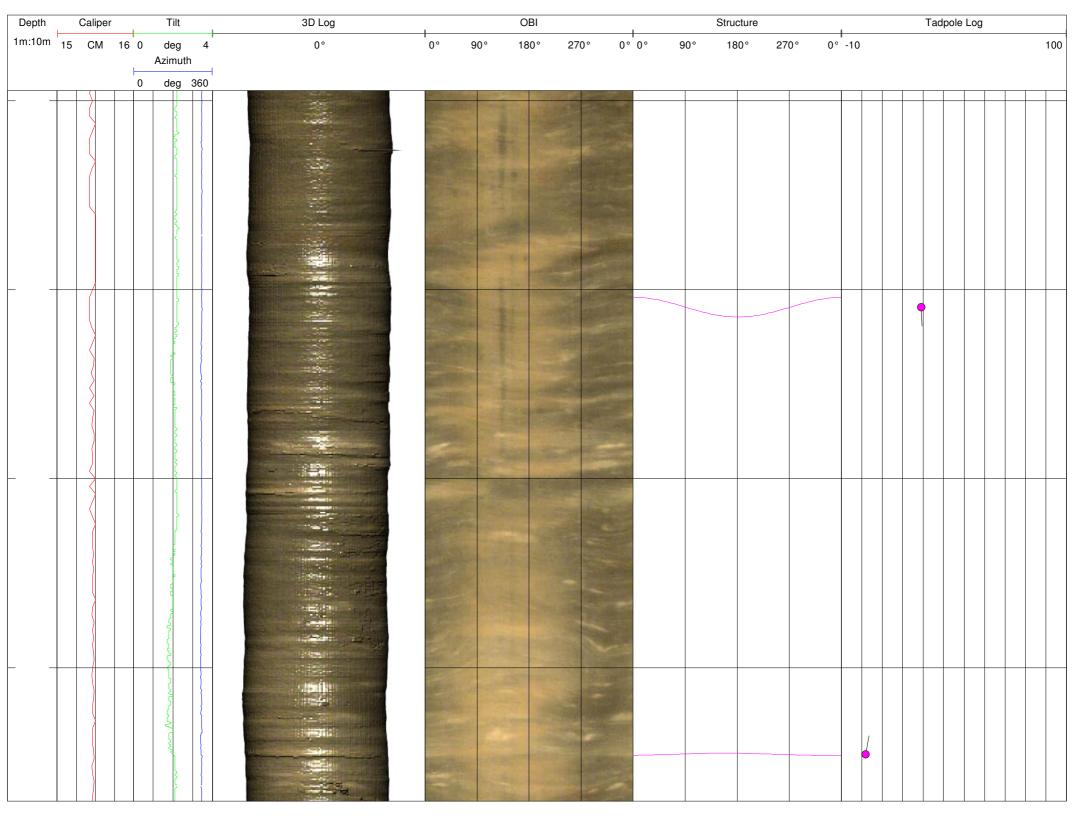
-Fu	Fugro En	gineering Services							
	Client:	Scottish and Southe	rn Energy PLC	Log Type:					
	Borehole:	BH1		Optical Televiewer Log					
Project: COI	N103001 Sloy Power Station			Approved:					
Location: Sloy	·	Reference:	Elevation:						
Drilled Depth:	20m	Date:	04/03/2010						
Logged Depth:	19.87m	Recorded By	:						
Logging Datum:	Ground Level			Remarks:					
Logged Interval:				North reference is magnetic, Tadpol	le log and tabulated data is correct	ted for borehole deviation			
Fluid Level:									
Structure Key: -	Foliation Fracture	Vein							
BOREHOLE	ERECORD			CASING RECORD					
Bit Diameter:	From:	То:		Туре	Size From		То		
150mm	0m	4.2m		Steel	150mm 0m		4.2m		
120mm	4.2m	20.0m							
	Caliper Tilt	3D Log		OBI	Structure	1	Tadpole Log		
1m:10m 15	CM 16 0 deg 4	0°	0° 90°	180° 270° 0° 0°	90° 180°	270° 0° -10	100		
	Azimuth								
	0 deg 360		1 10 10 90						

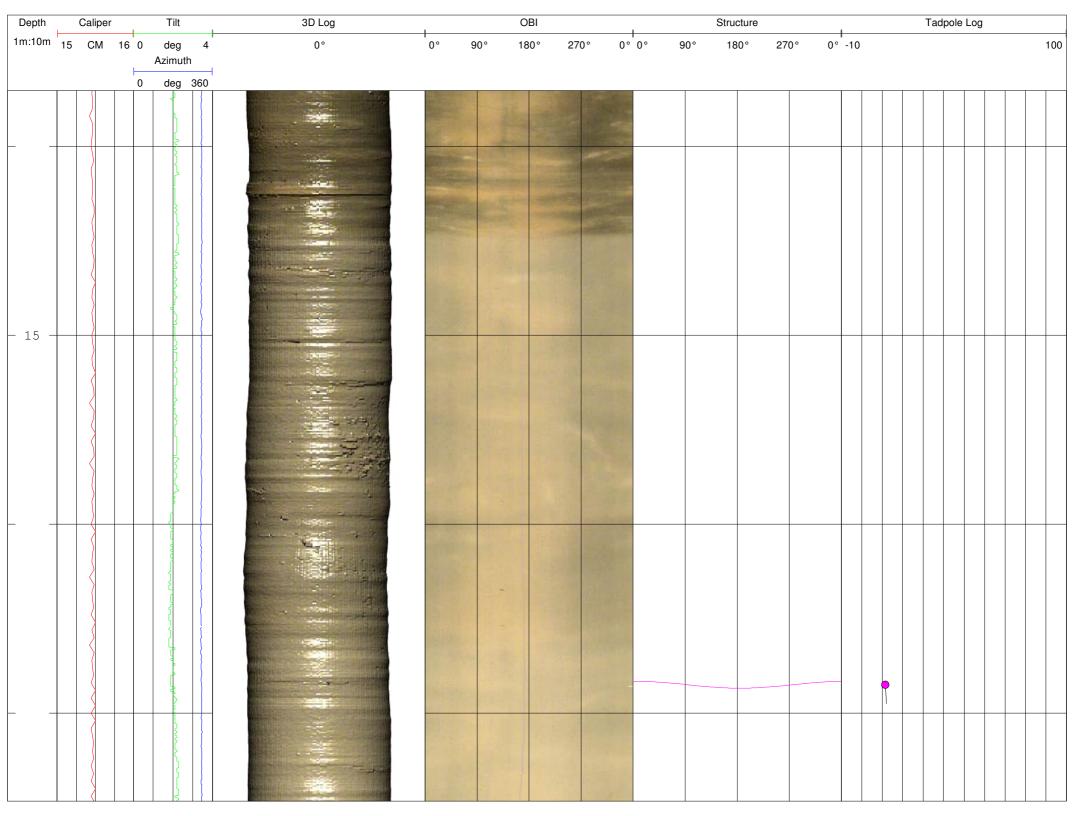


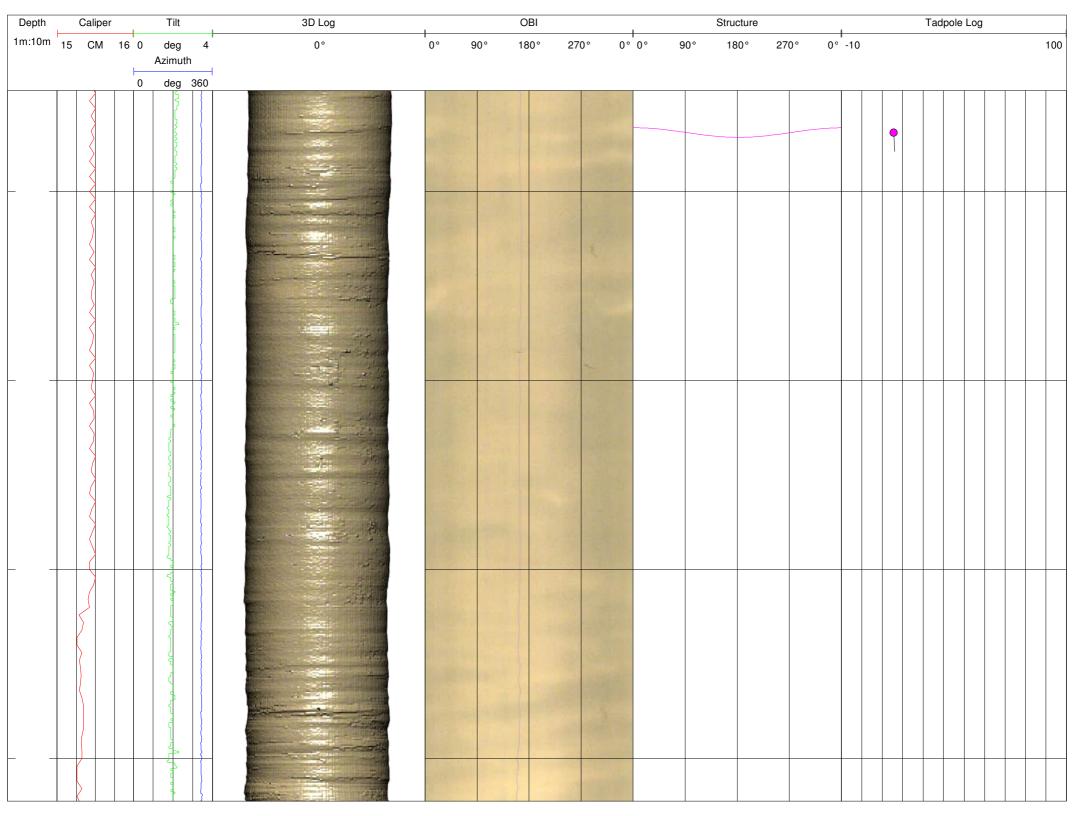


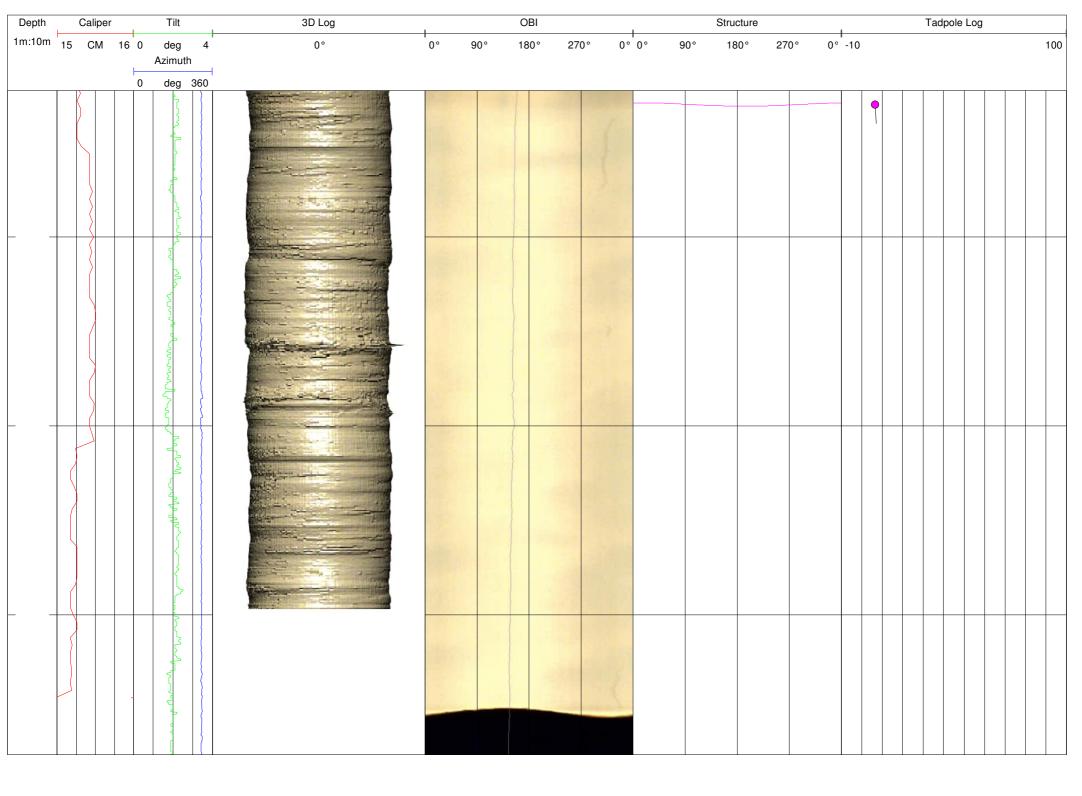




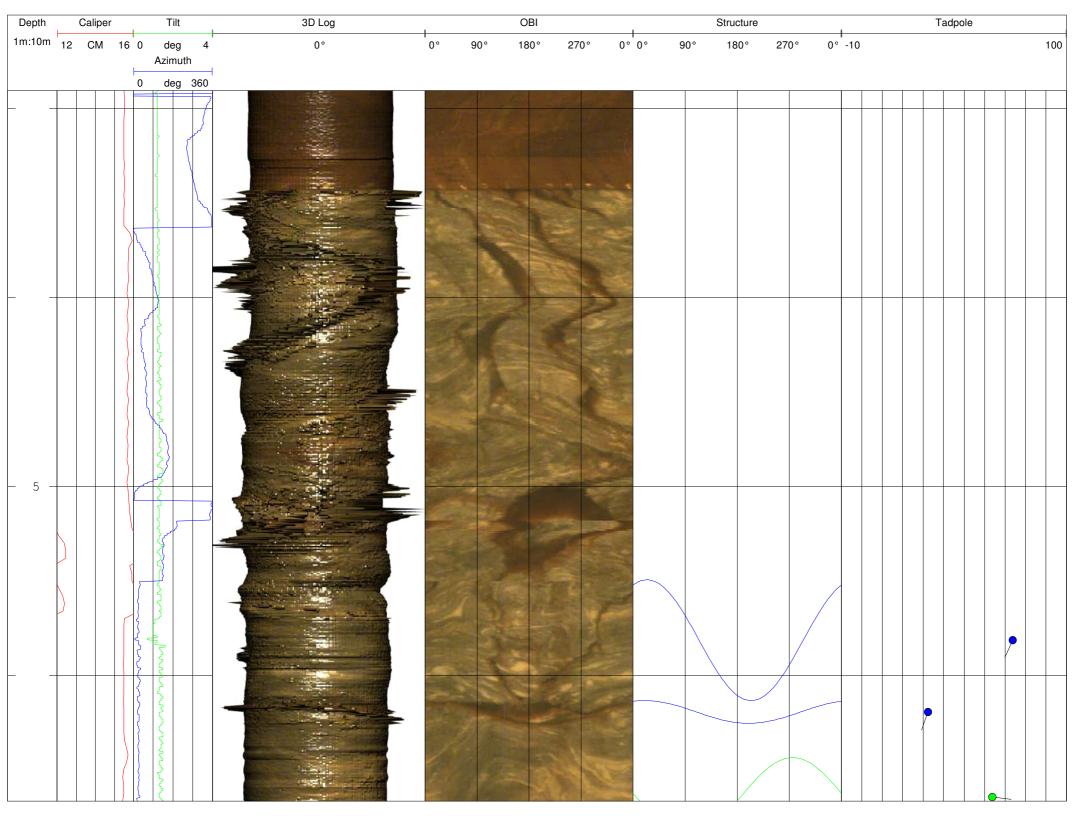


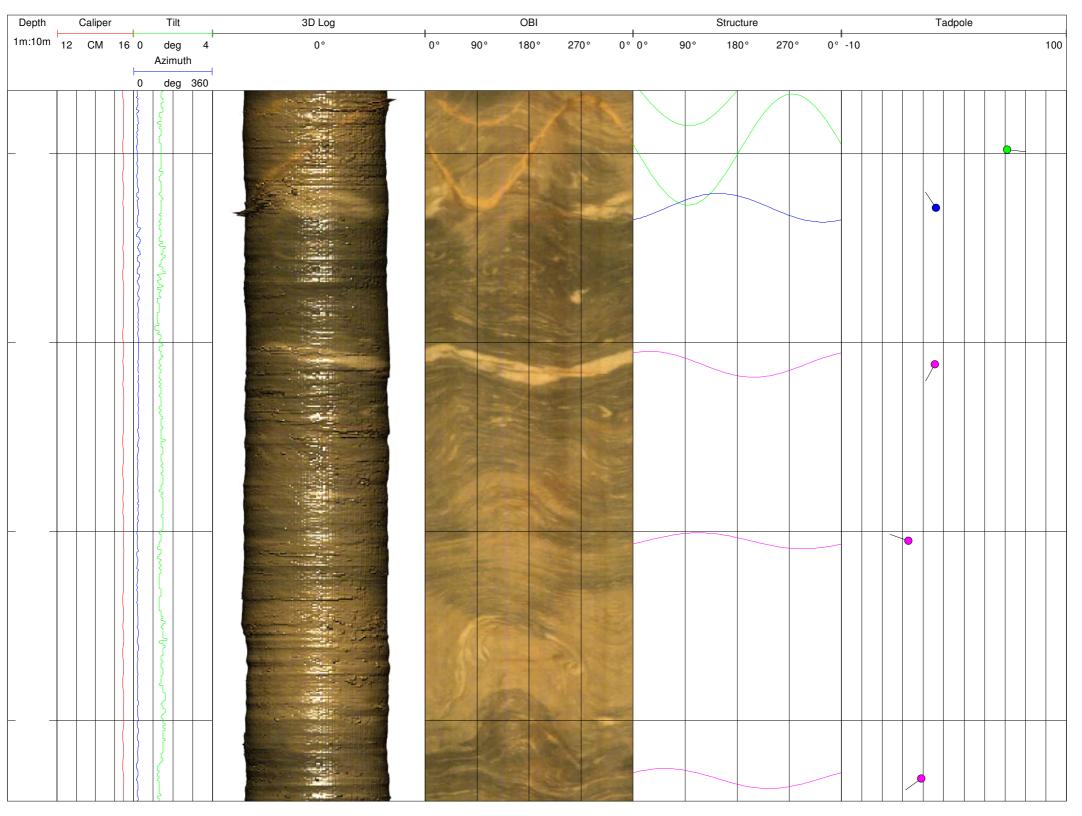


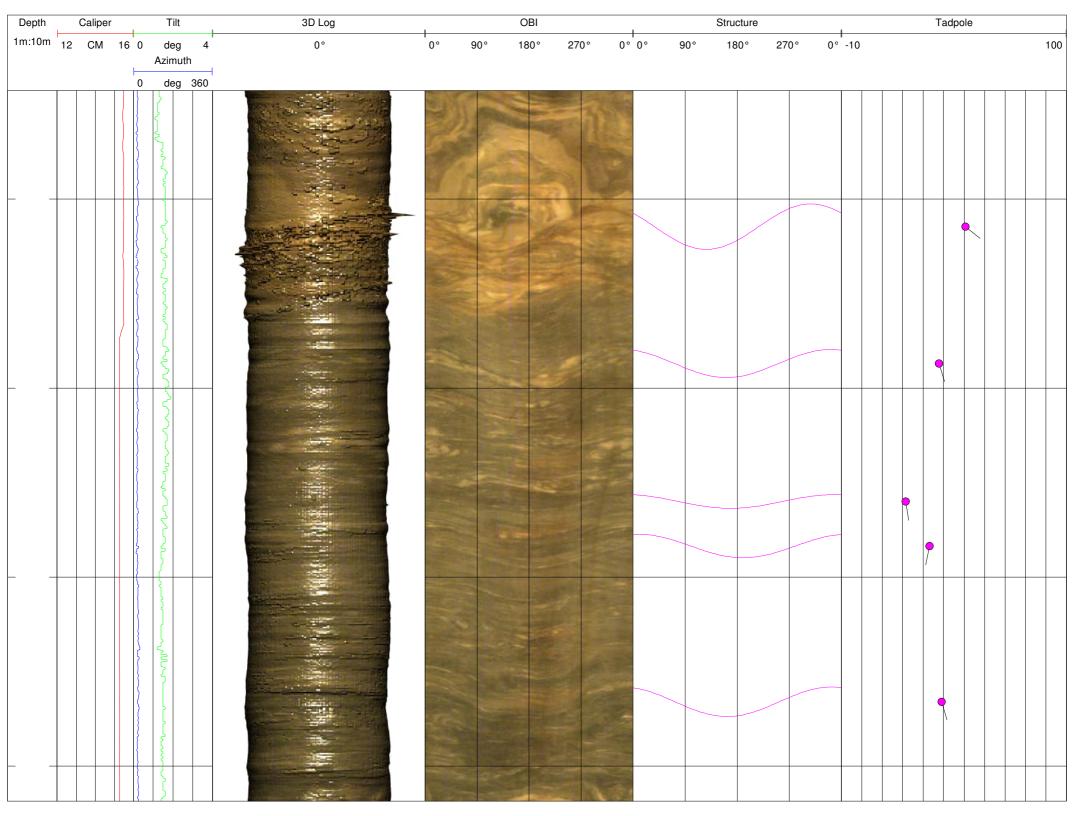


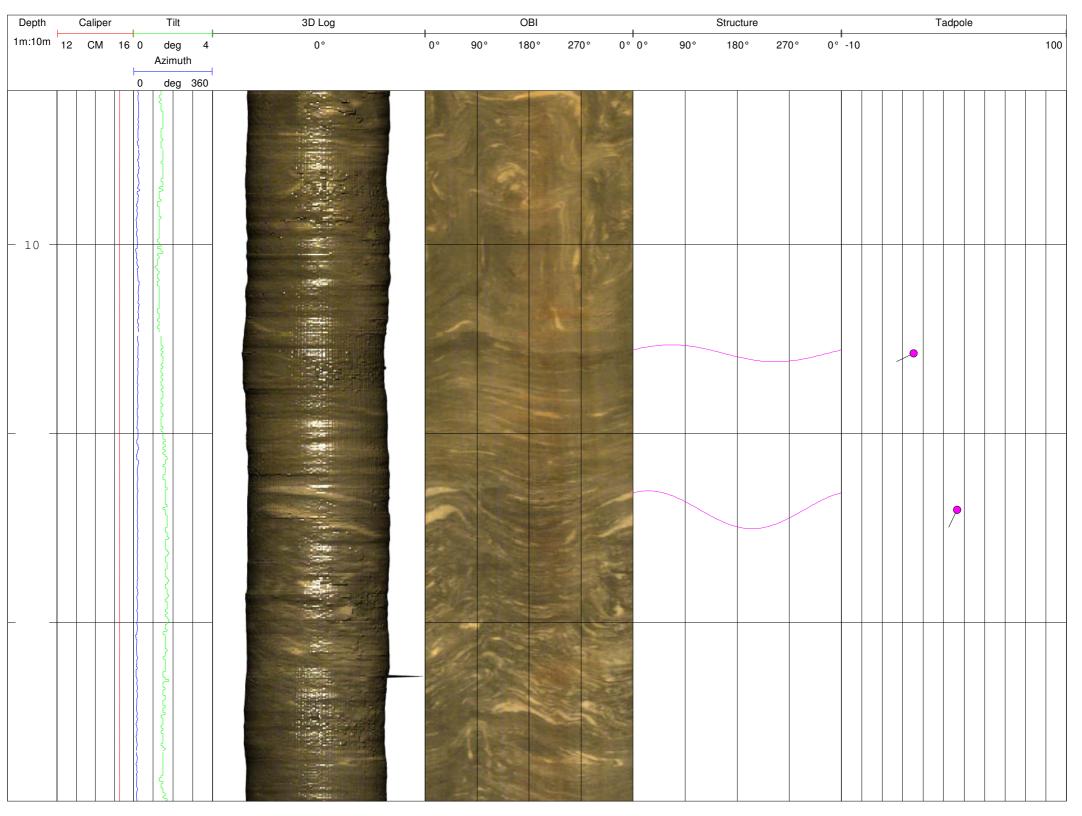


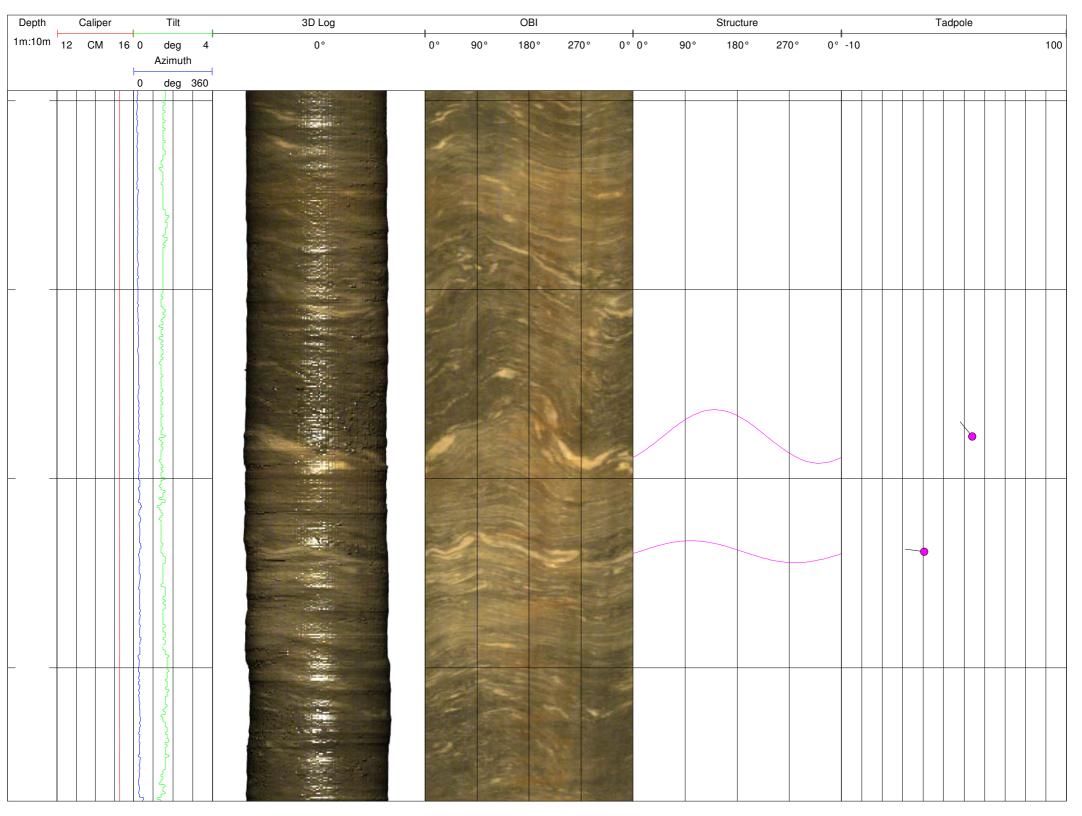
-Fu	Fugro Engi	neering Services												
	Client: Scottish and Southern Energy PLC			Log Type: Optical Televiewer Log										
	Borehole:	Borehole: BH2												
Proiect: CO	N103001 Sloy Power Station			Approved:										
Location: Sloy	Grid Ref	i:												
Drilled Depth:	35.0m	Date:	04/03/2010											
Logged Depth:	34.85m	Recorded By:												
Logging Datum:				Remarks:										
Logged Interval:				North reference is magnetic, T	Tadpol	le log and tabulate	d data is correct	ed for borehole de	viation					
Fluid Level:														
Structure Key:	Foliation Fracture	Vein												
BOREHOL	E RECORD			CASING RECOR	D									
Bit Diameter:	From:	То:		Туре		Size		From			То			
150mm	0m	.2m		Steel		150mm		0m			4.2m			
120mm	4.2m 35.0m													
Depth	Caliper Tilt	3D Log	Л	OBI Structure			1			Tadpole				
1m:10m 12	CM 16 0 deg 4	0°	"0° 90°	180° 270° 0°	°'o°	90°	180°	270° 0°	-10					100
	Azimuth 0 deg 360													

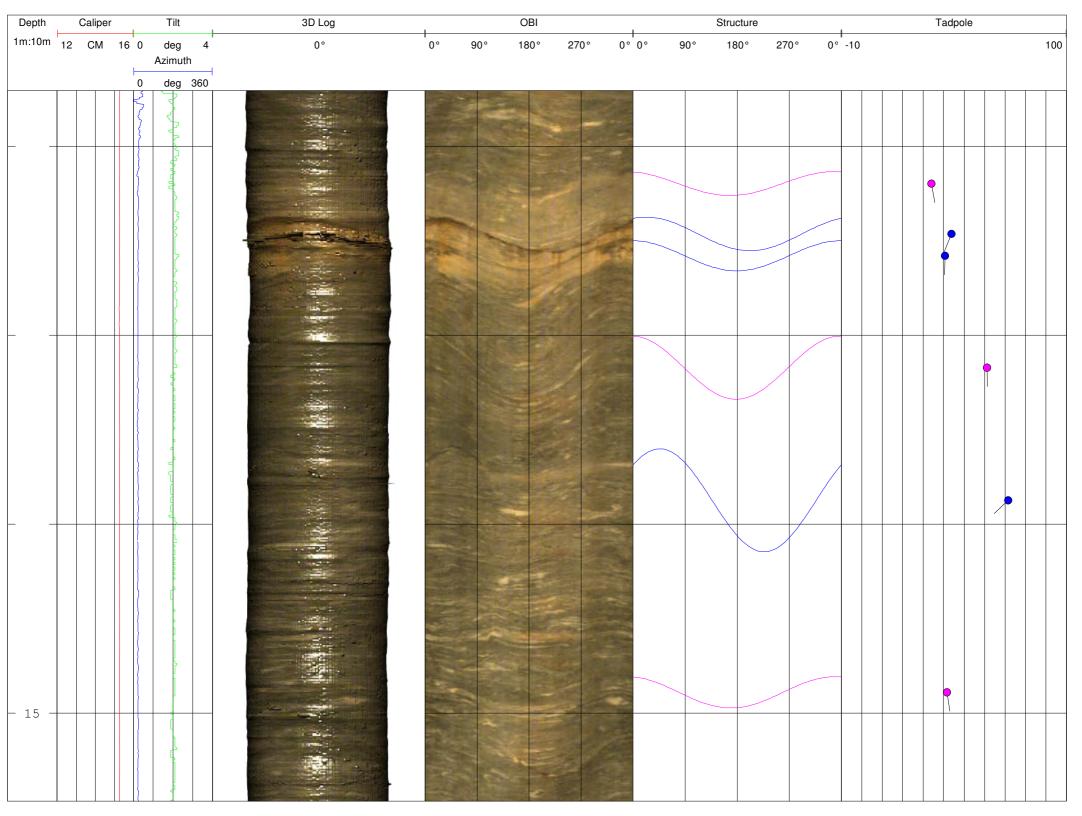


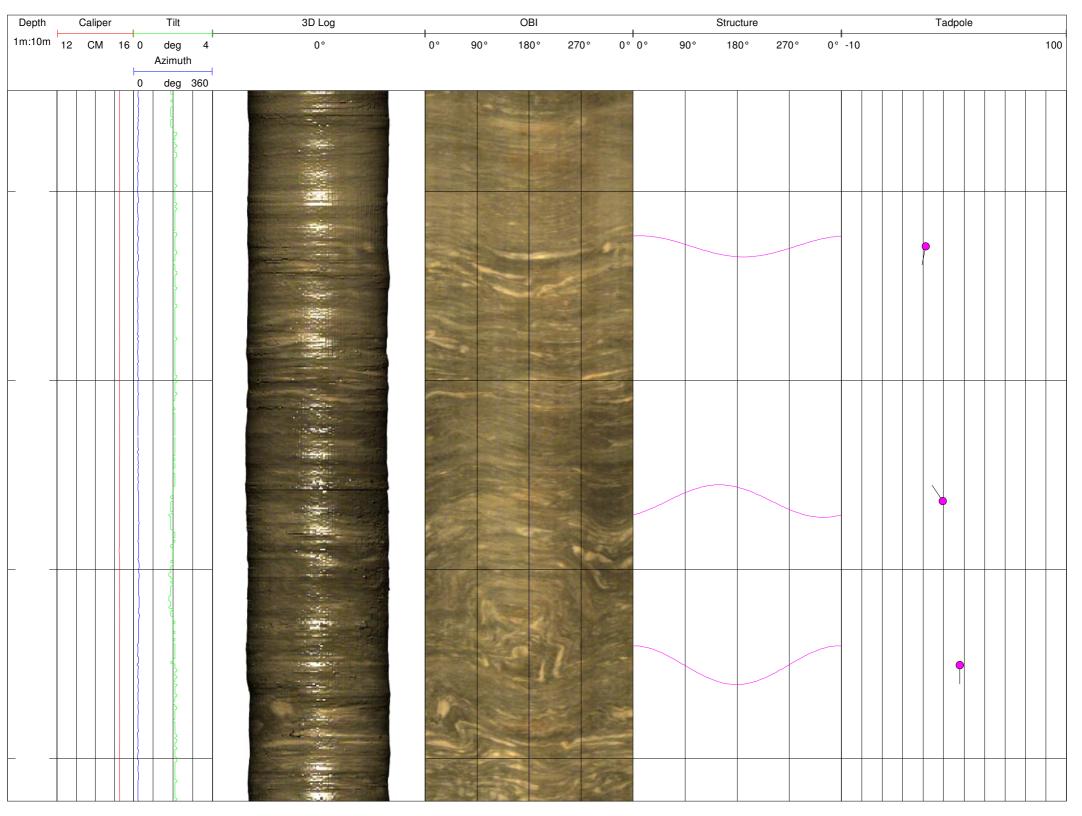


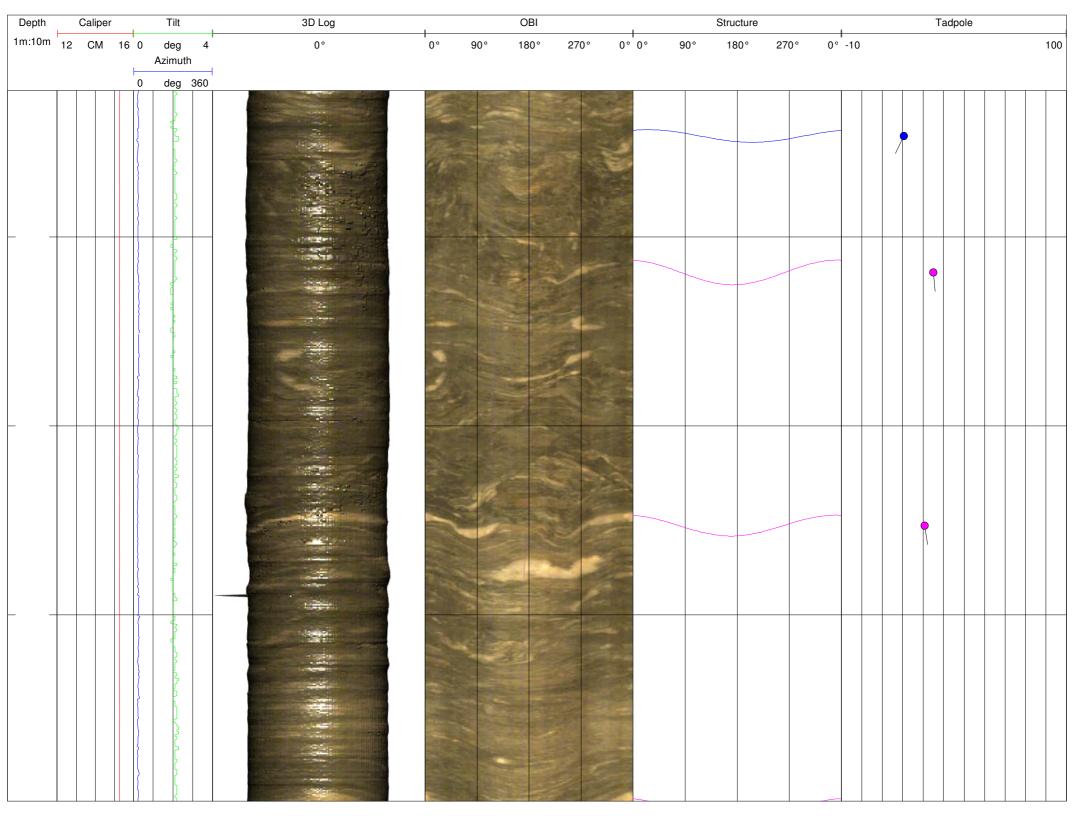


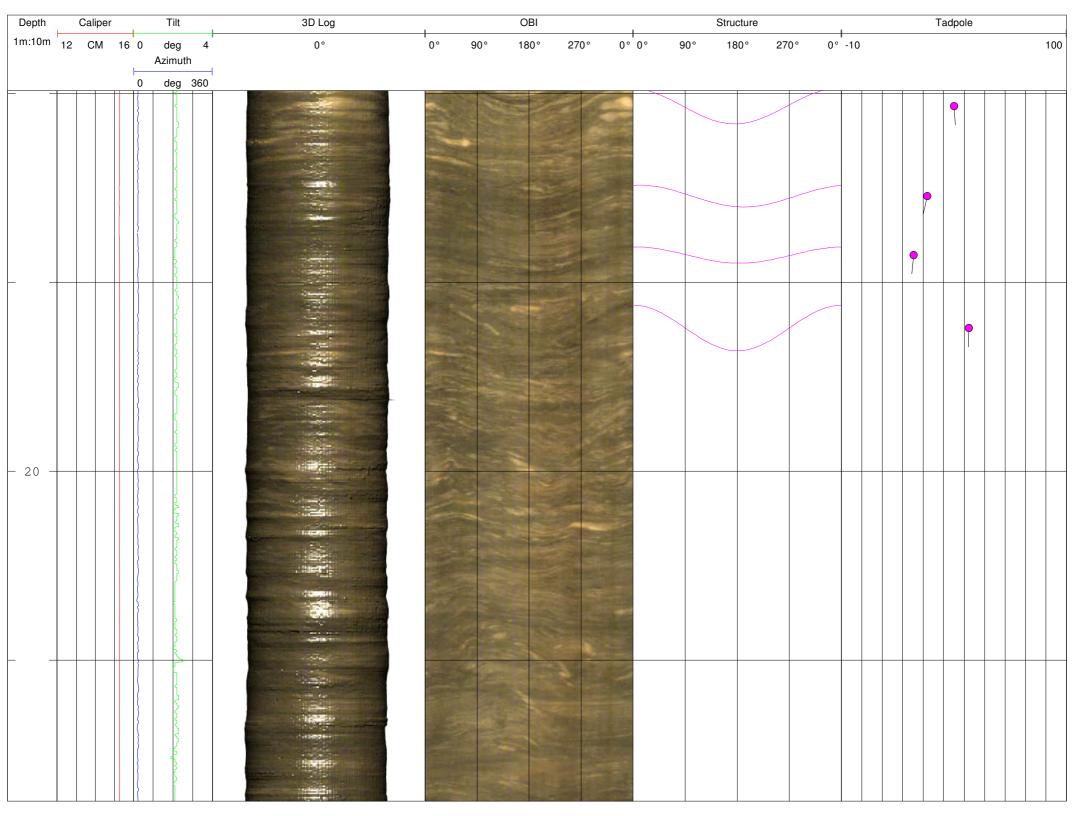


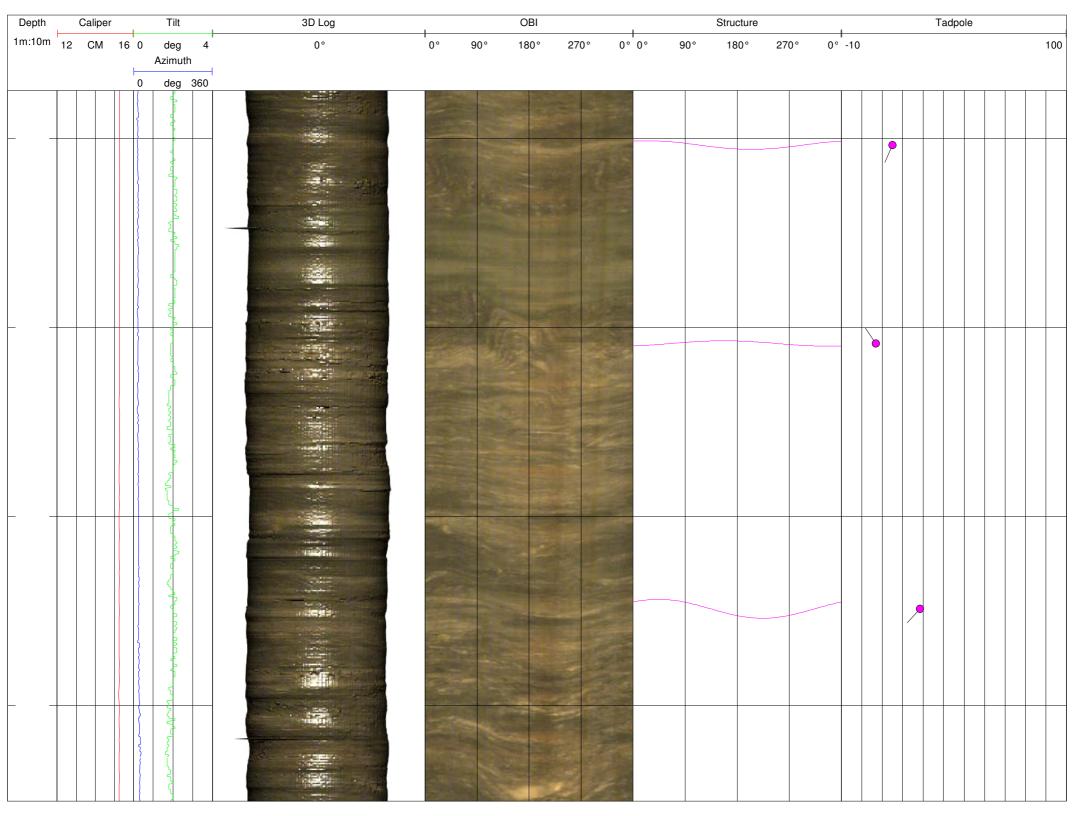


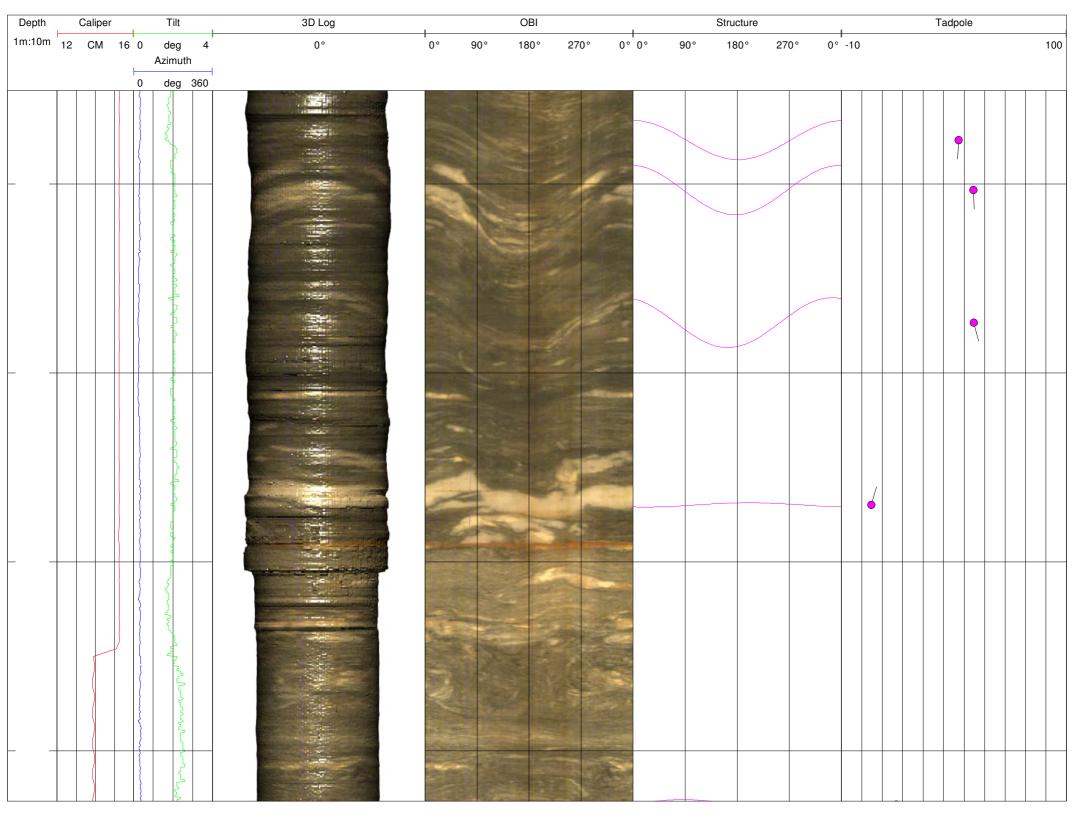


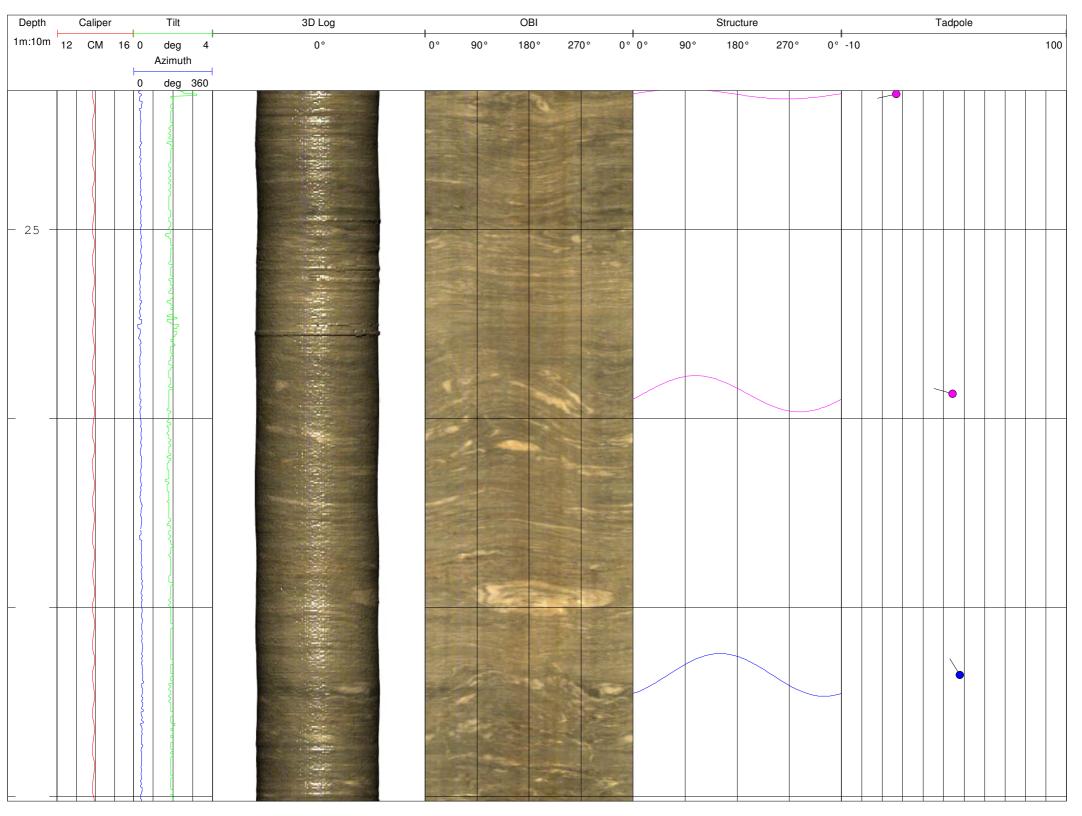


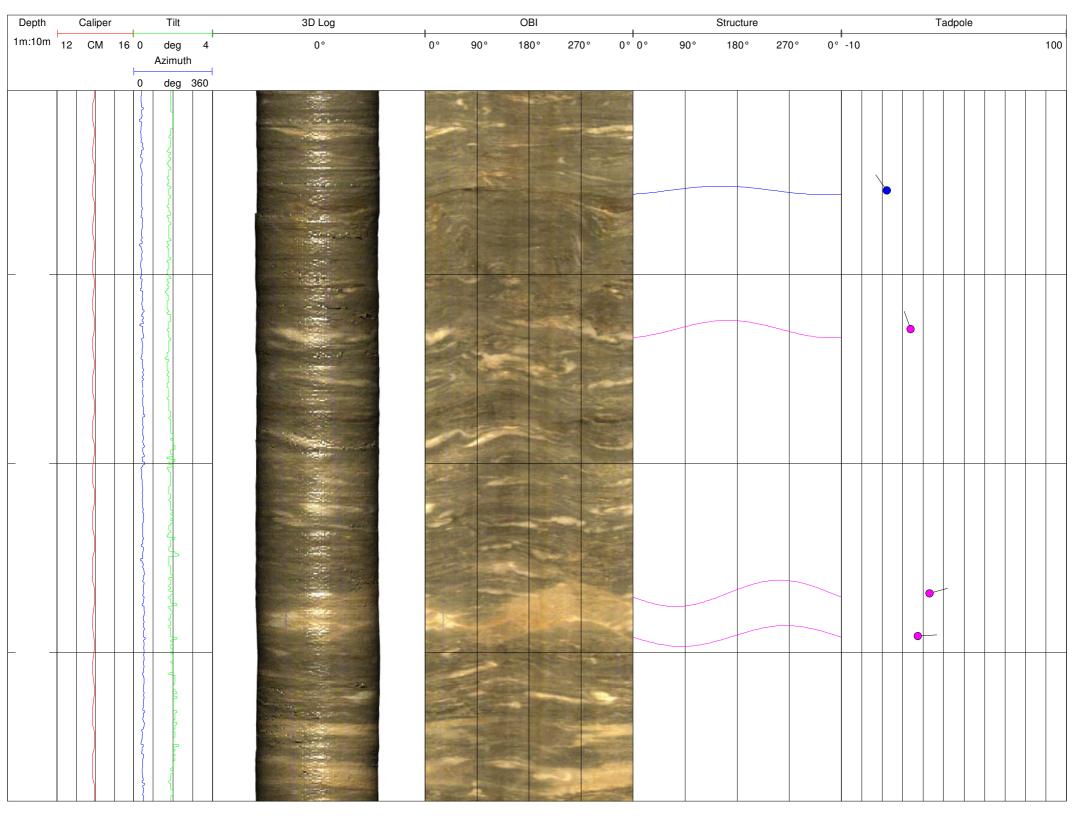


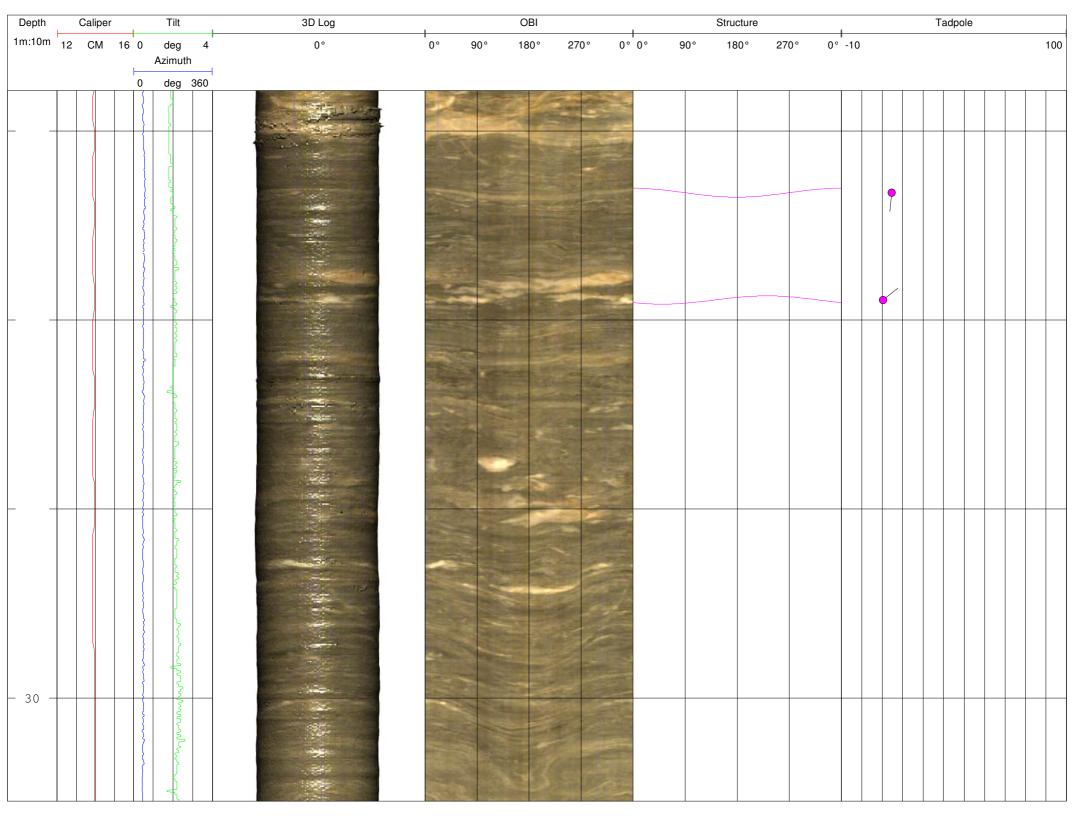


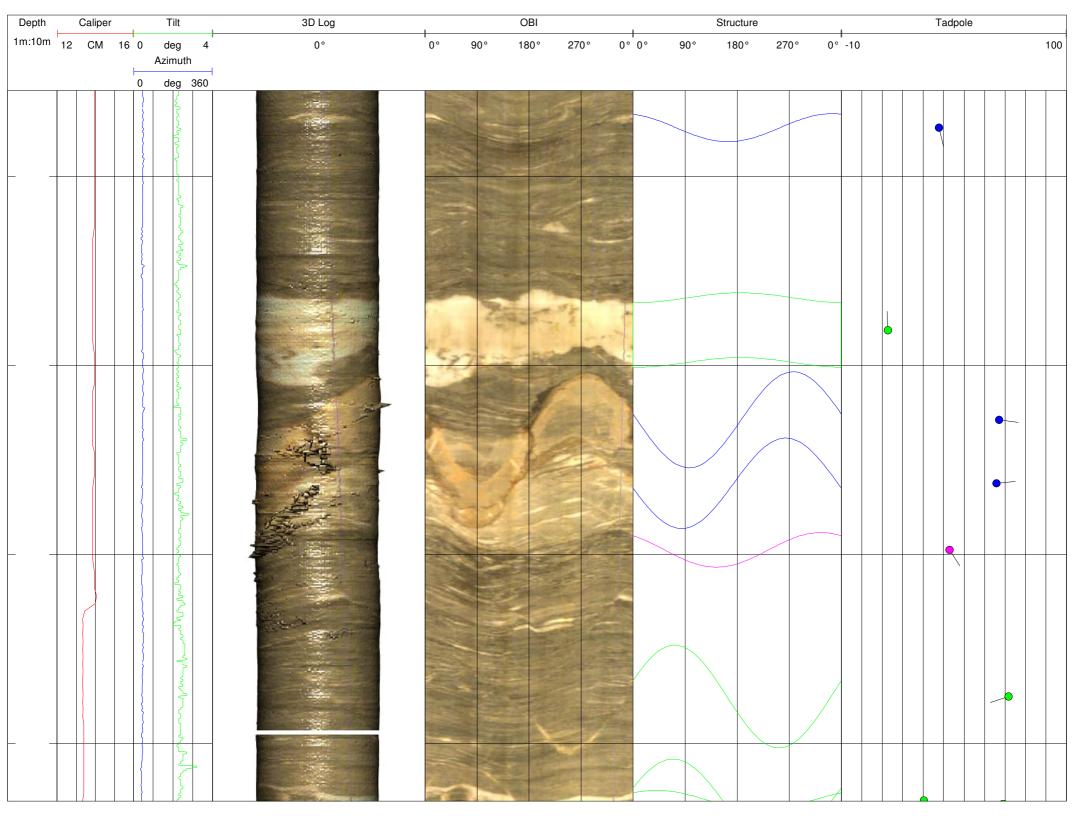


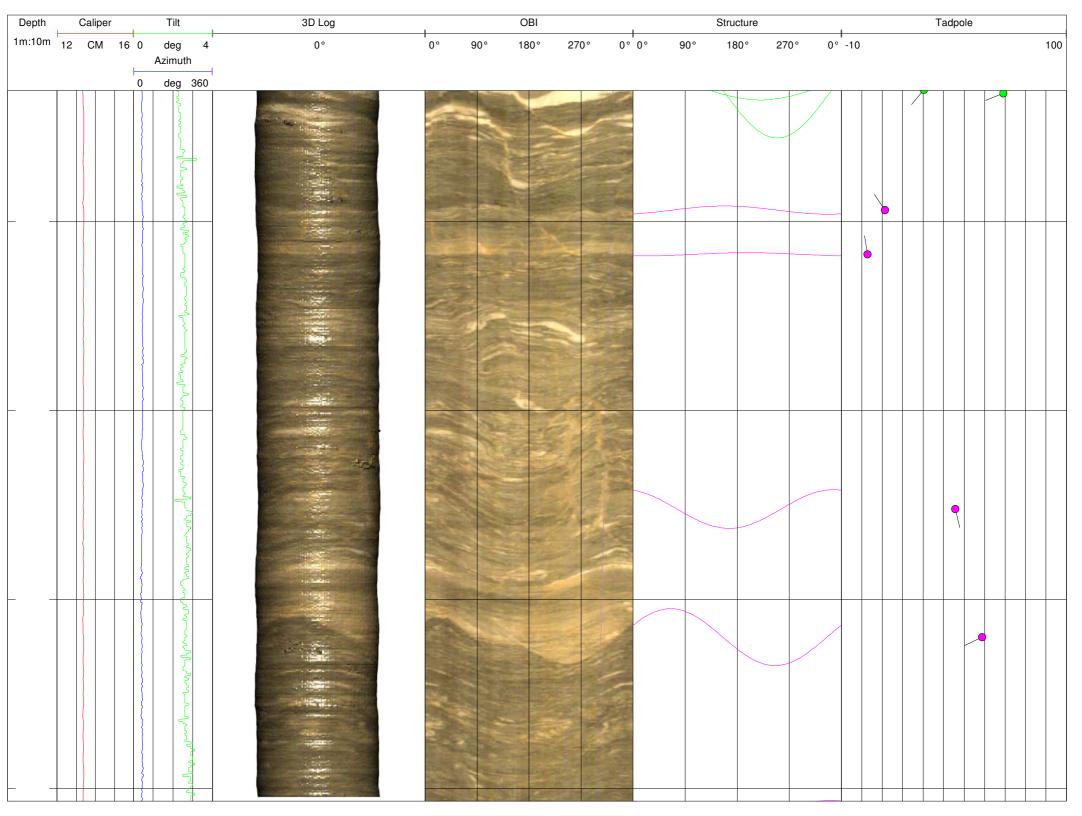


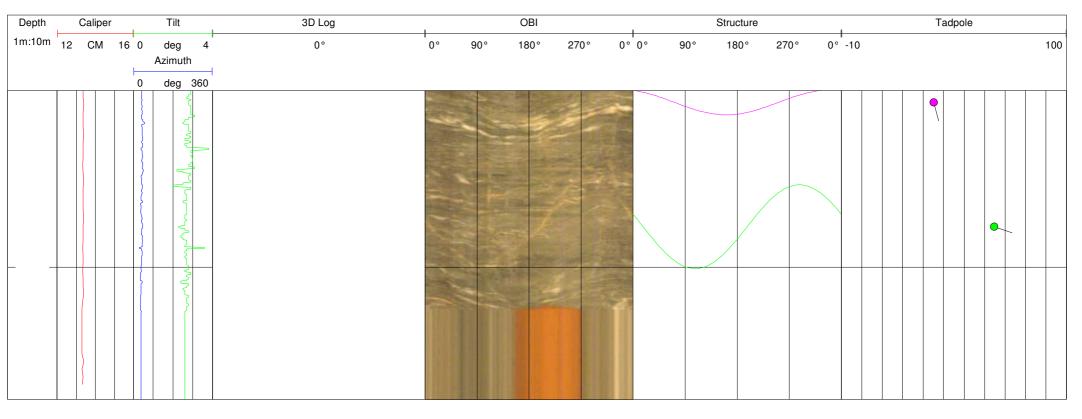




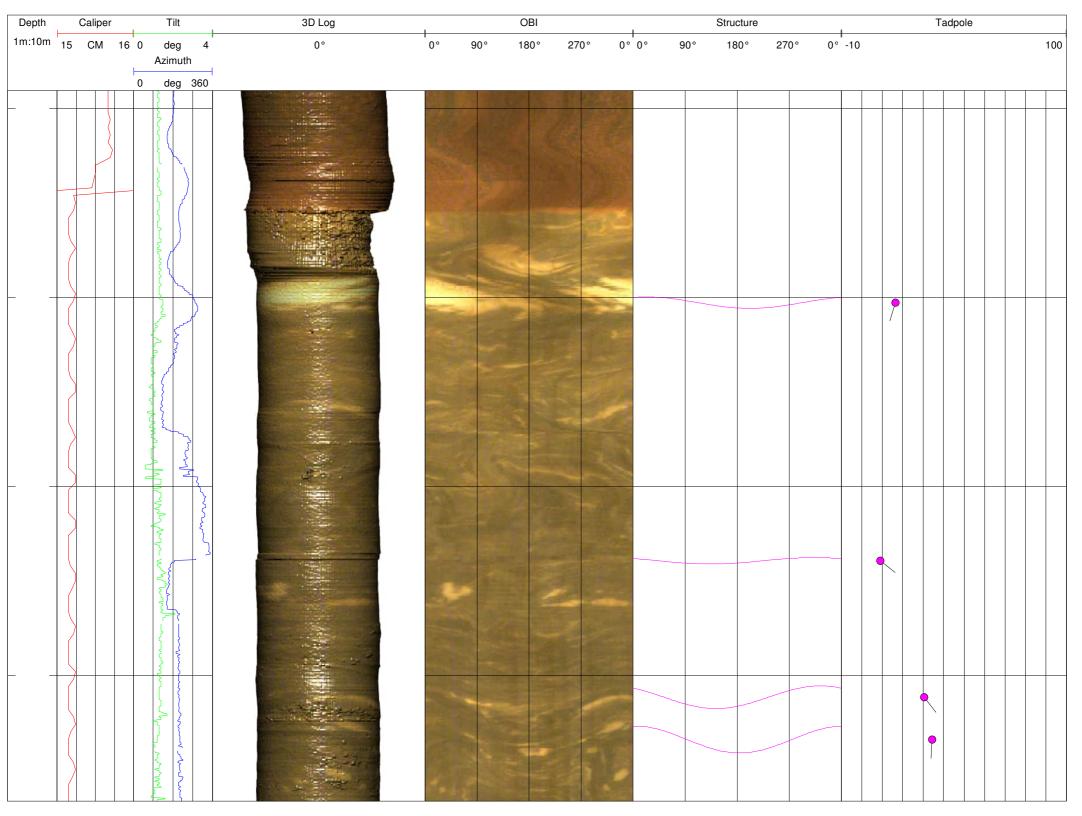


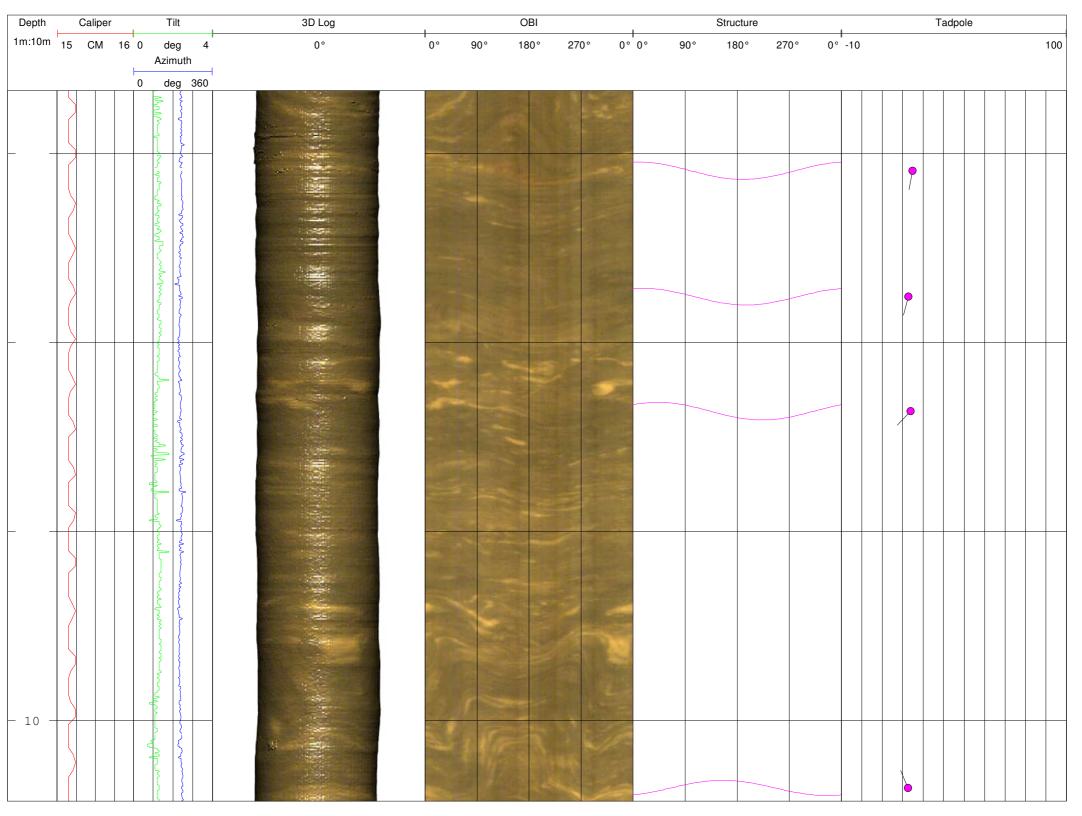


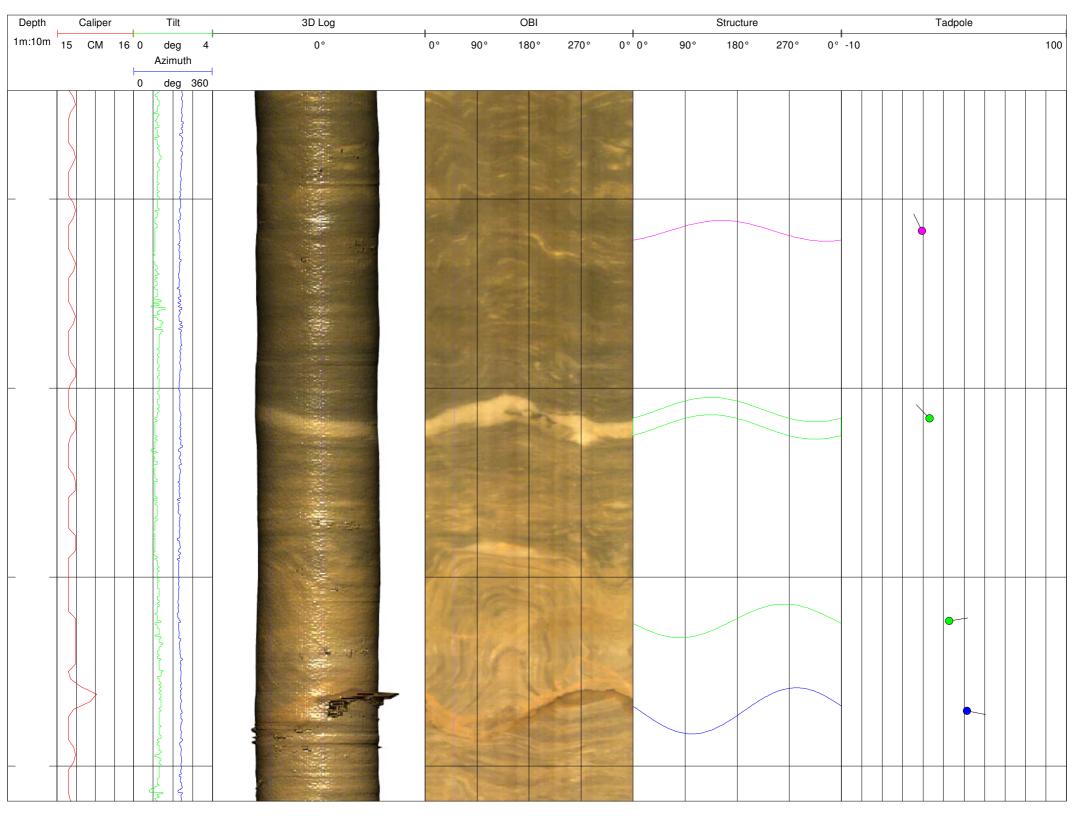


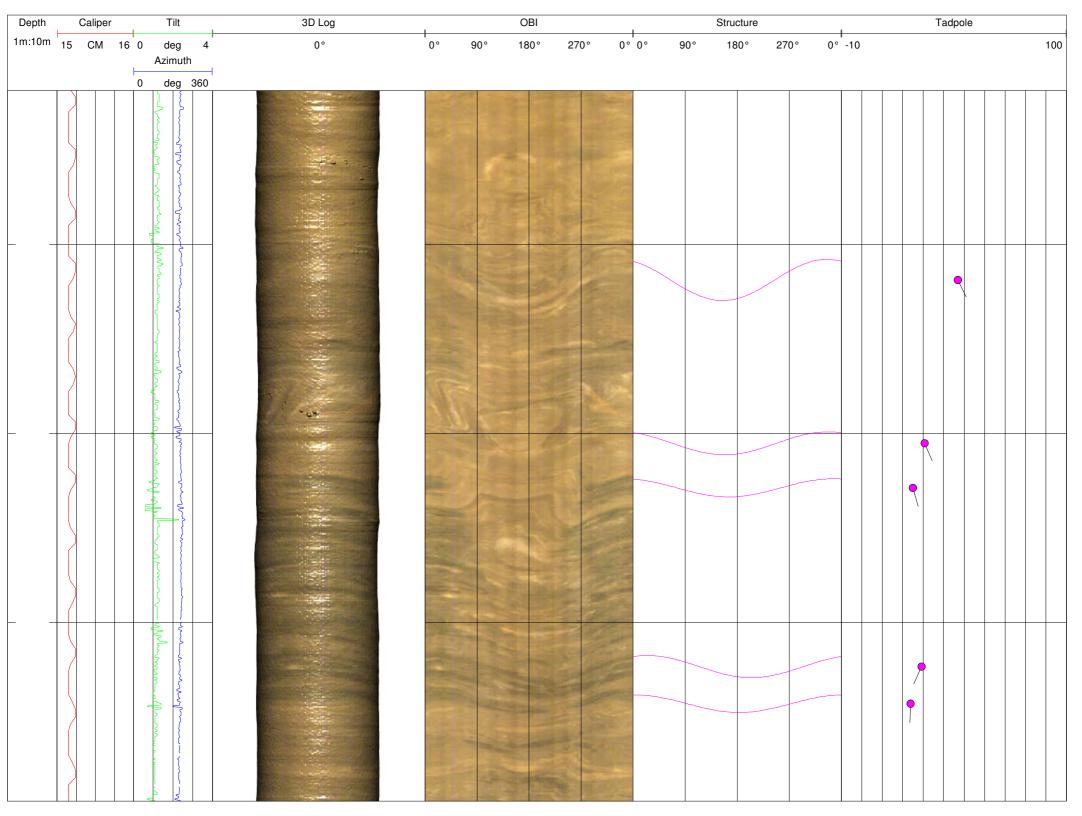


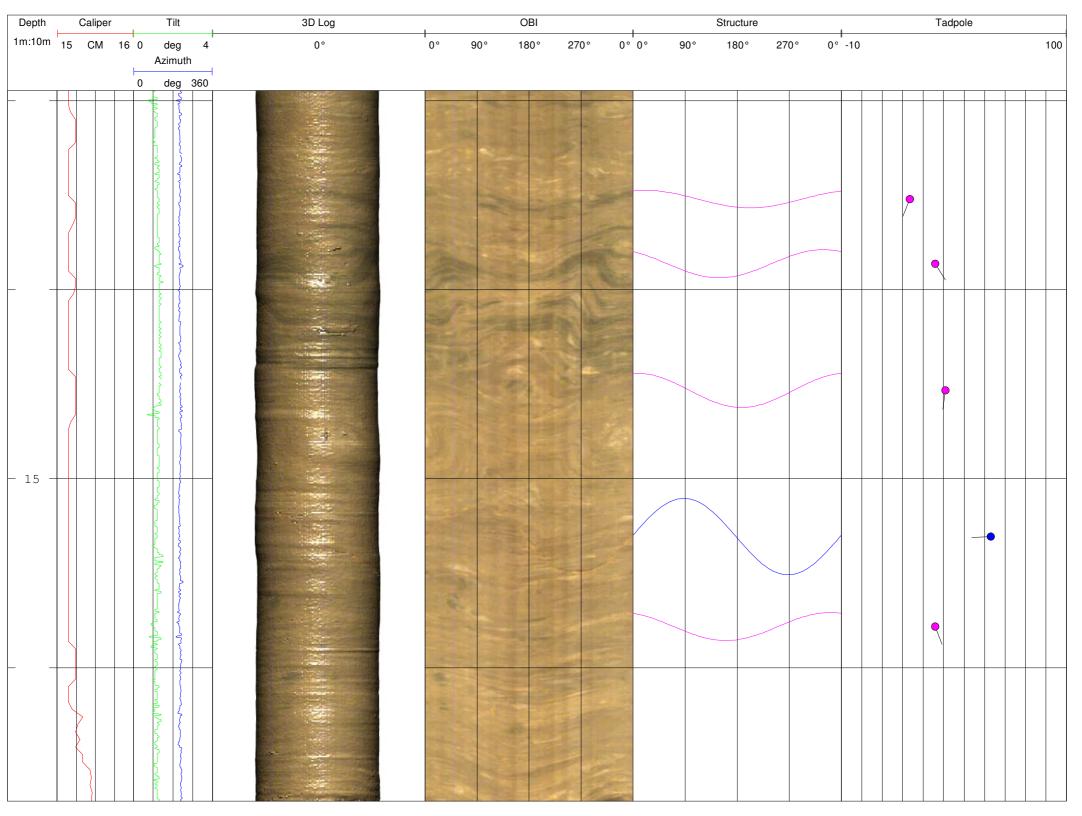
-Fu	RO	Fugro Engi	neering Service	ces												
	Client: Scottish and Southern Energy PLC Borehole: BH3		Southern Energy PLC	Log Type:												
V			Optical Televiewer Log													
Duningto OO	NA OCCUPATION		БПЗ		0											
						Approved:										
Location: Sloy Grid Reference: Elevation:																
Drilled Depth:	35m		Date:	04/03/2010												
Logged Depth:	33.73m		Record	ded By:												
Logging Datum:	Ground Level				Remarks:											
Logged Interval:					North reference i	s magnetic, Tadpo	le log and tabulate	ed data is correct	ed for borehole de	viation						
Fluid Level:																
Structure Key:	Foliatio	n — Fracture —	Vein													
BOREHOLI	E RECORD				CASING	RECORD										
Bit Diameter:	From:		To:	Туре		Size		From		То						
150mm	0m		8m		Steel		150mm		0m		6.8m					
120mm	6.8m	35.0m														
1	Caliper	Tilt	3D Log		OBI		Structure				Tadpole					
1m:10m 15	CM 16 0	deg 4	0°	["] 0° 90°	180° 270)° 0° 0°	90°	180°	270° 0°	· -10				100		
	0	Azimuth deg 360														
			. = 1 2			1										

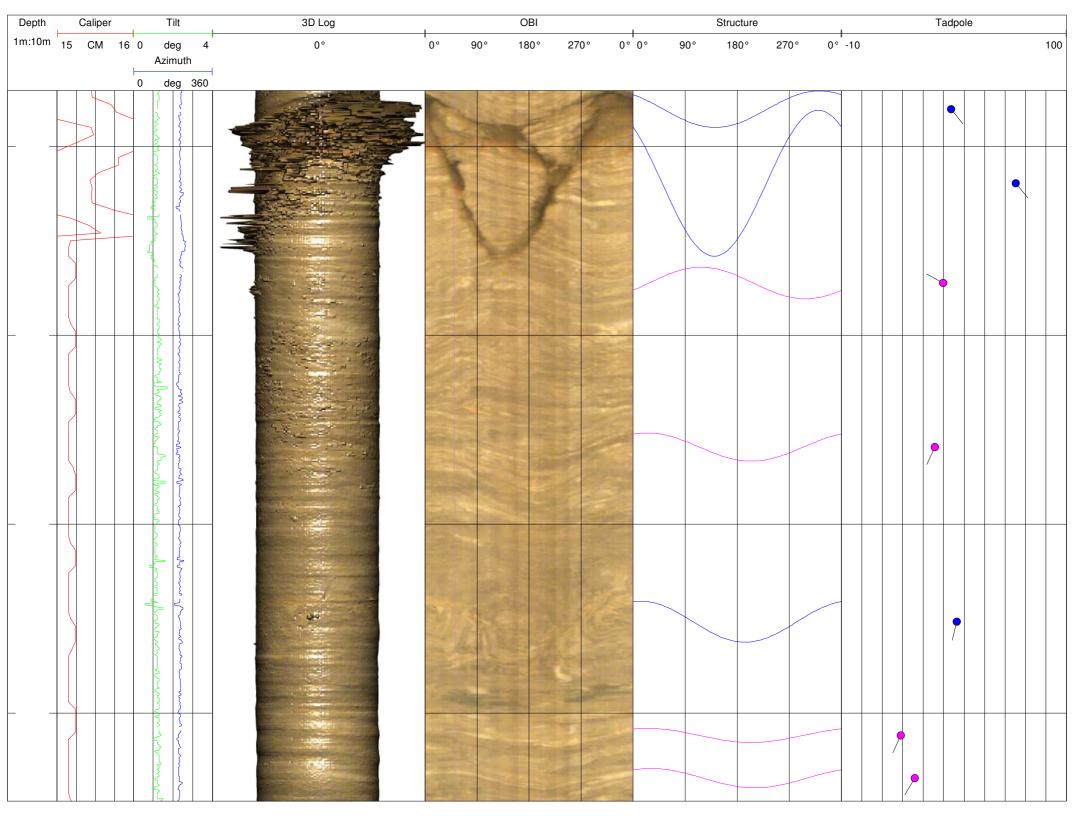


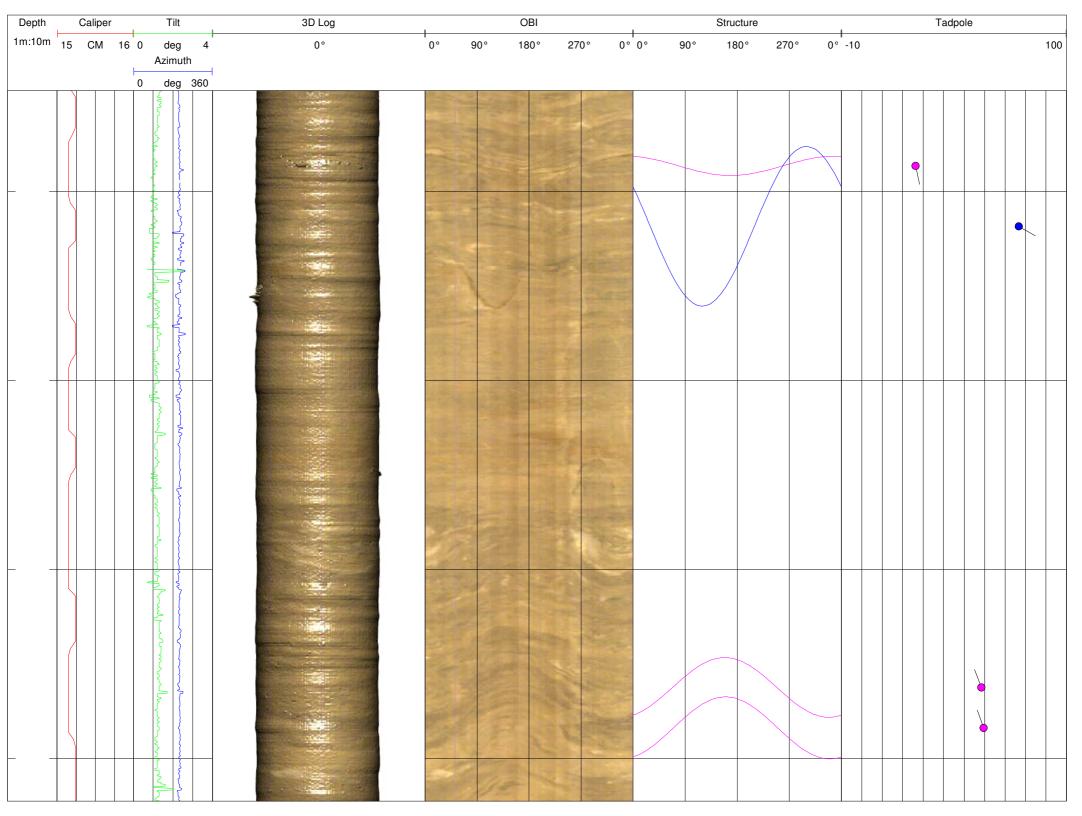


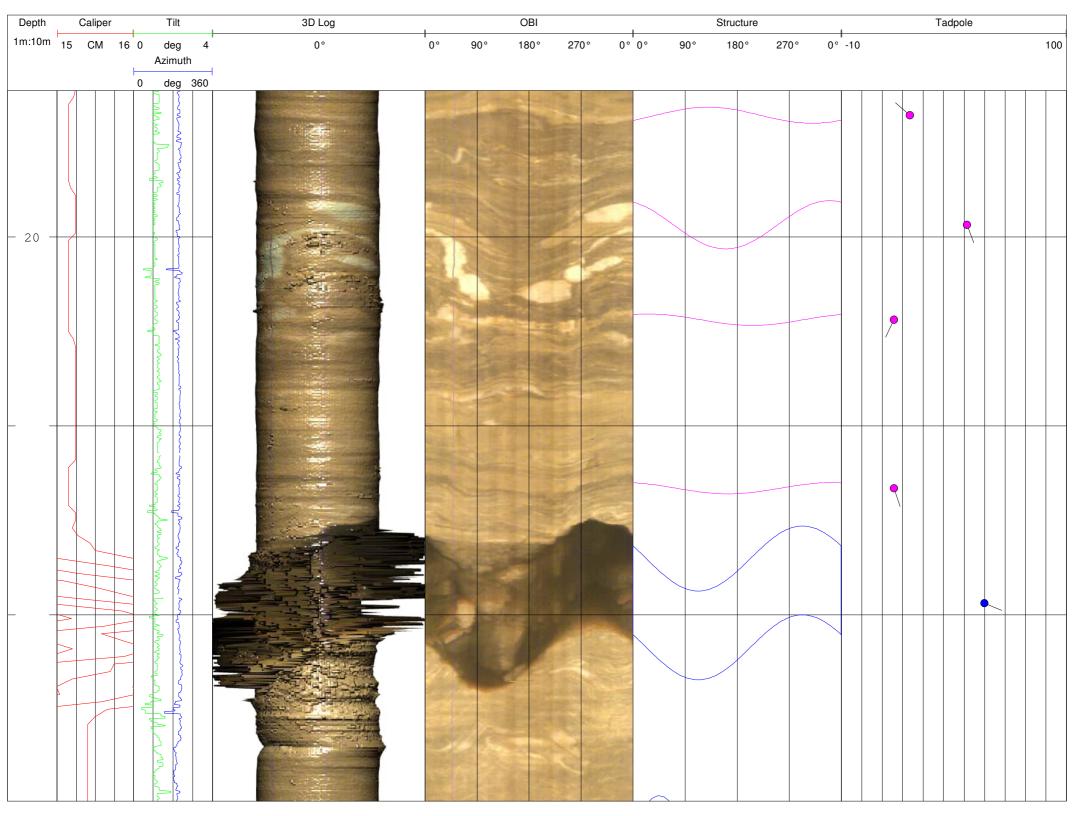


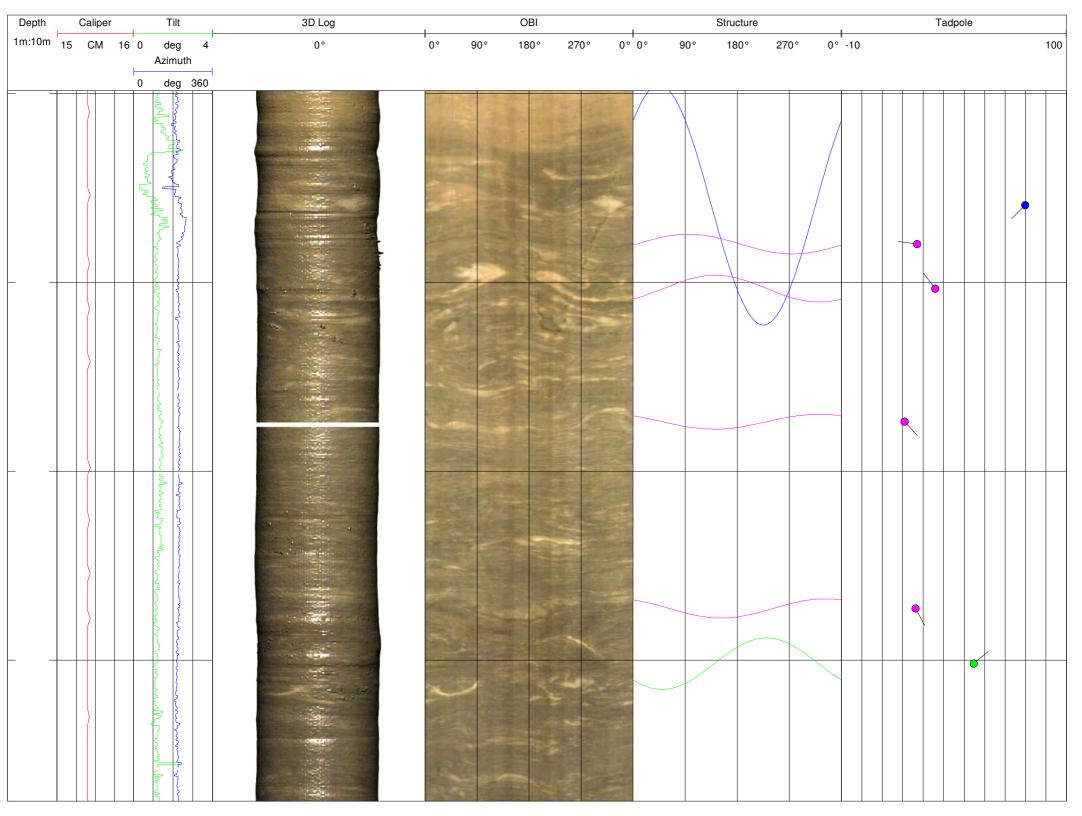


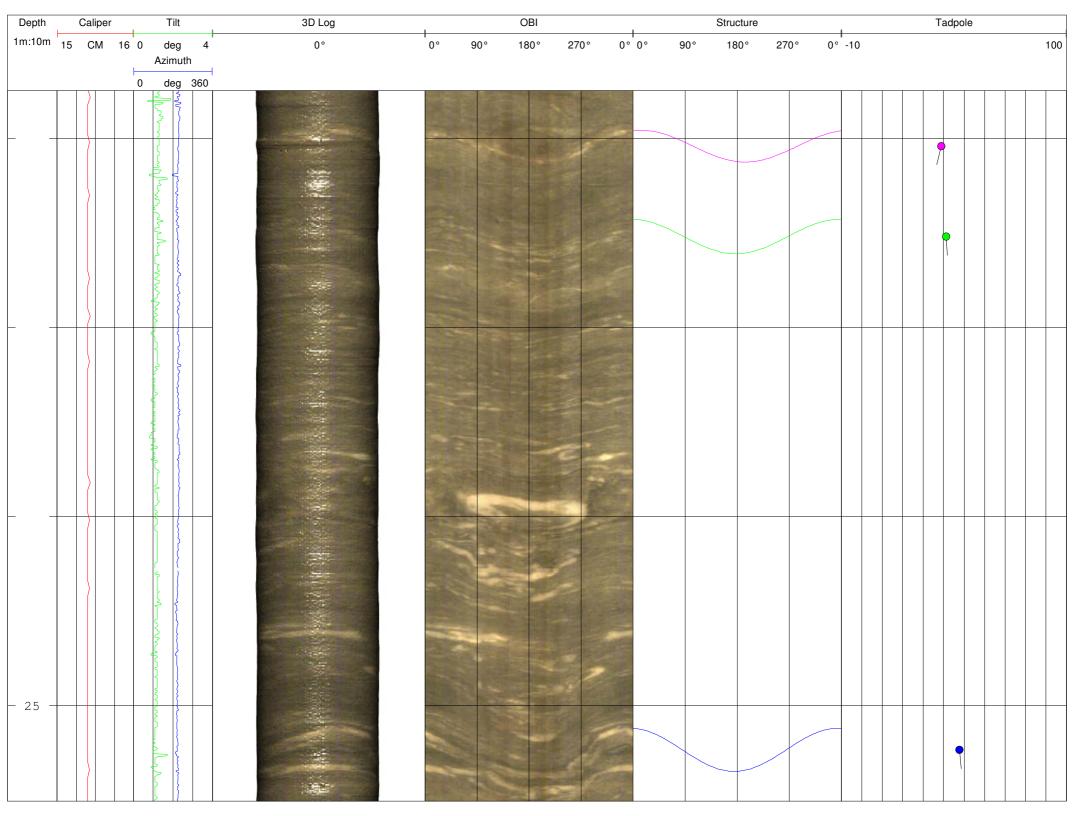


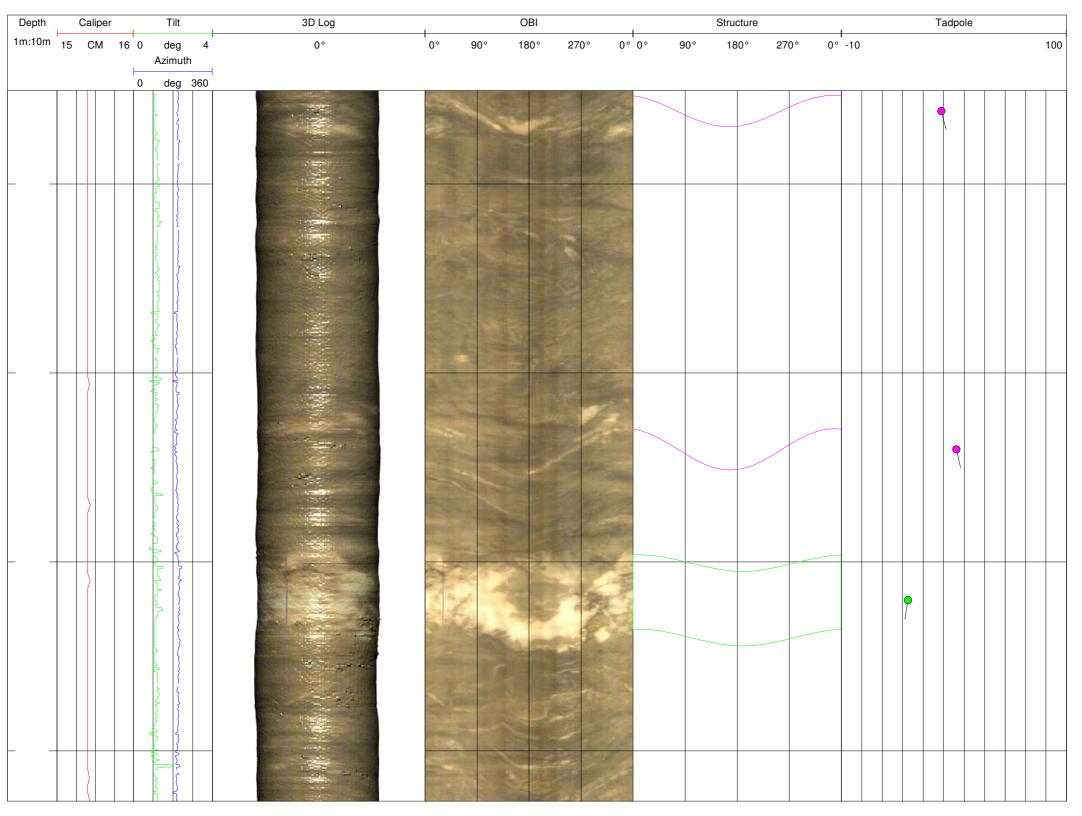


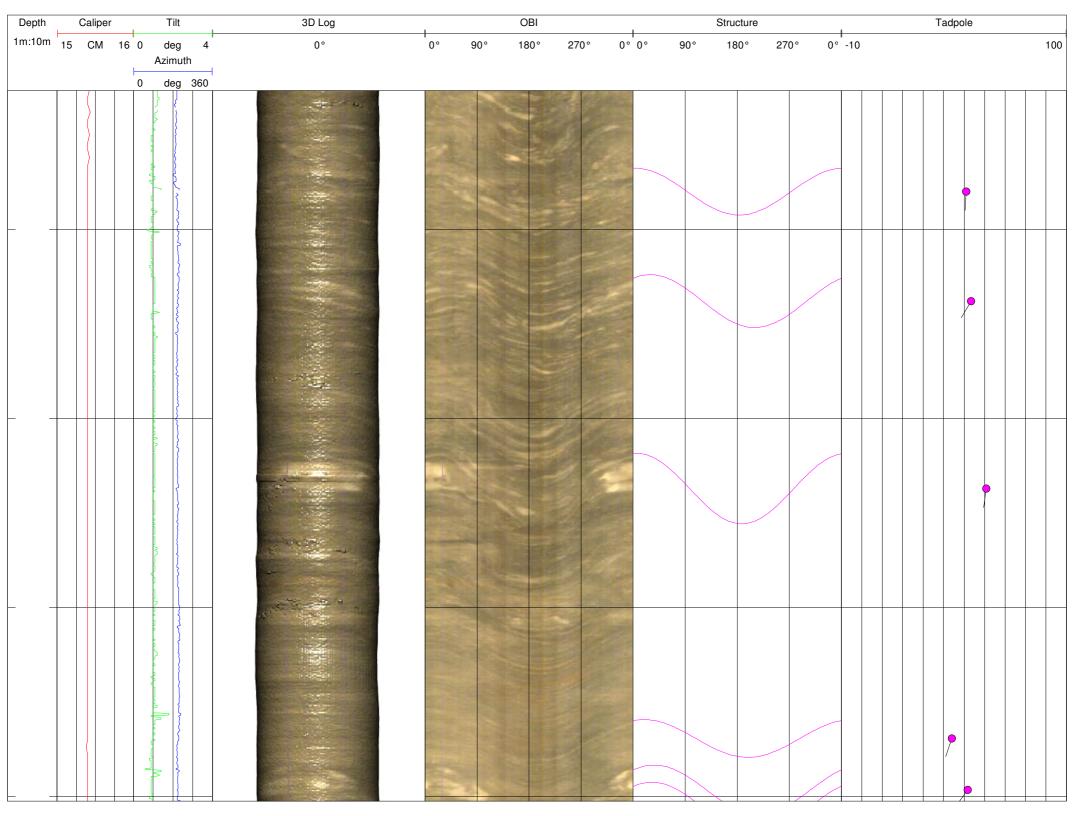


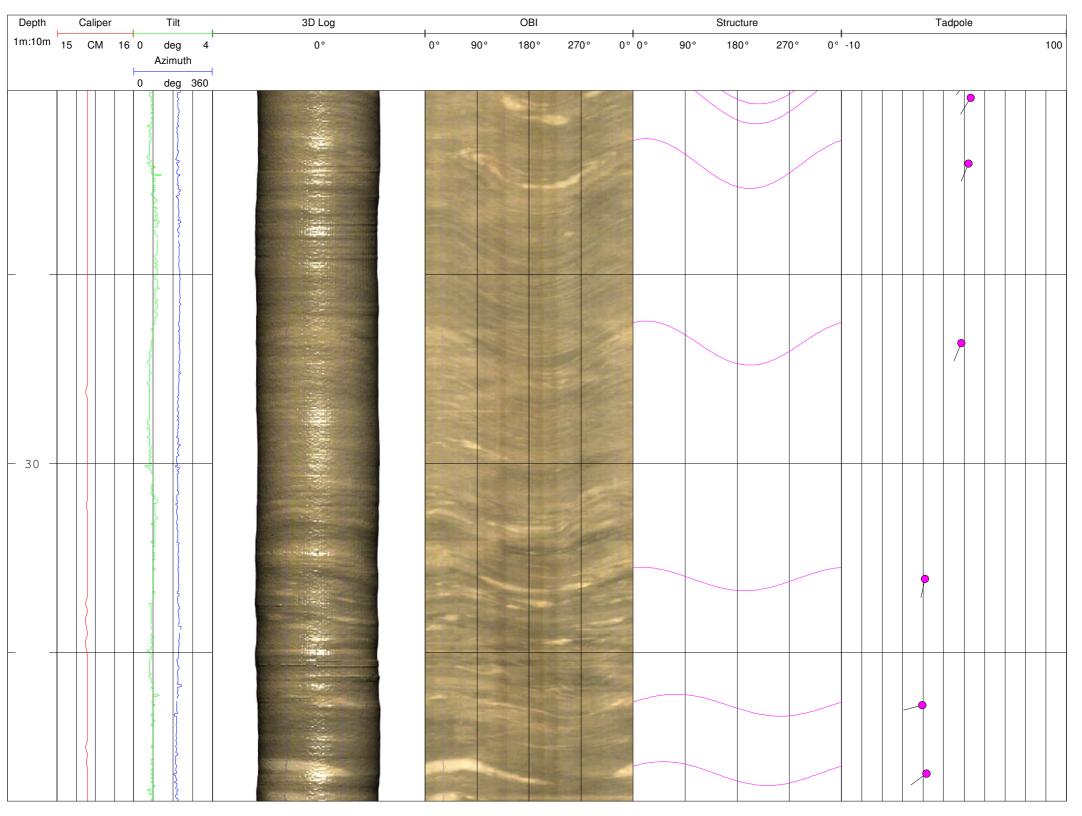


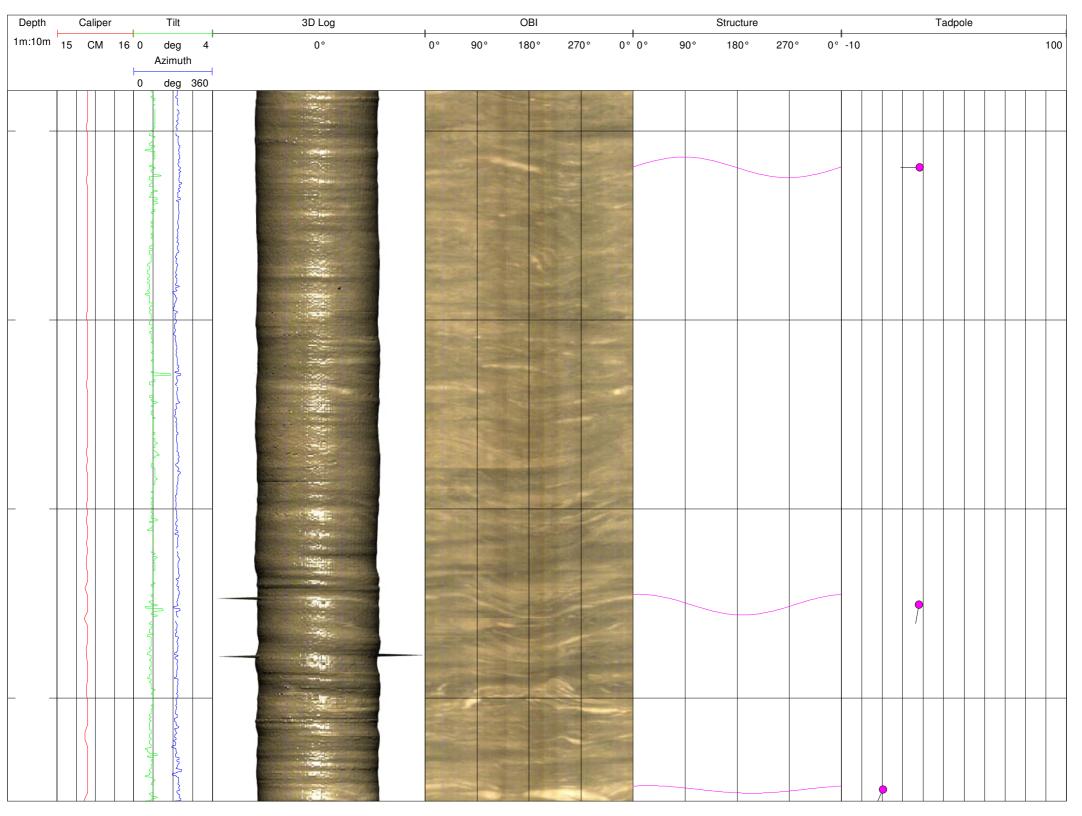


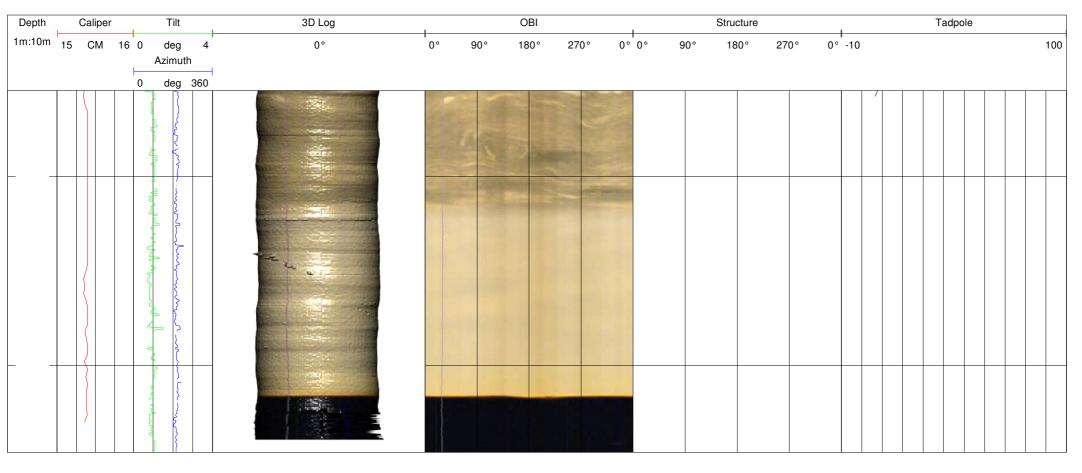




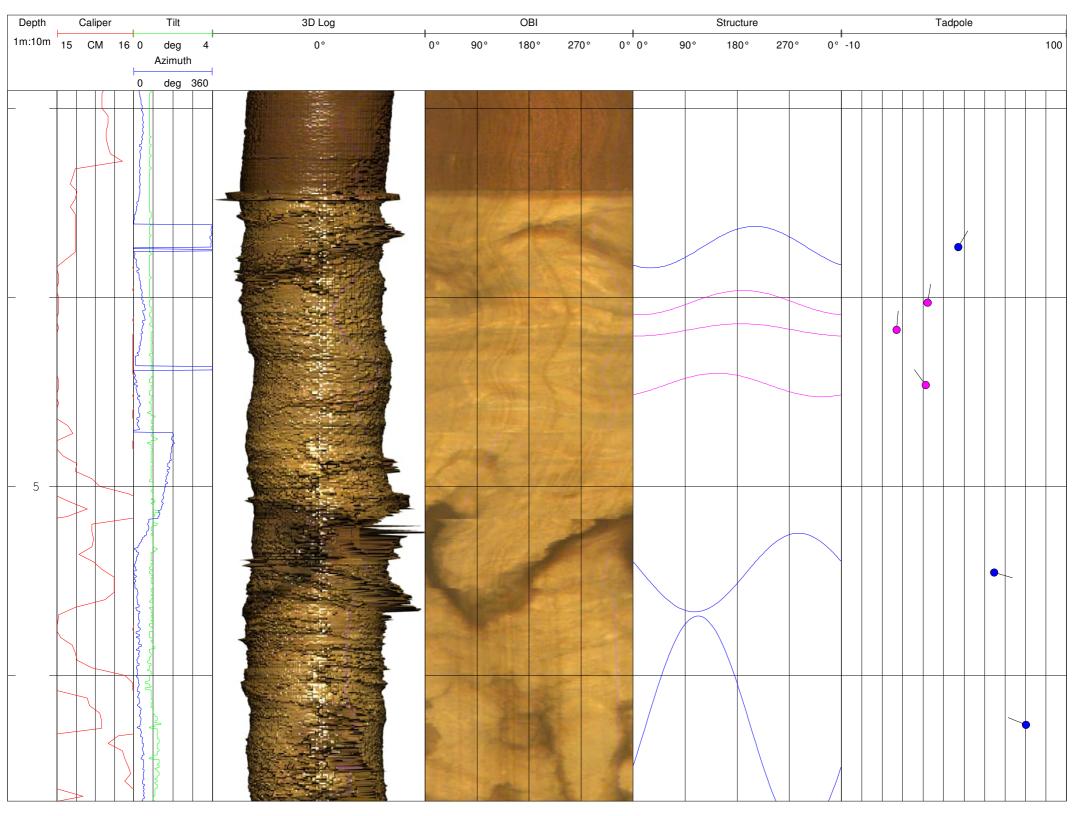


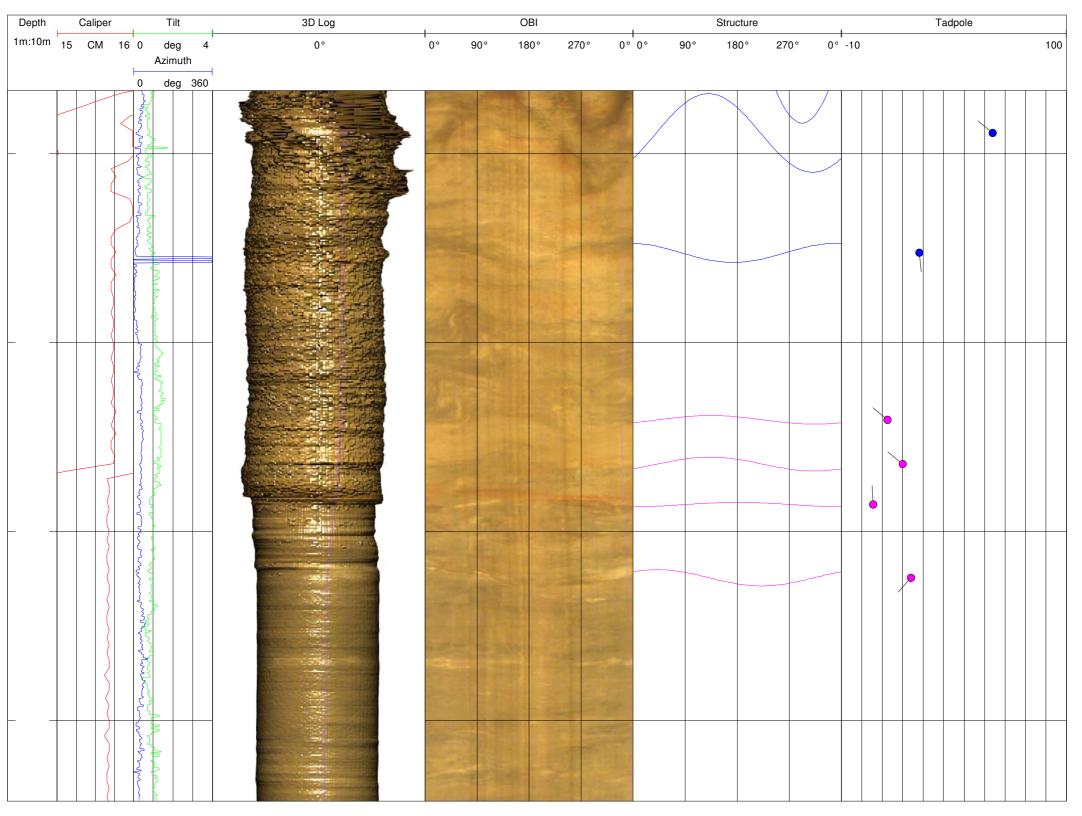


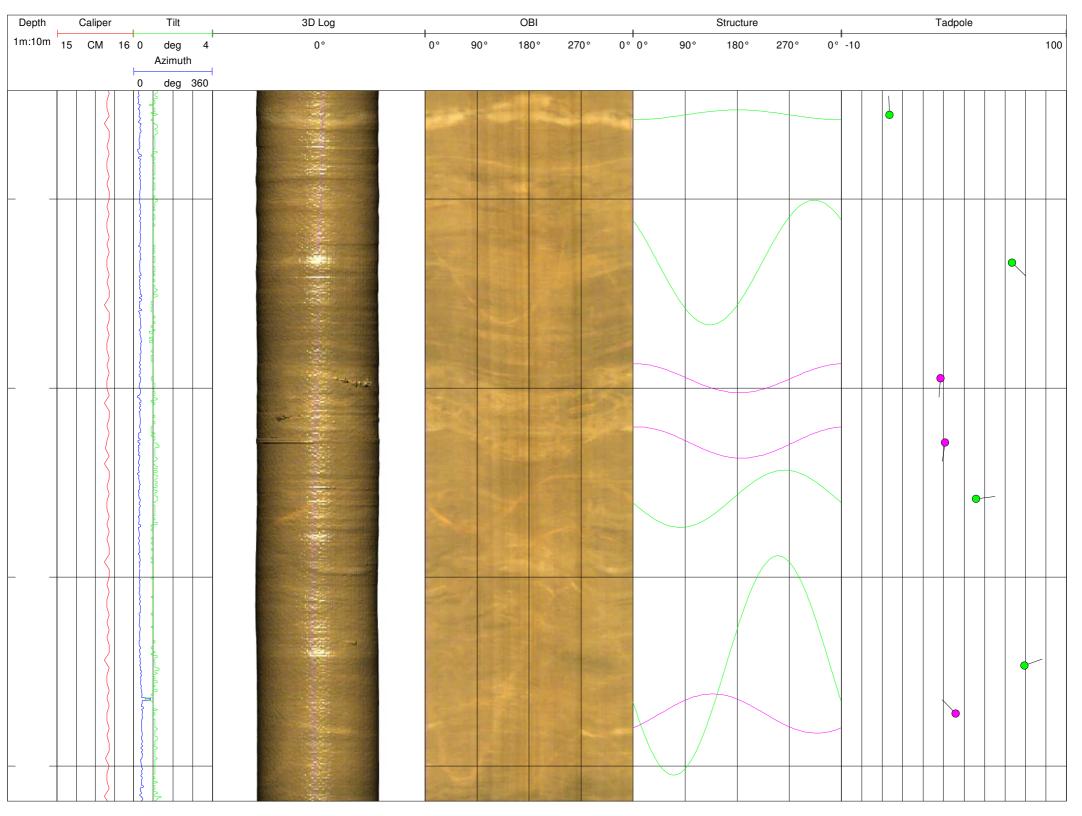


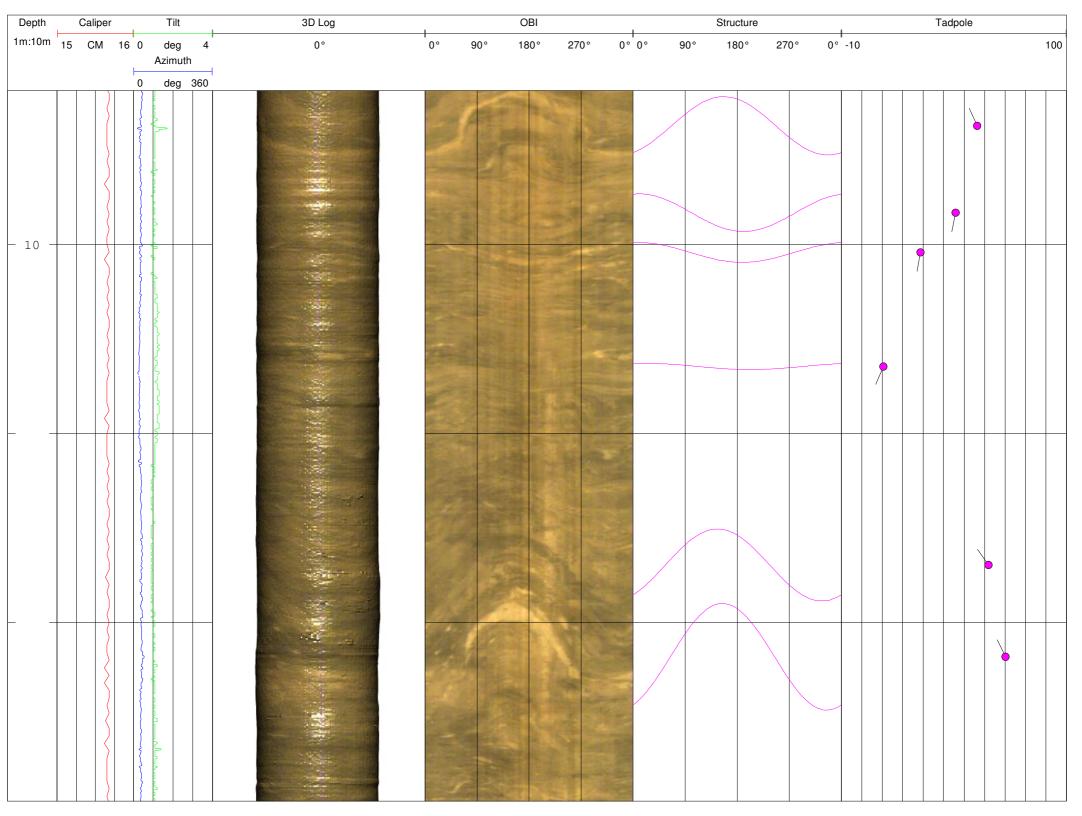


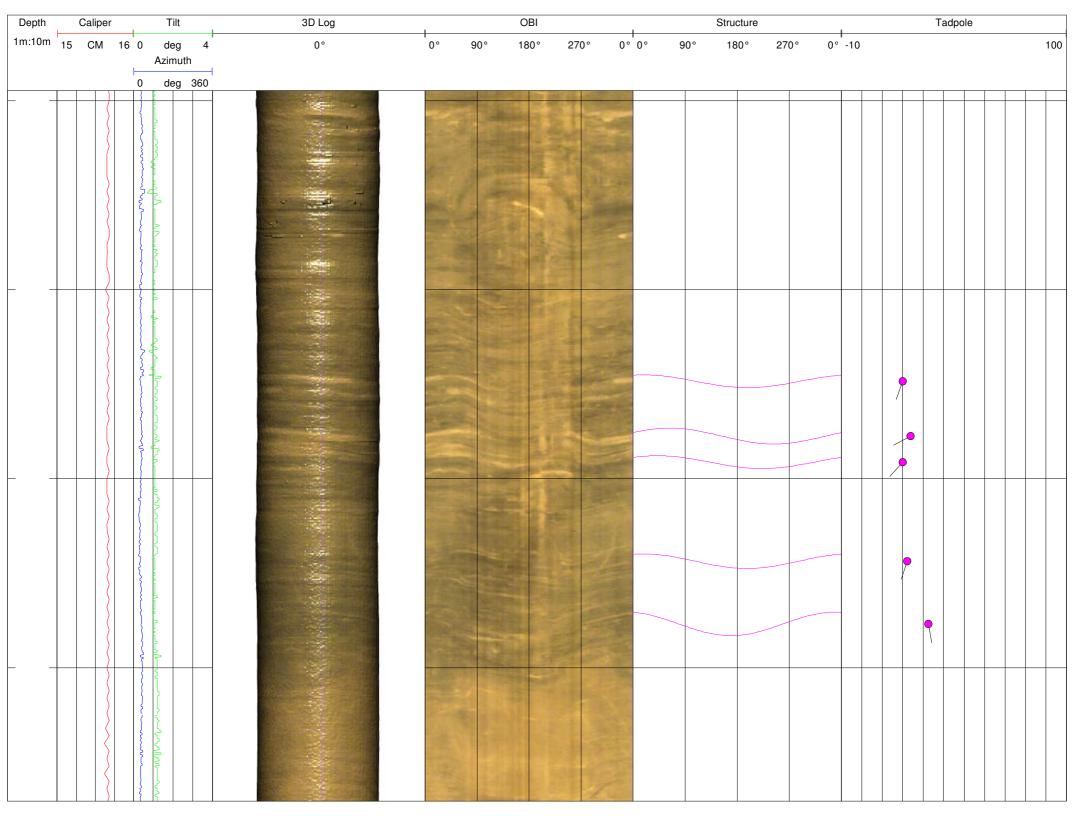
-Fu	GRO	Fugro Engi	neering Services	S												
	Client: Scottish and Southern Energy PLC			Log Type:												
-	$= \stackrel{\sim}{\sim} =$	Borehole: BH4				Optical Televiewer Log										
Location: Sloy Grid Reference: Elevation:						Approved:										
Drilled Depth:	35m		Date:	04/03/2010												
Logged Depth:	33.97m		Recorded B													
Logging Datum:					Remarks:											
Logged Interval:					North reference is a	nagnetic, Tadpo	le log and tabulate	d data is correct	ed for borehole de	viation						
Fluid Level:					-											
Structure Key:	Foliation	n — Fracture —	Vein		-											
BOREHOL	E RECORD				CASING R	ECORD										
Bit Diameter:	From: To:				Type Size			From		То						
150mm	0m		4.2m		Steel		150mm		0m		4.2m	4.2m				
120mm	4.2m 35.0m															
	Caliper	Tilt	3D Log		OBI			Structure			Tadpole					
1m:10m 15	CM 16 0	deg 4	0°	["] 0° 90°	180° 270°	0° 0°	90°	180°	270° 0°	· -10				100		
	0	Azimuth deg 360														

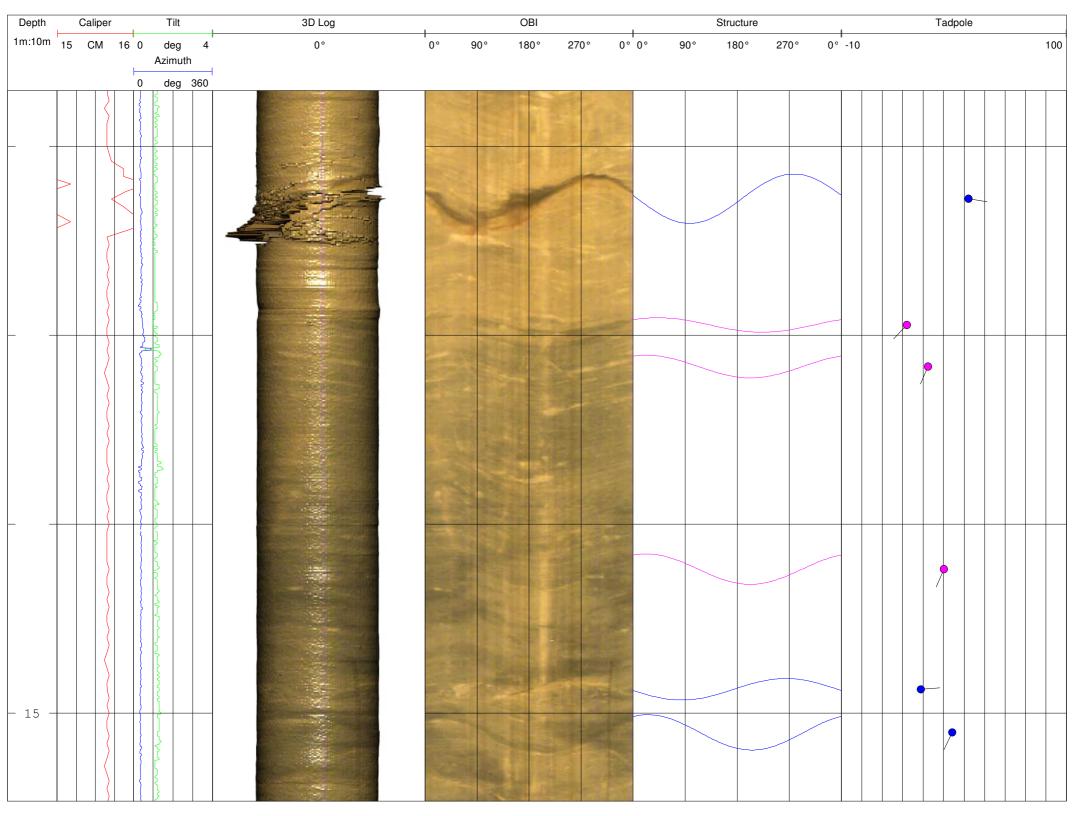


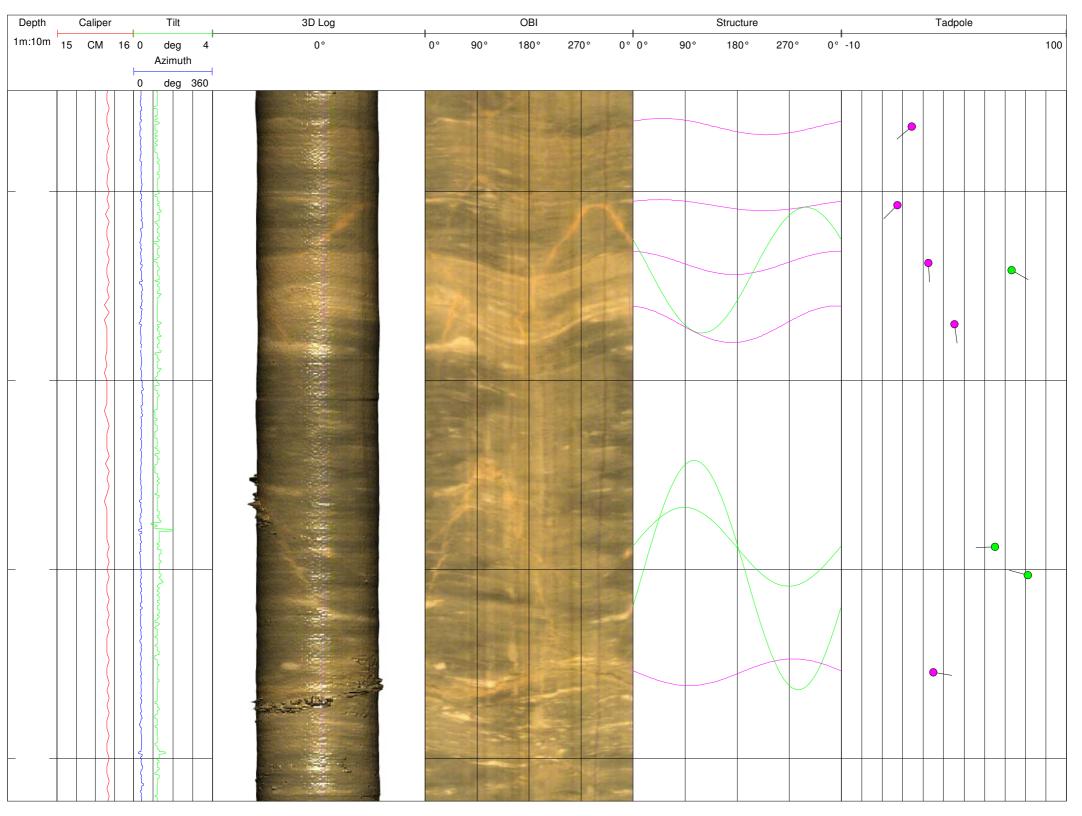


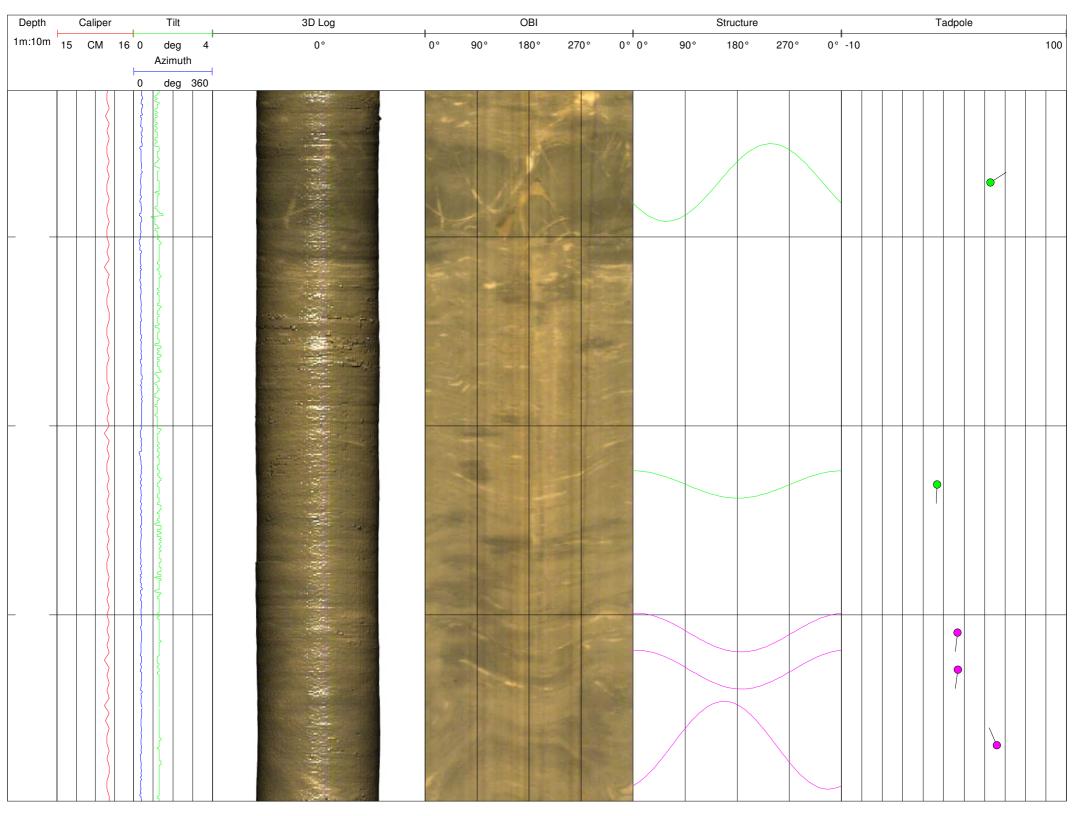


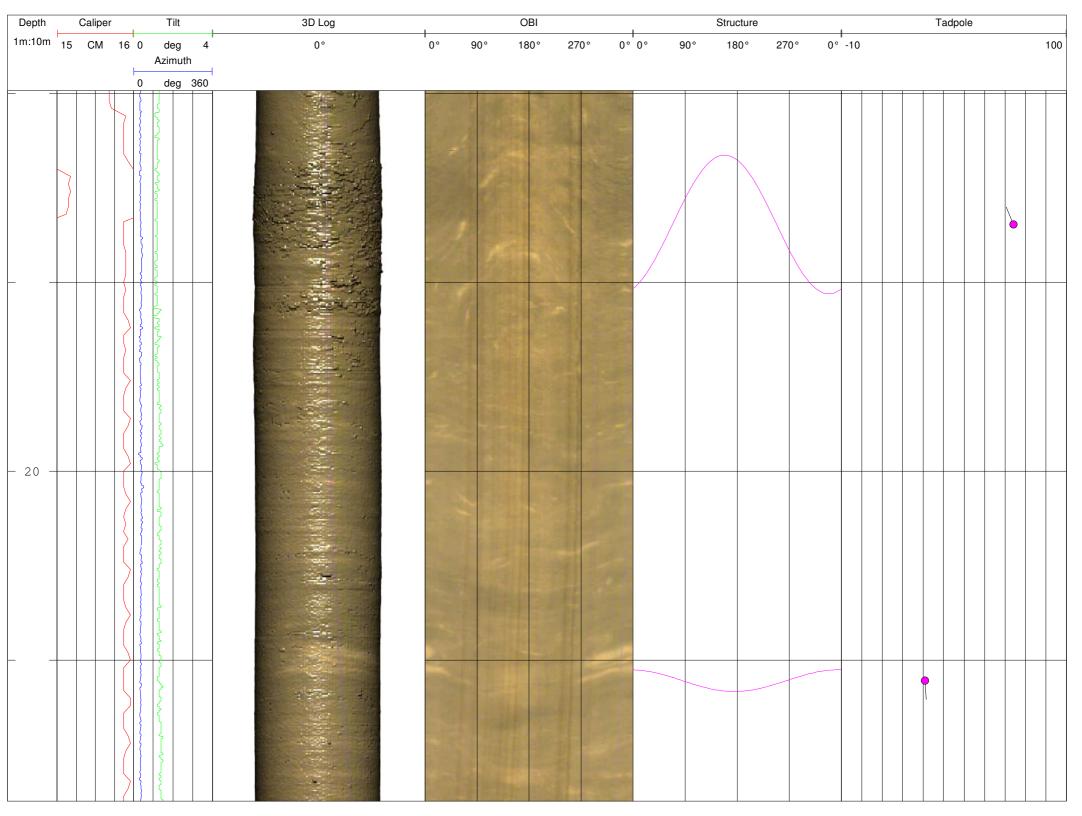


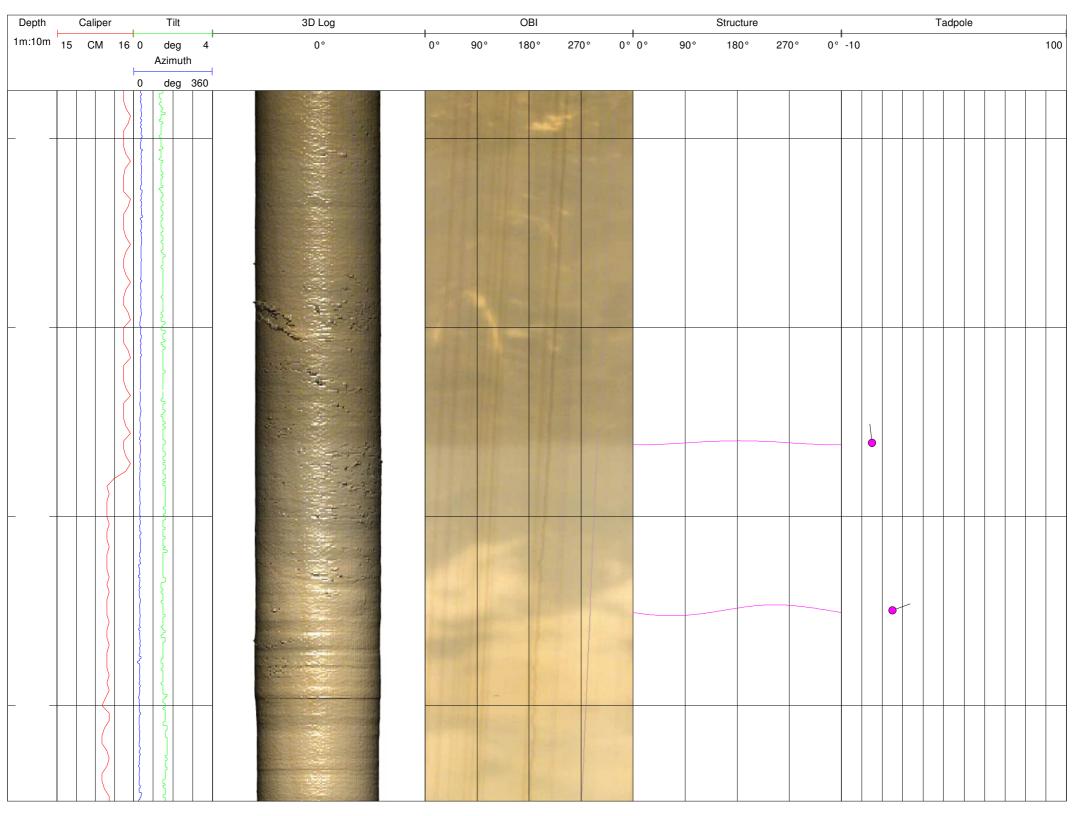


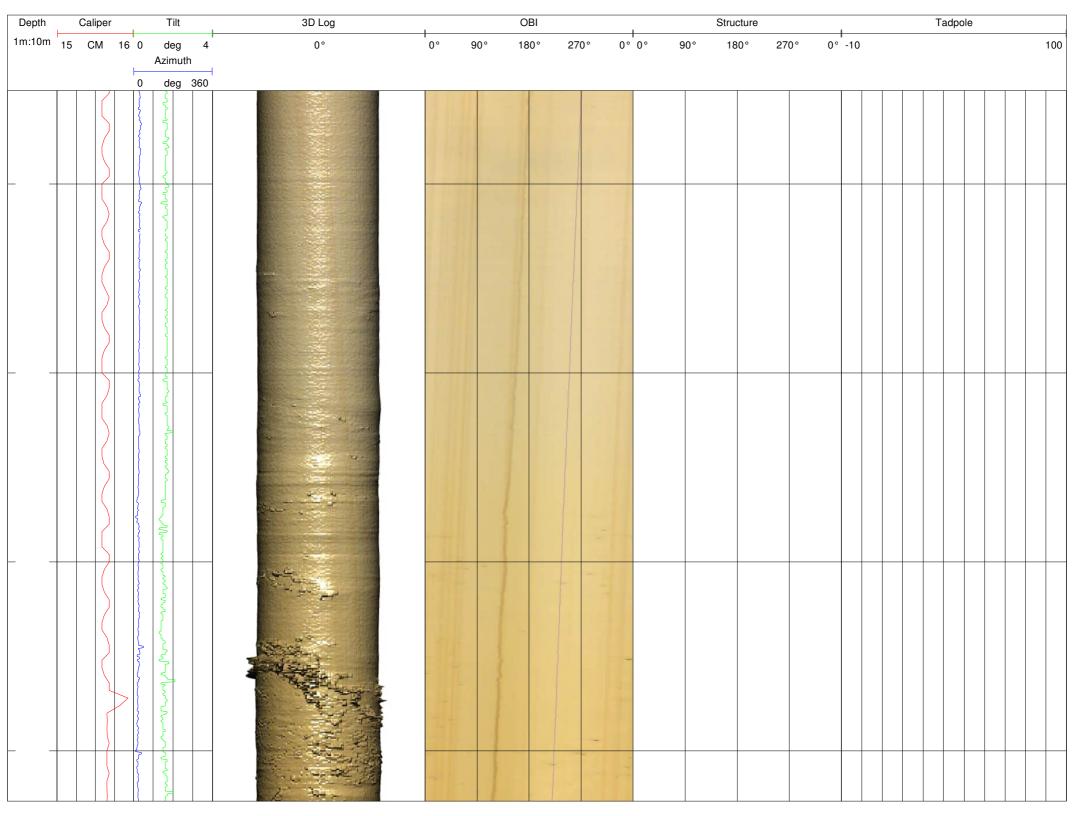


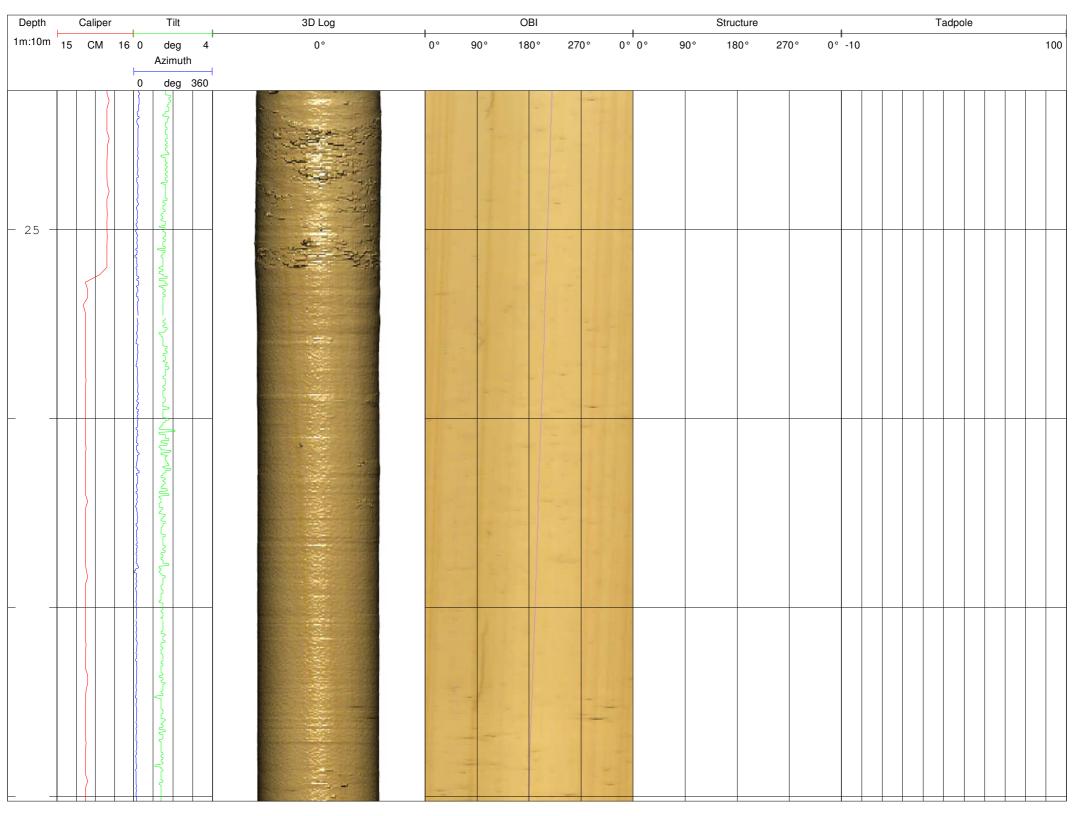


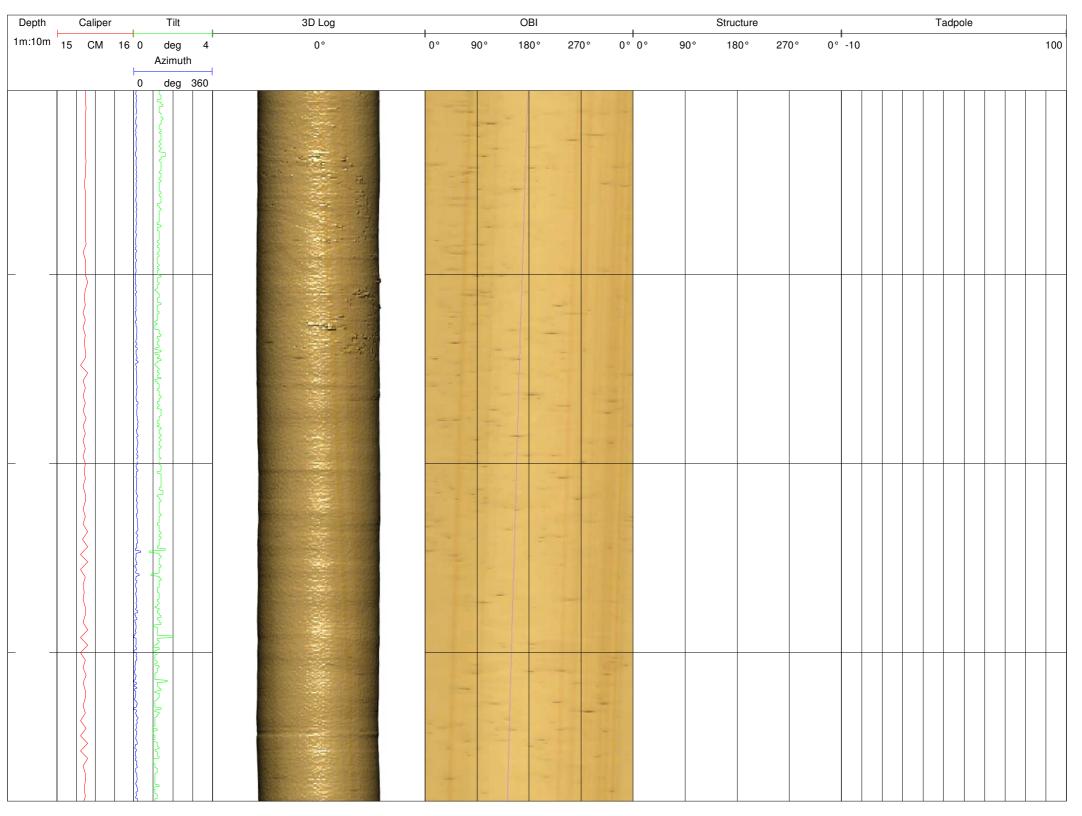


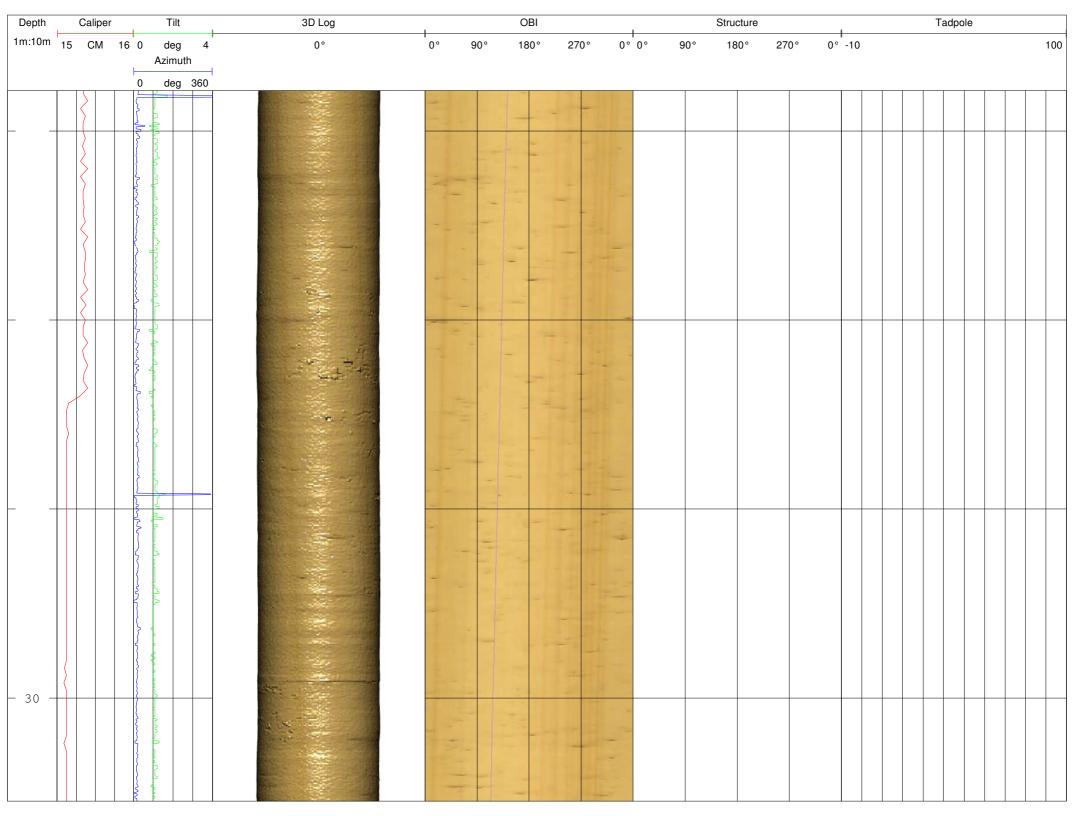


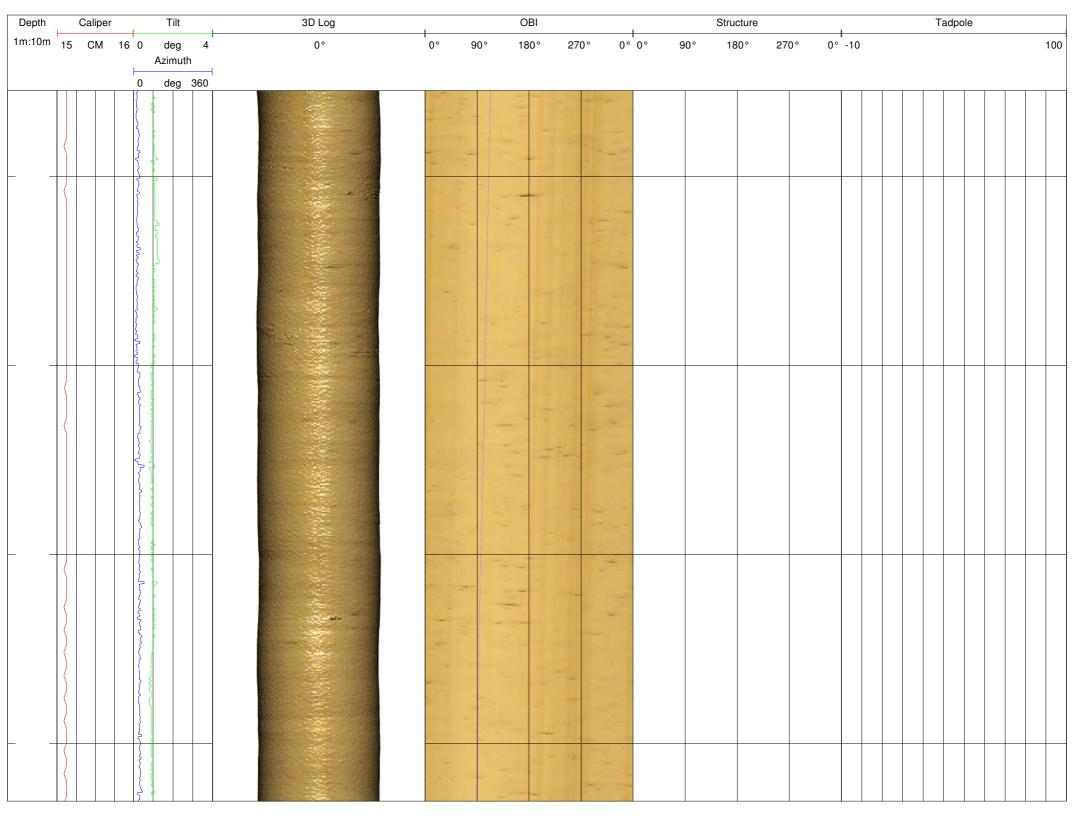


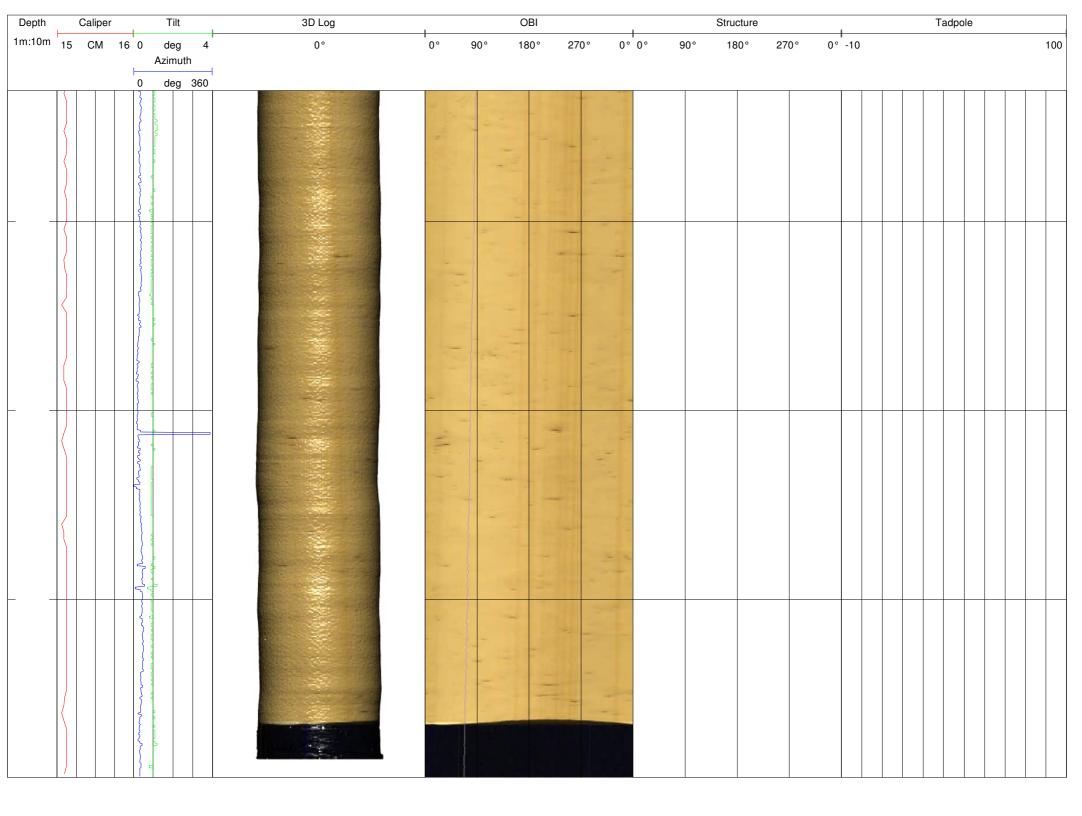




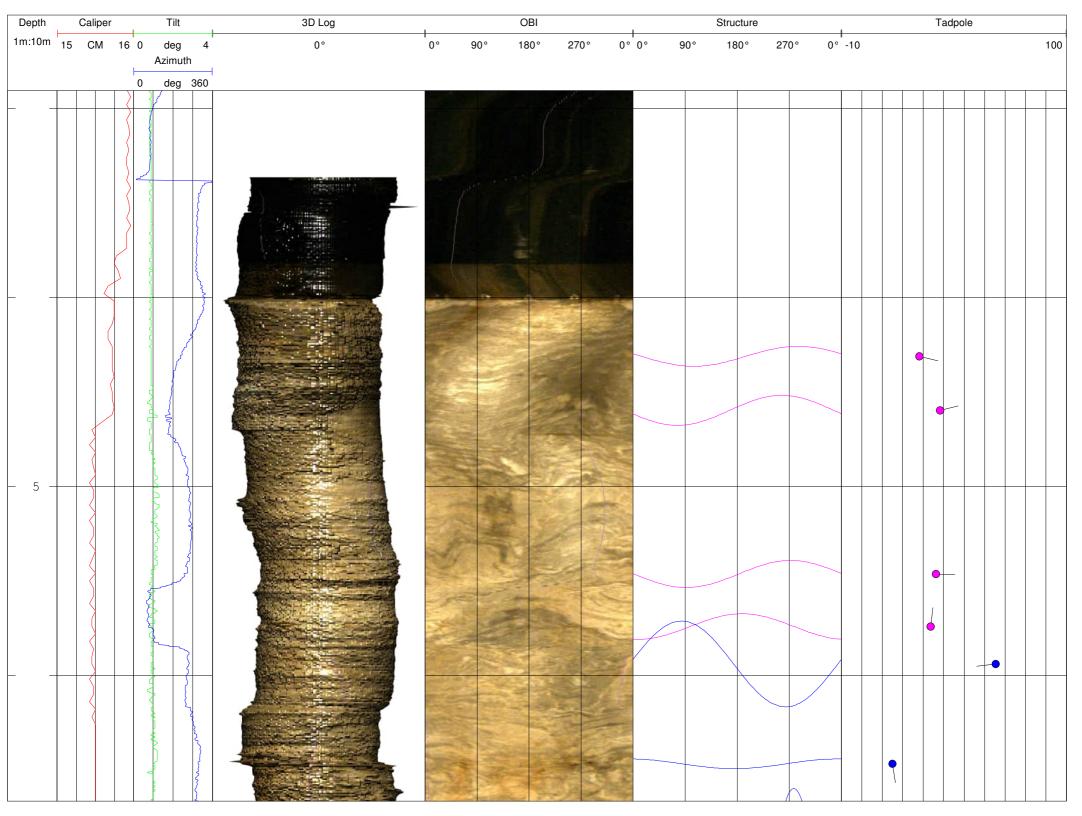


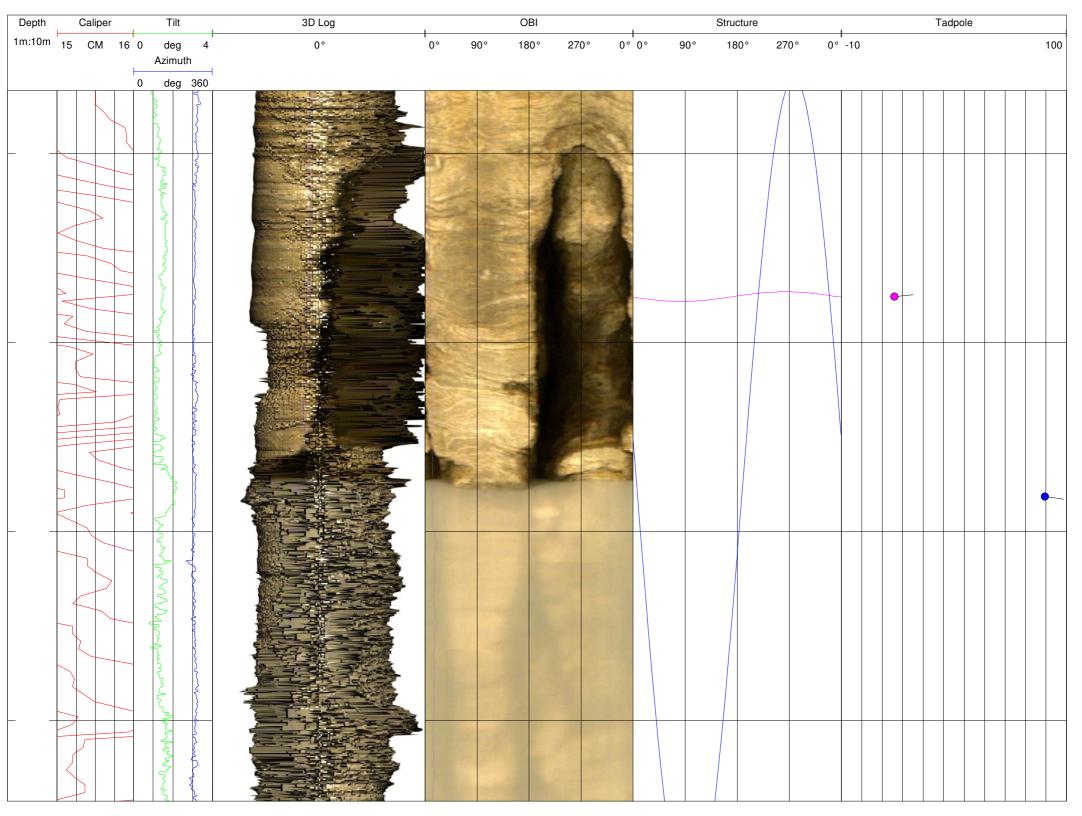


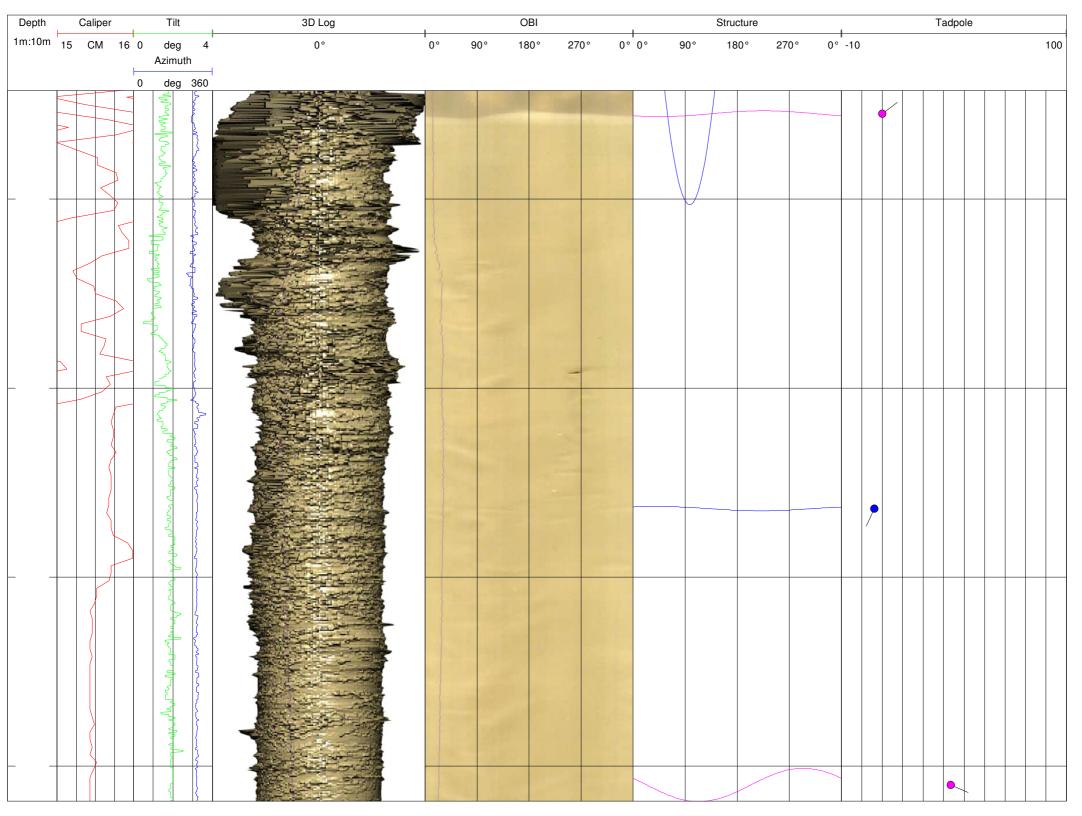


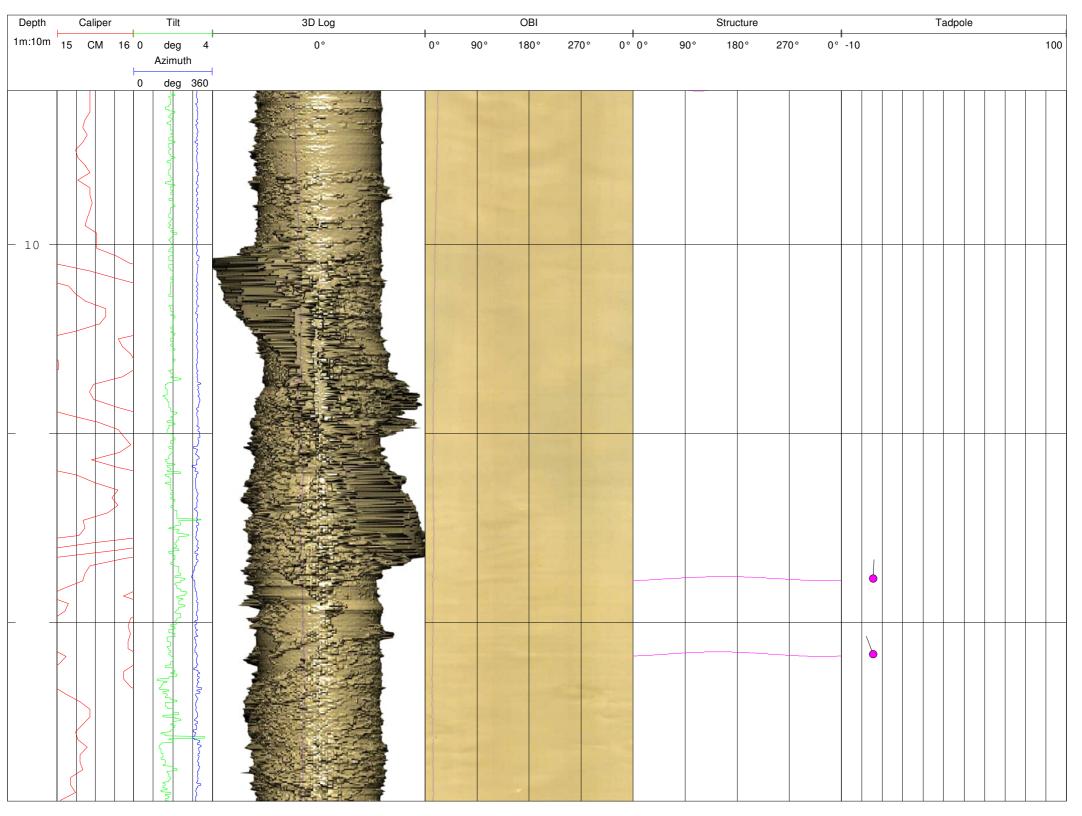


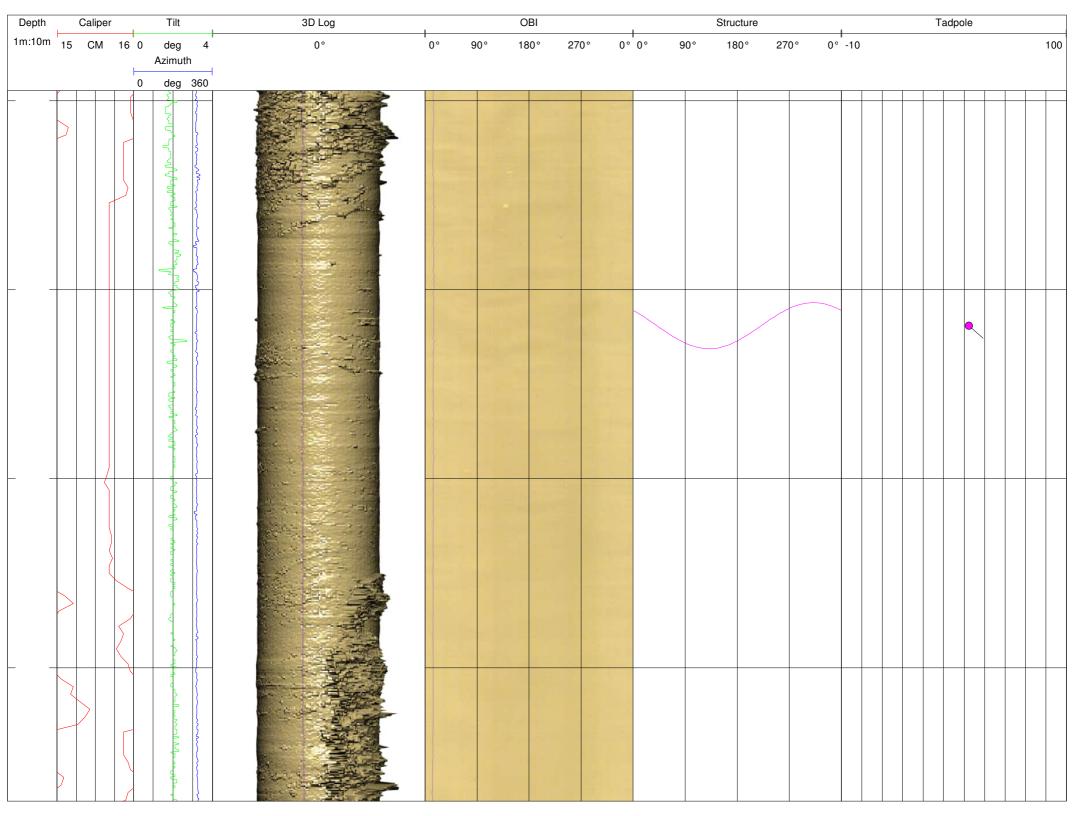
	GRO	Fugro Engi	neering Services	<u> </u>												
	\sim	Client:	Scottish and Southe	ern Energy PLC	Log Type	e:										
٧	$= \hat{\sim} =$	Borehole:	BH6		Optical	Televiewer	Log									
Project: CO	N103001 Slov		Bilo		Approve	d.										
Project: CON103001 Sloy Power Station Location: Sloy Grid Reference: Elevation:						Approved:										
Drilled Depth:	35m	Gild Ne	Date:	04/03/2010												
			Recorded By													
Logged Depth: Logging Datum:	33.76m Ground level		Recorded by		Remarks											
					1		-K- T-d				1					
Logged Interval:					4	_		-	ed data is correct conditions below 6		leviation					
Fluid Level:					Teatures	are difficult to	pick due to	very poor water	onditions below t							
Structure Key:	Foliatio	n — Fracture —	Vein													
BOREHOL	E RECORD				CASII	NG REC	ORD									
Bit Diameter:	From:		То:			Туре		Size		From			То			
150mm	0m		4.5m			Steel		150mm		0m			4.5m			
120mm	20mm 4.5m 35.0m															
1	Caliper	Tilt	3D Log		OBI	OBI Structure			Tadpole							
1m:10m 15	CM 16 0	deg 4	0°	0° 90°	180°	270°	0° 0°	90°	180°	270° ()° -10					100
	0	Azimuth deg 360														

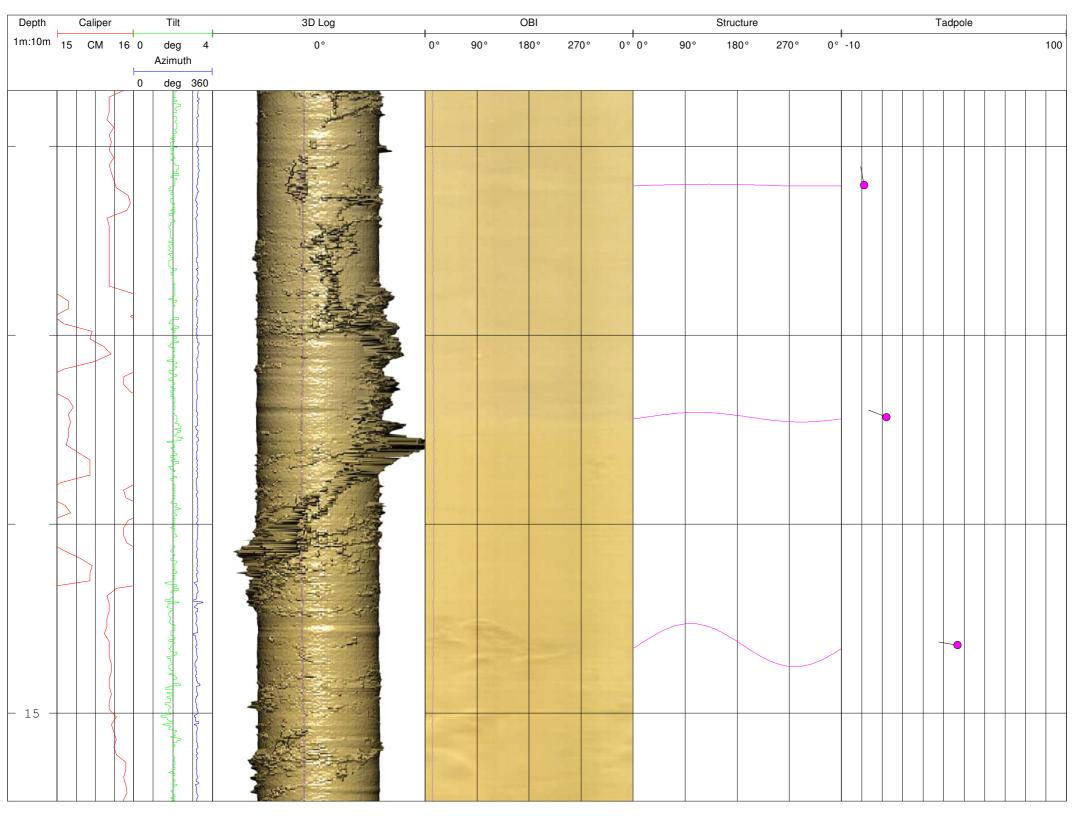


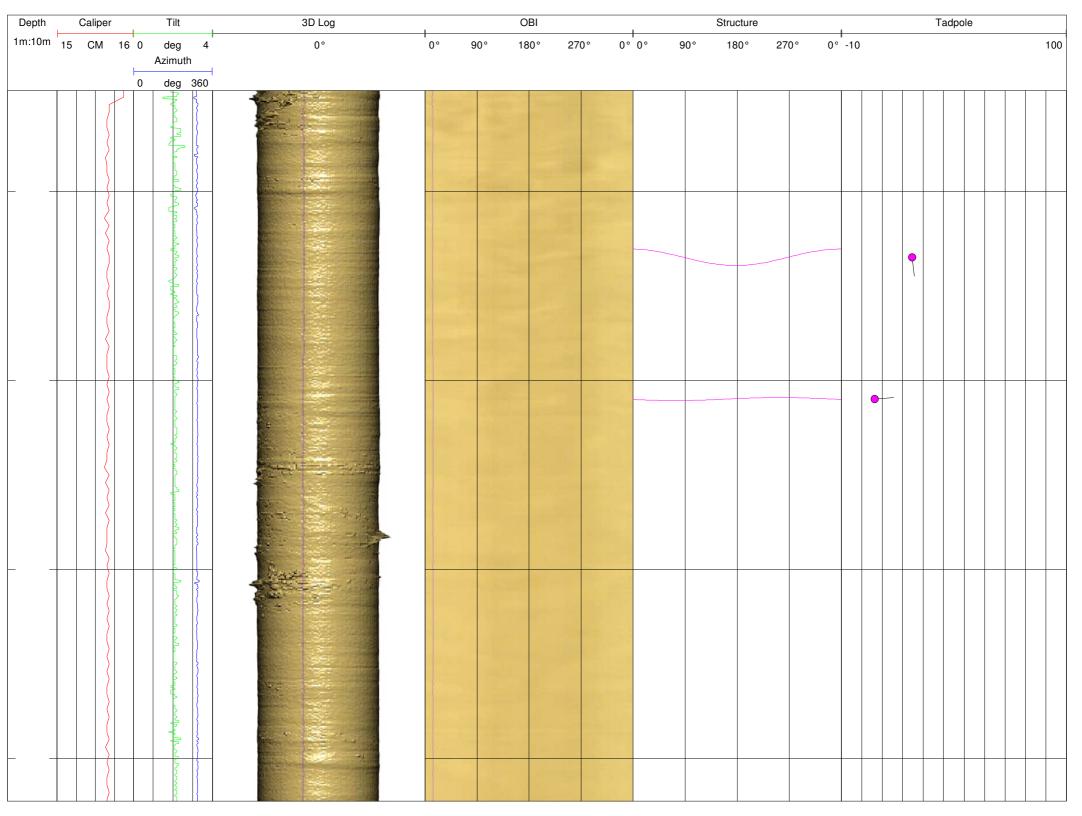


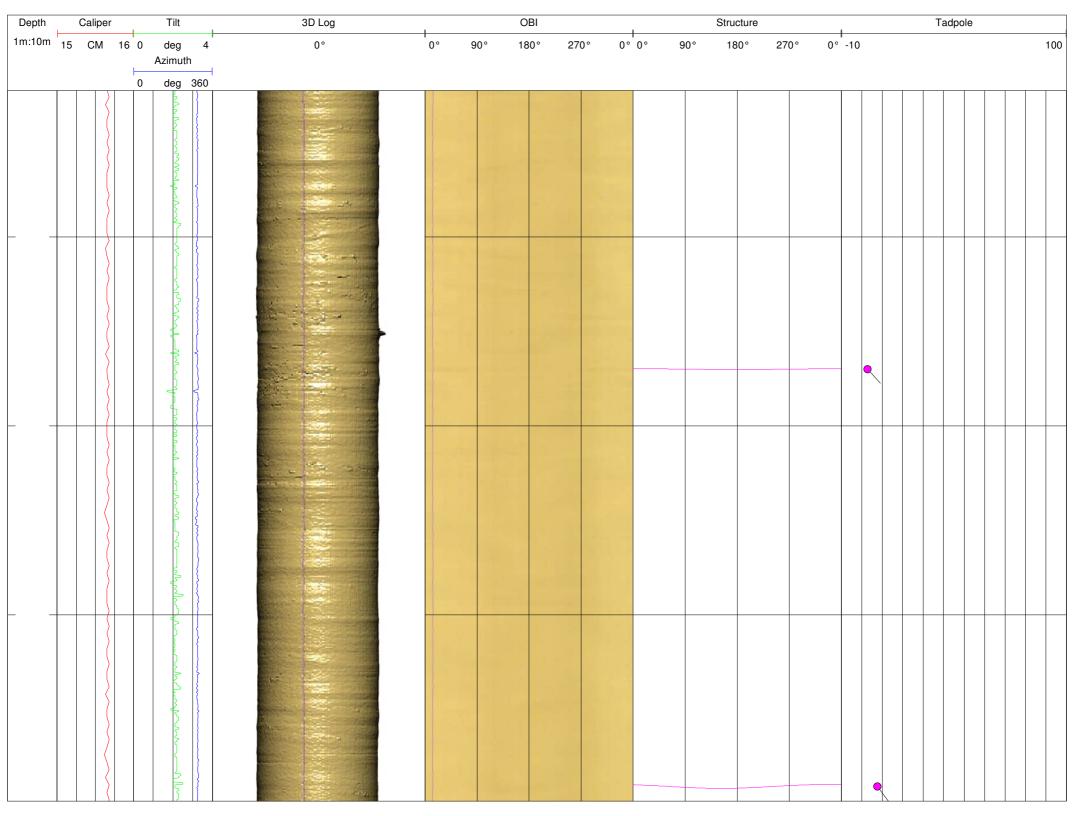


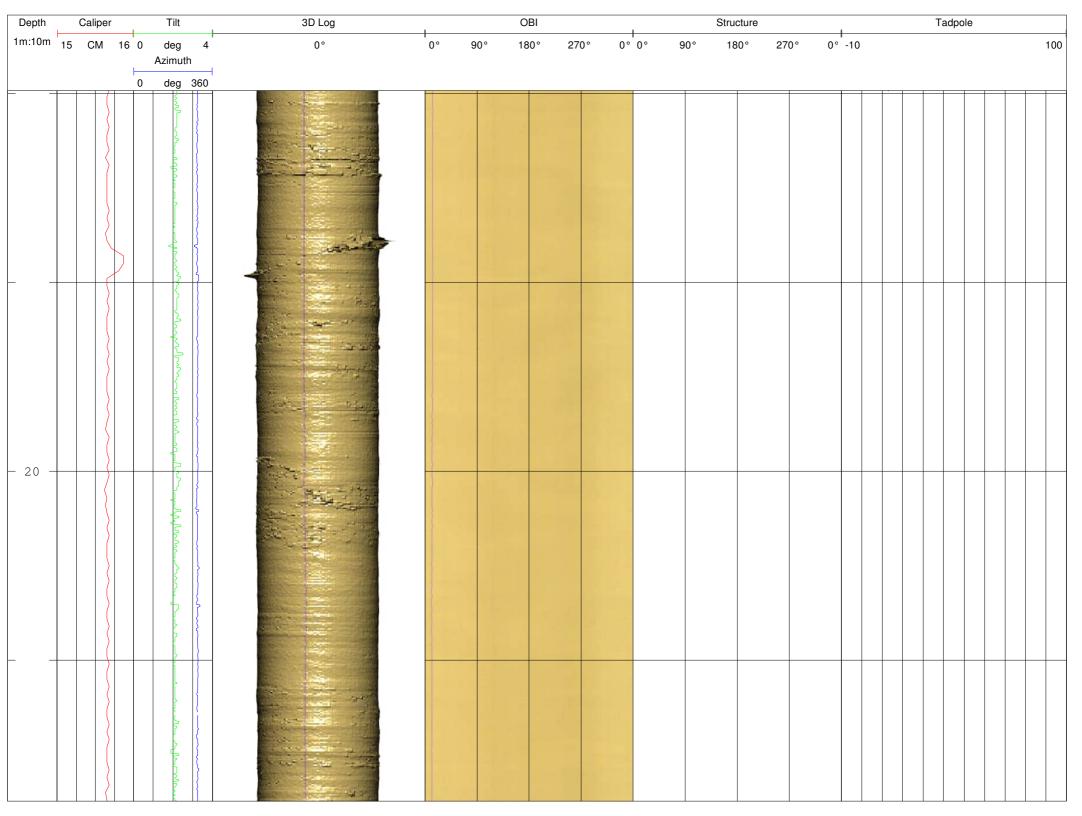


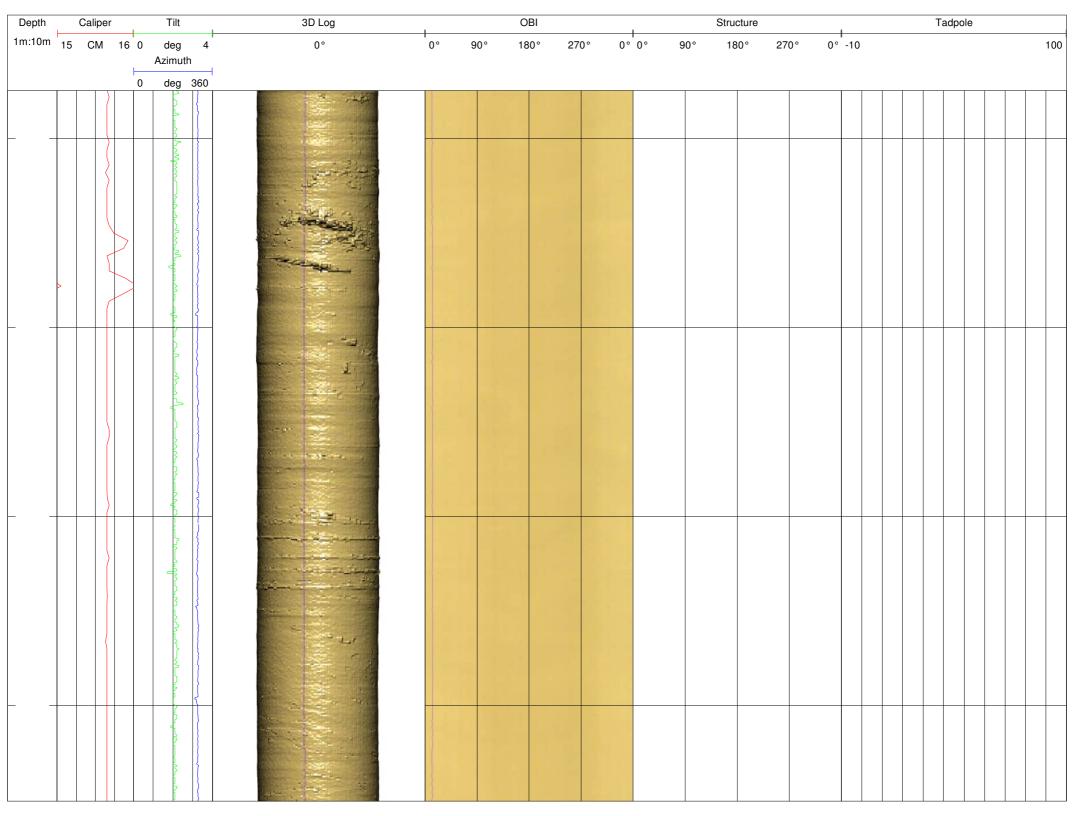


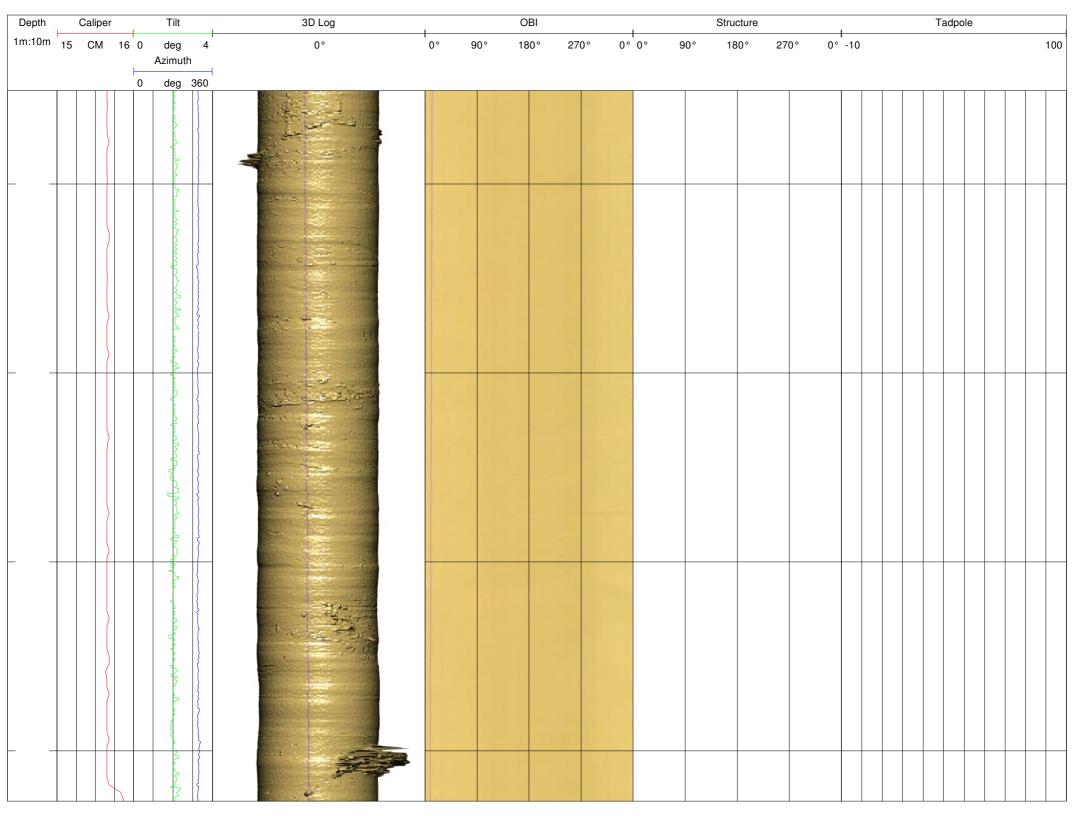


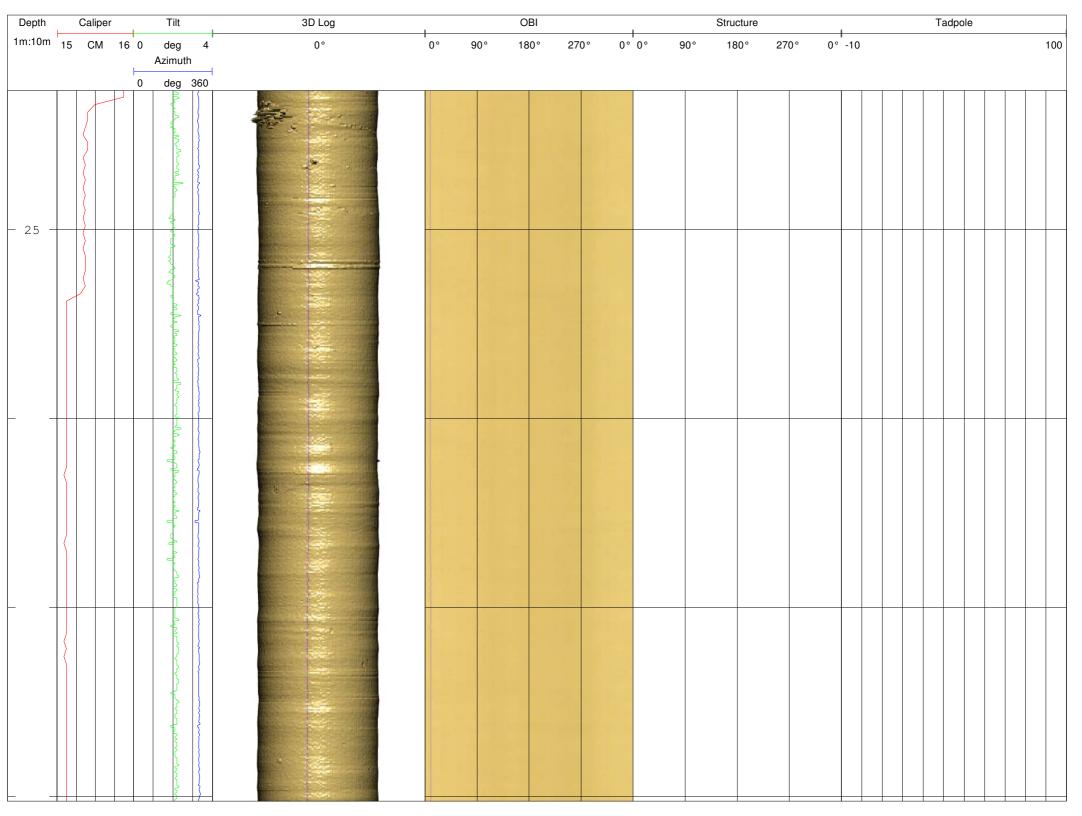


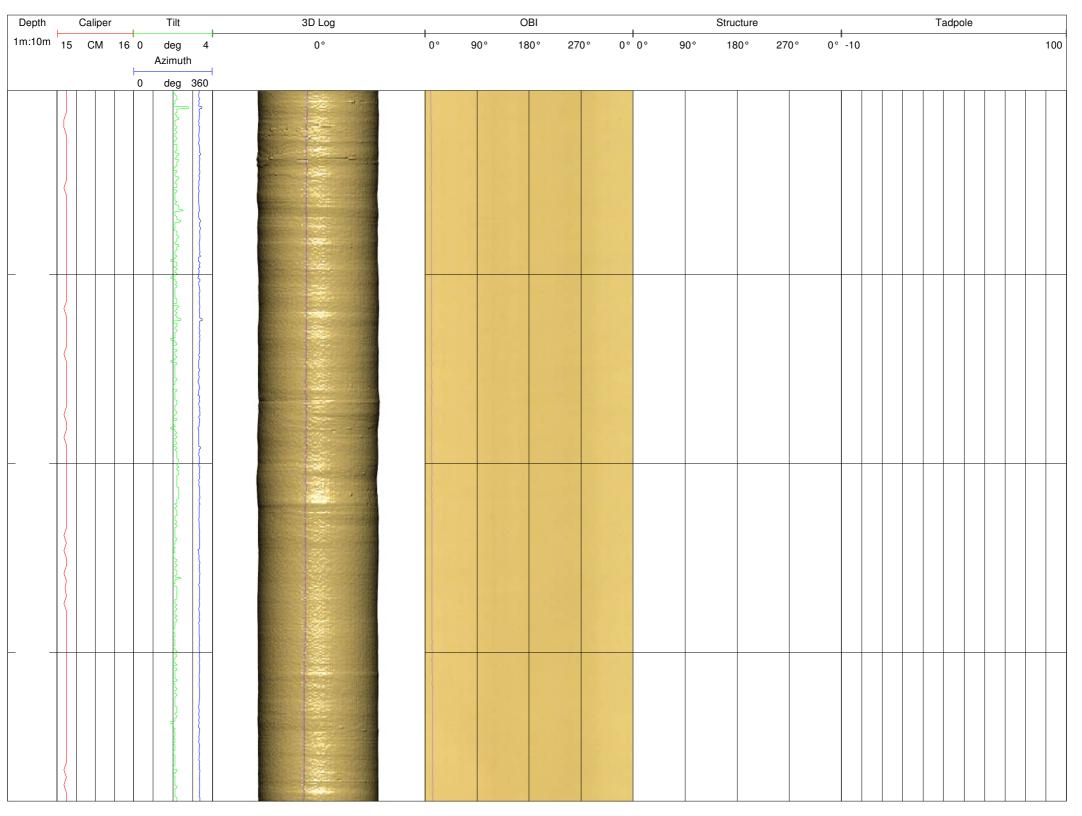


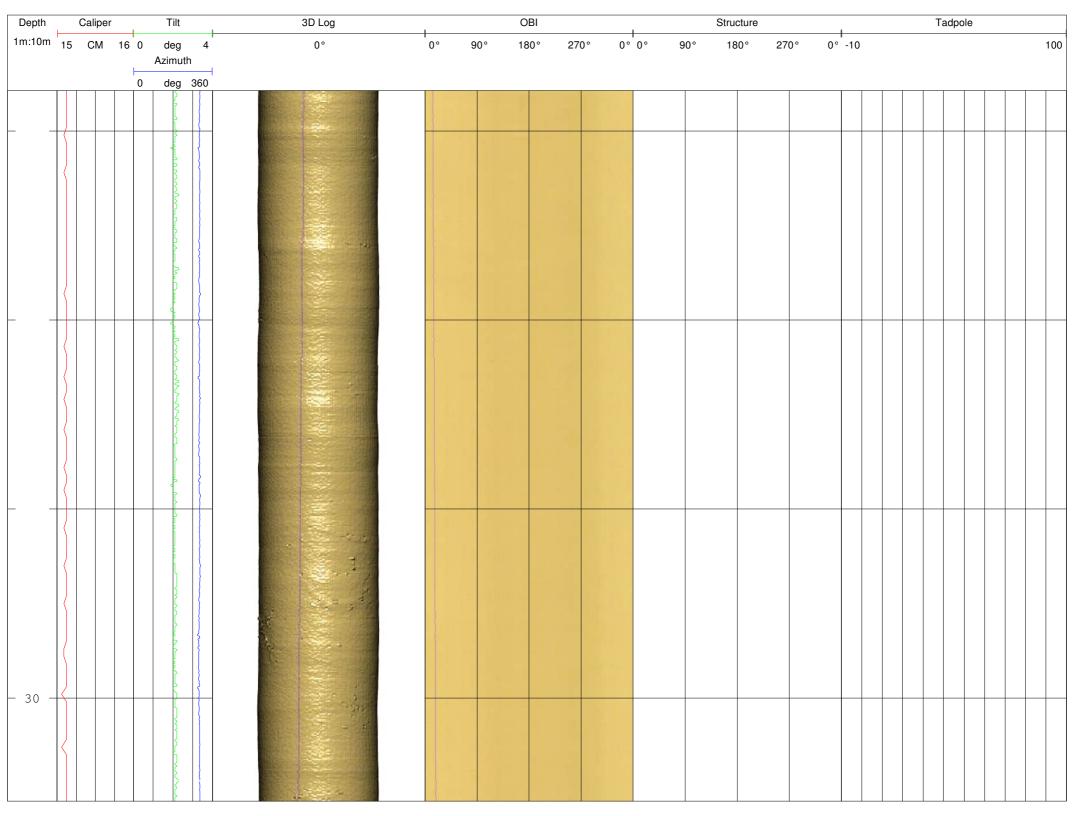


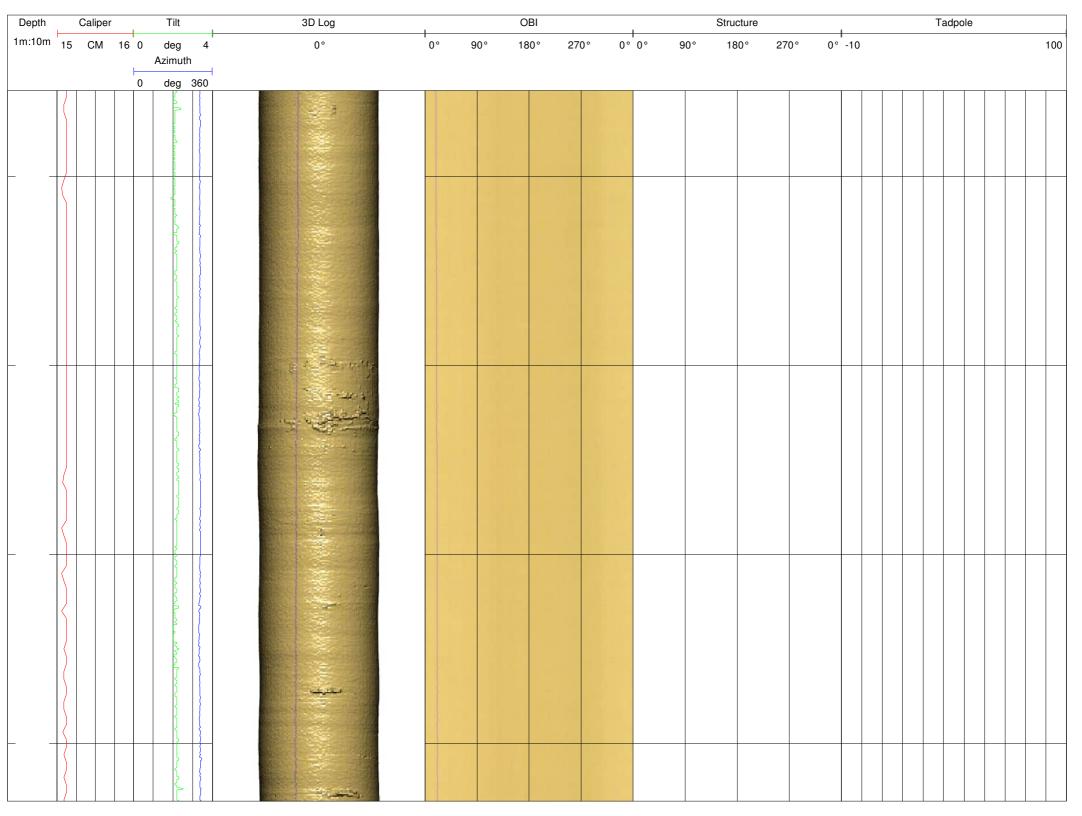


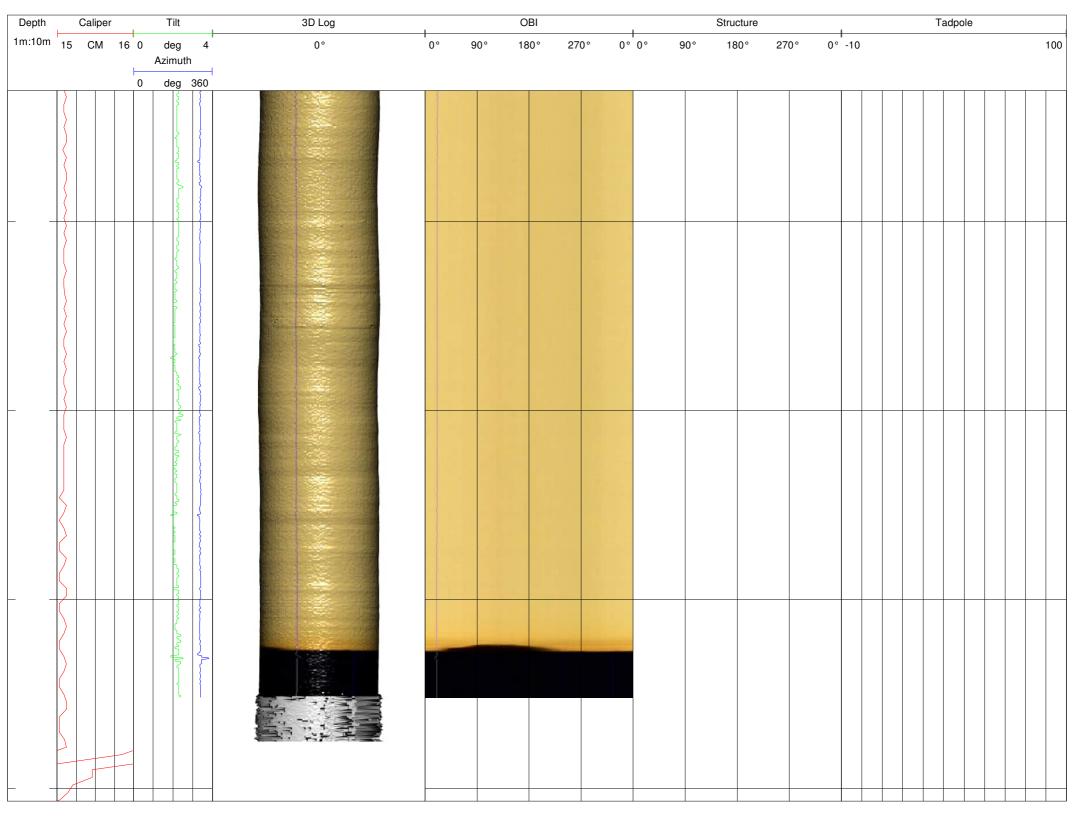






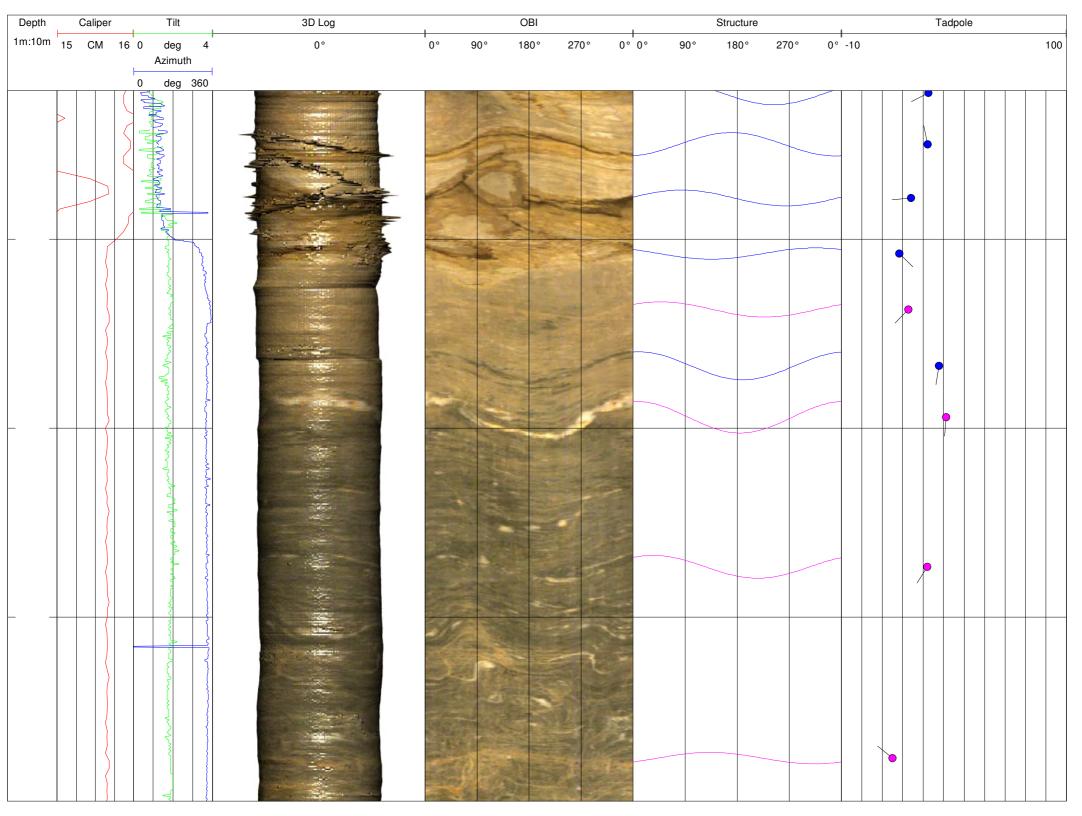


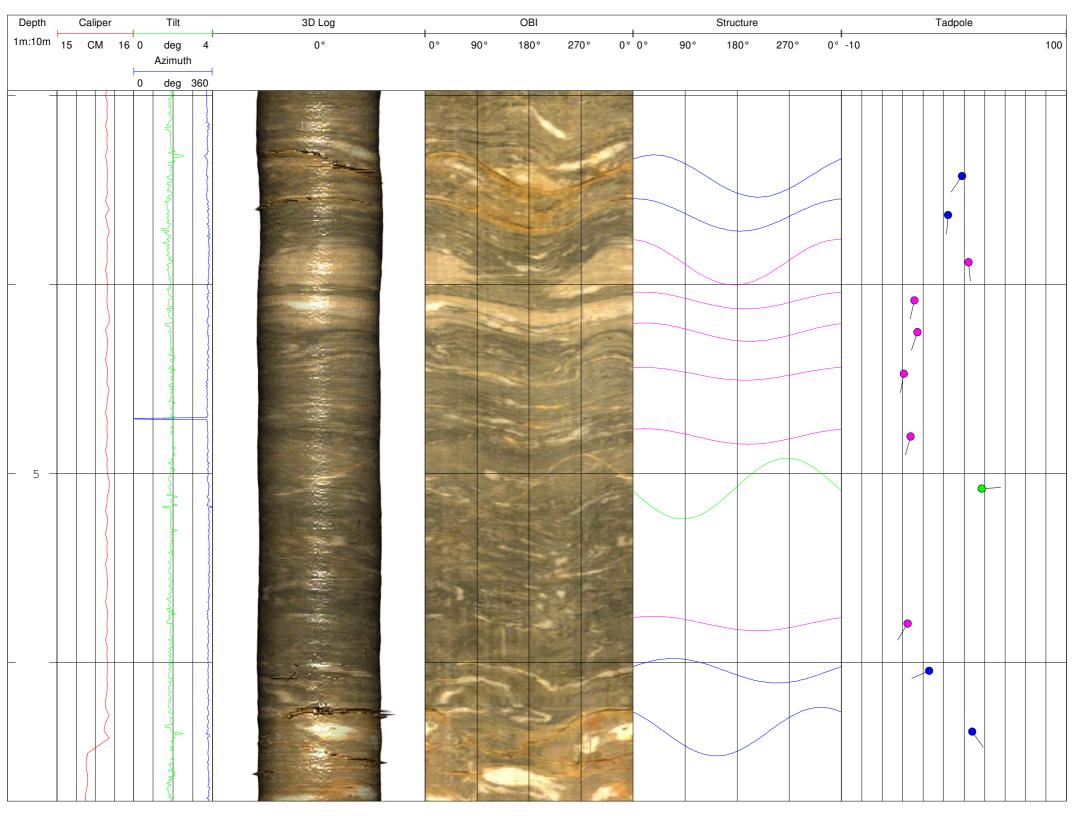


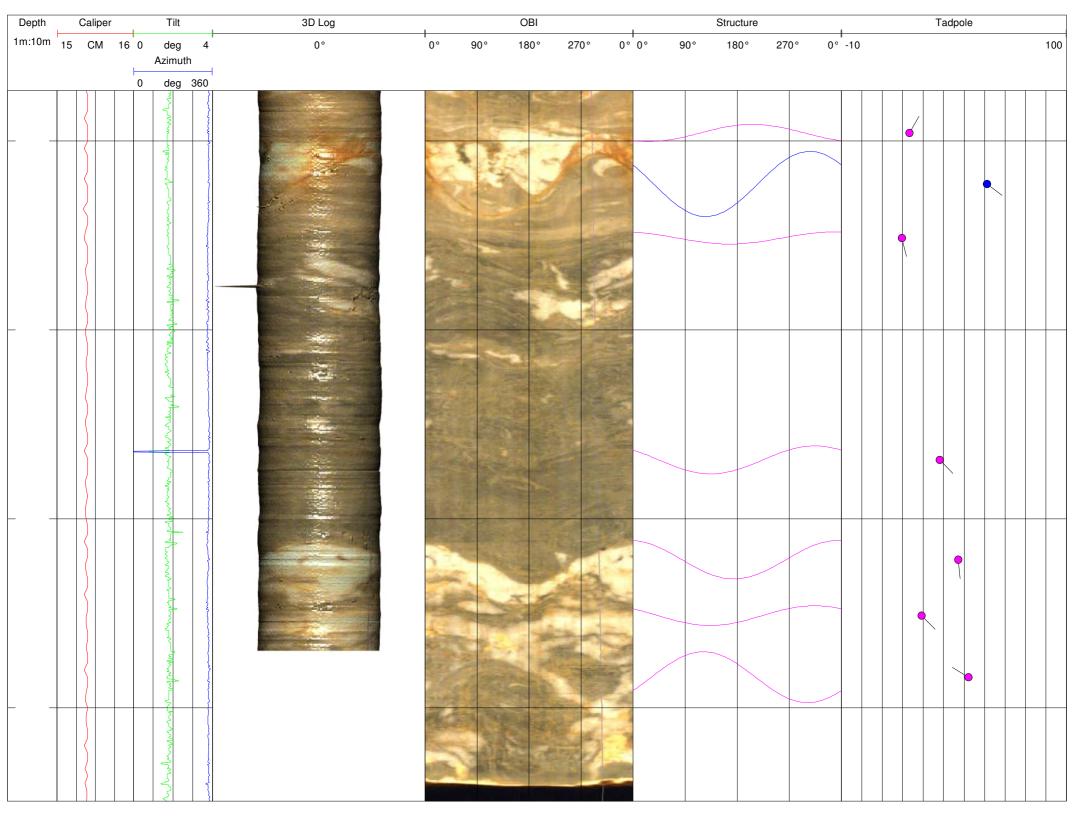


Depth	Caliper	Tilt	3D Log			DBI			Struc	oture			Ta	dpole	
1m:10m	15 CM 16	0 deg 4	0°	0°	90° 18	80° 27	0° 0°	0° 90	0° 18	0° 270	° 0°	-10			100
		Azimuth 0 deg 360													

	Fugro Engi	neering Services									
			Log Type:								
	Client: Scottish and Southern Energy PLC		— Optical Televiewer Log								
	Borehole:	ВН7									
Project: CO	N103001 Sloy Power Station		Approved:								
Location: Sloy	Grid Ret	ference: Elevation:									
Drilled Depth:	8.0m	Date: 04/03/2010									
Logged Depth:	7.88m	Recorded By:	_								
Logging Datum:	Ground Level		Remarks:								
Logged Interval:			North reference is magnetic, Tadpo	ele log and tabulated data is correct	ted for borehole deviation						
Fluid Level:											
Structure Key:	Foliation Fracture	Vein									
BOREHOL	E RECORD		CASING RECORD								
Bit Diameter:	From:	То:	Туре	Size	From	То					
150mm	0m	1.7m	Steel	150mm	0m	1.7m					
120mm	1.7m	8.0m									
<u> </u>	Caliper Tilt	3D Log	OBI	Structure		Tadpole					
1m:10m 15	CM 16 0 deg 4	0° 0° 90°	180° 270° 0° 0°	90° 180°	270° 0° -10	100					
	Azimuth										
	0 deg 360										
	*Allender of the state of the s										
	Jahren James Comment of the Comment										
						,					

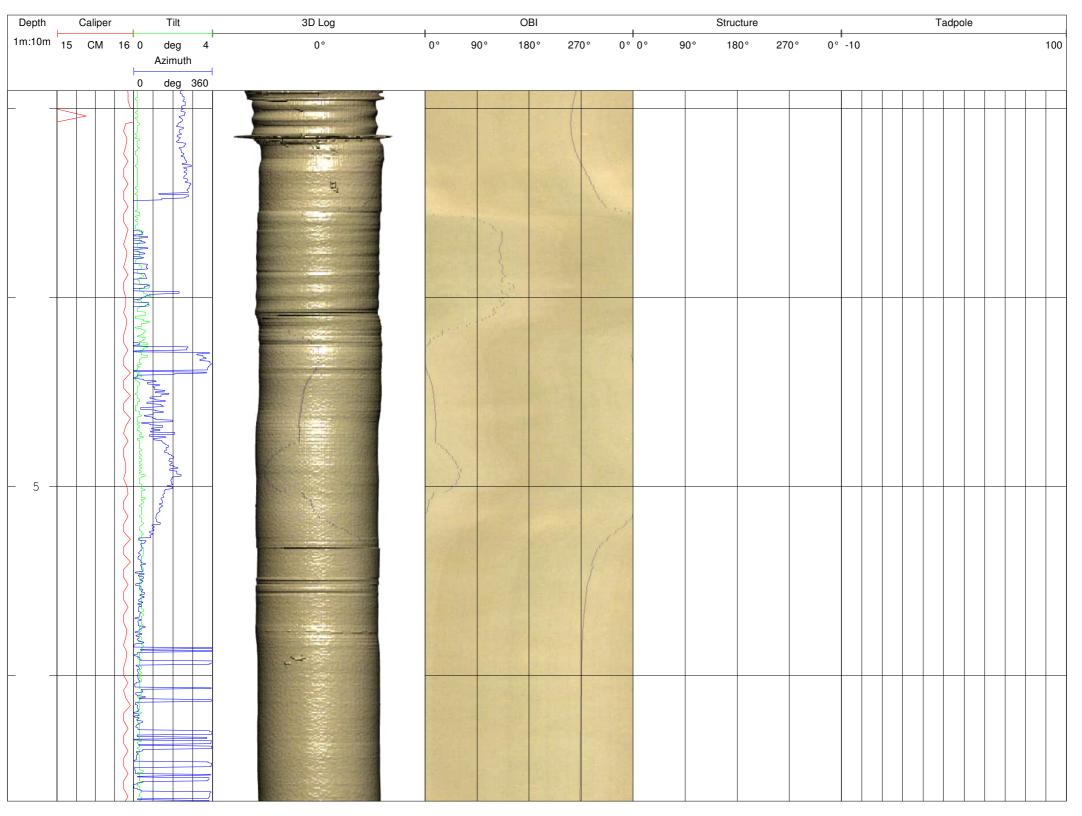


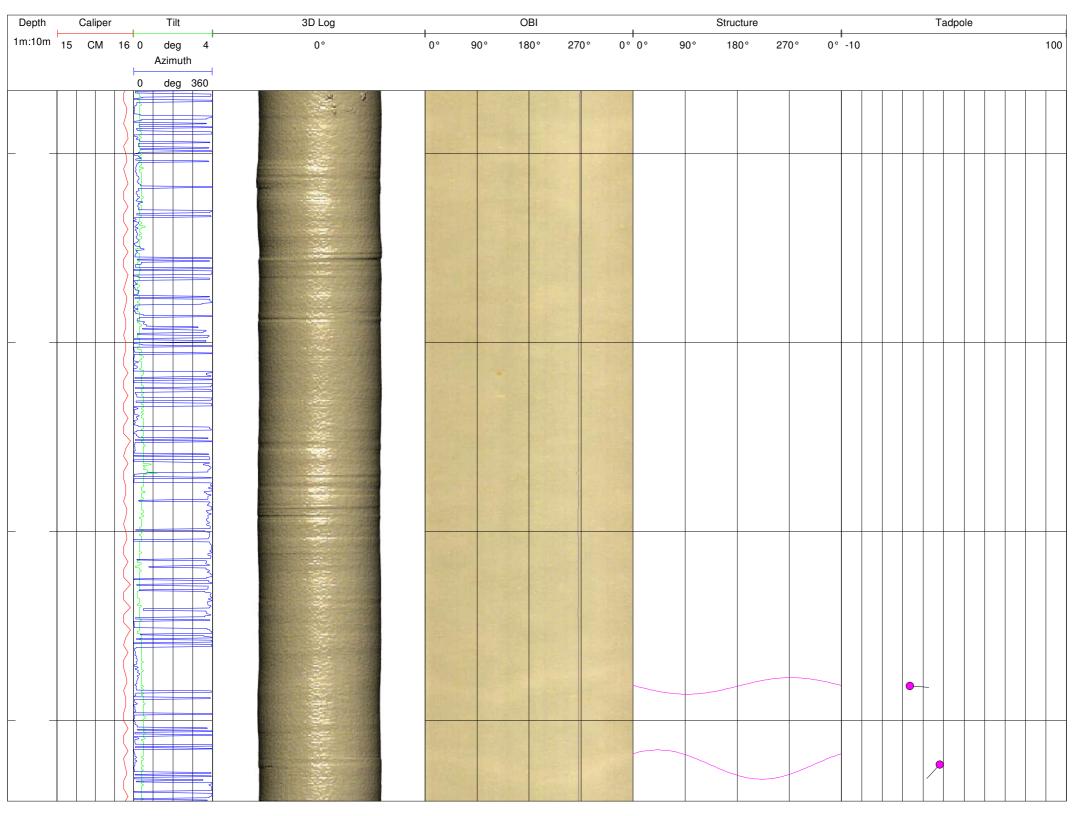


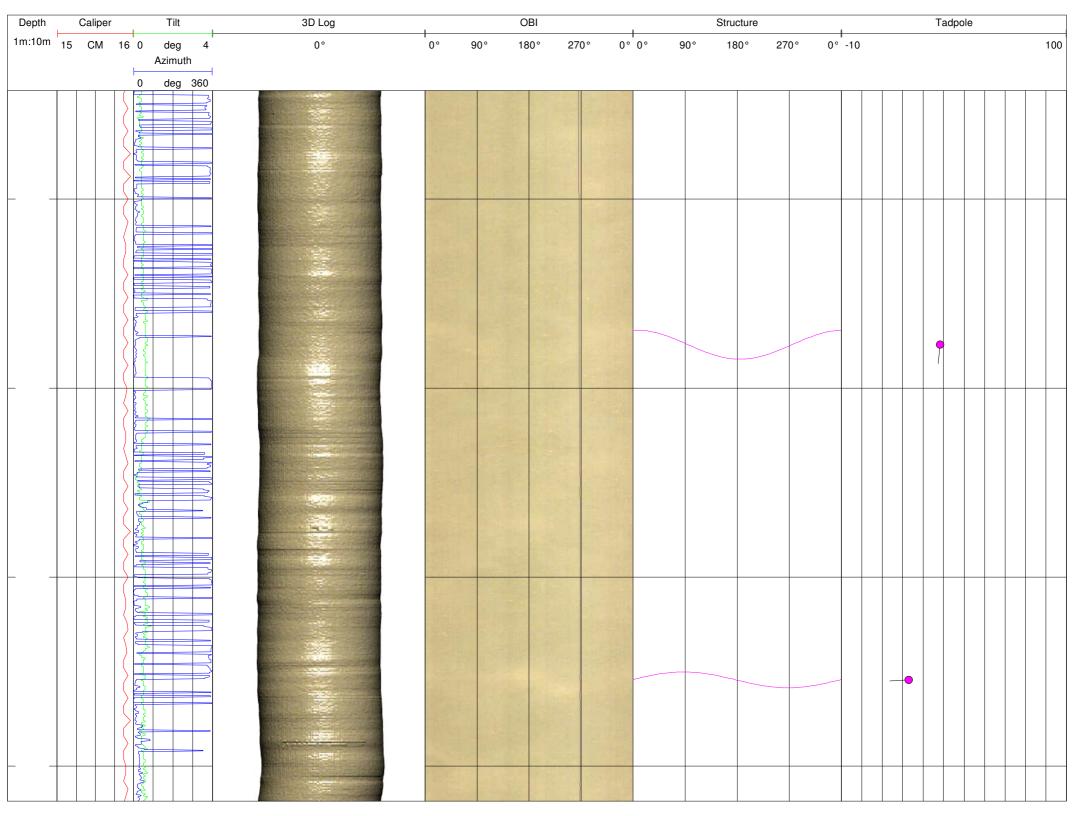


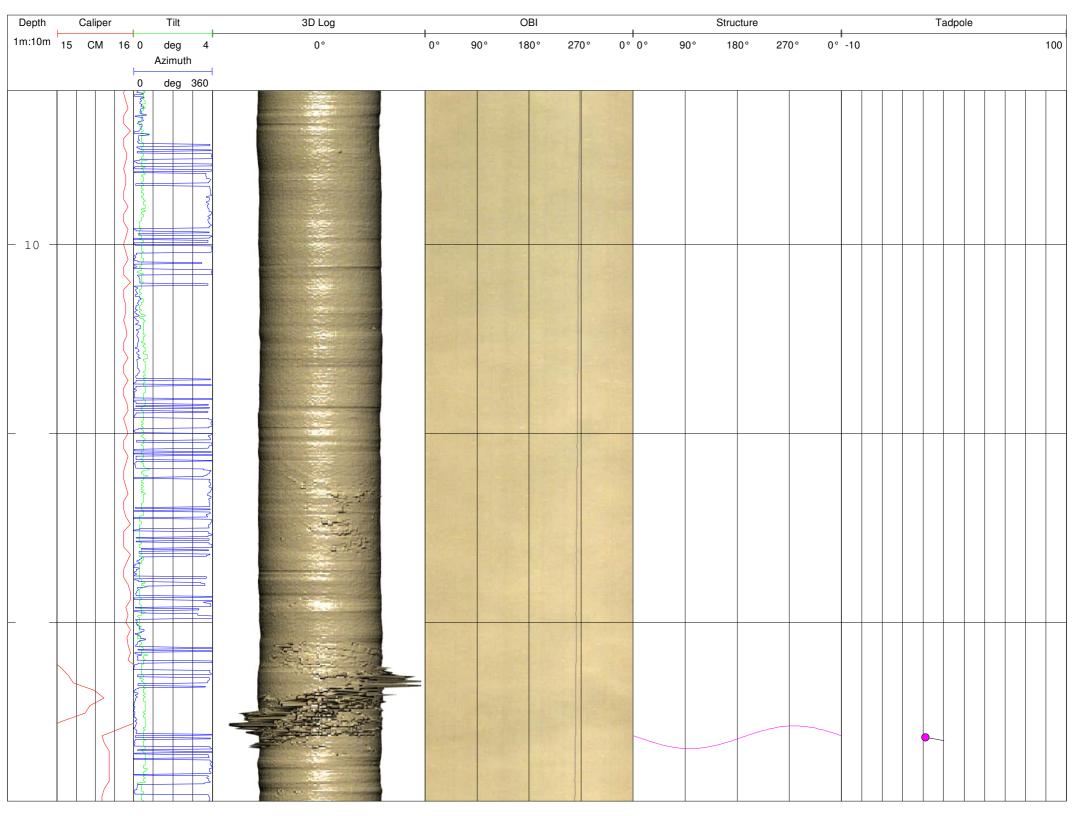
Depth		Calipe	r		Tilt		3D Log		<u> </u>		OBI				Structure				Tad	dpole	
1m:10m	15	СМ	16	6 0	deg	4	0°	1	0°	90°	180°	270°	0° 0°	90°	180°	270°	0°	-10			100
				-	Azimu		4														
				0	deg	360	1														
						}															

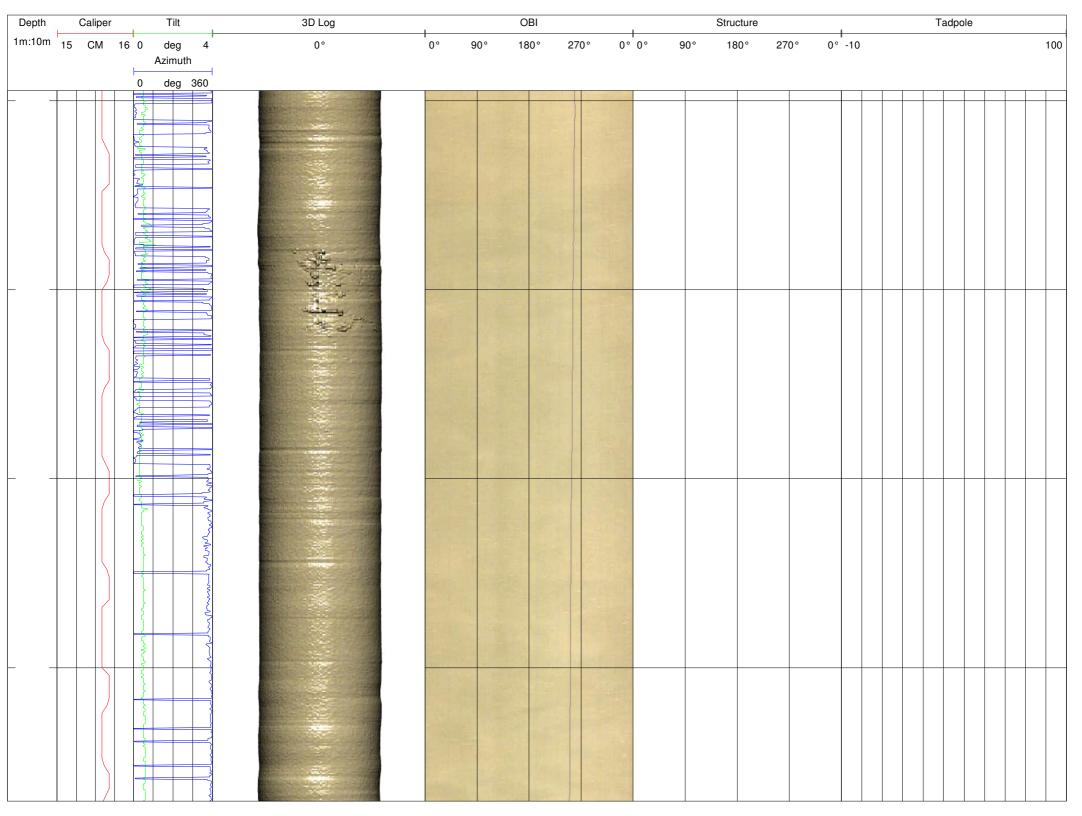
R	Fugro Engi	neering Service	S											
Client:		ient: Scottish and Southern Energy PLC				Log Type:								
$\stackrel{\sim}{\sim}$	Borehole:	BH12		Optical Televiewer L	og									
103001 Sloy	Power Station			Approved:										
wer Station	Grid Ref	ference:	Elevation:											
35m		Date:	05/03/2010											
14.95m		Recorded E	Ву:											
Ground Level				Remarks:										
Logged Interval:						le log and tabulat	ed data is correc	ted for borehole de	eviation					
					e to poor	visibility as a res	ult of the water co	ndition						
Foliation	racture —	Vein												
RECORD				CASING RECO	RD									
From:		То:	Туре		Size		From		То					
)m		4.1m	Steel 150mm			0m		4.1m						
4.1m		35.0m												
aliper	Tilt	3D Log		OBI			Structure			Tadpo	ole			
CM 16 0	deg 4	0°	"0° 90°	180° 270°	0° ['] 0°	90°	180°	270° 0°	· -10			100		
-														
	John Mary Mary Mary Mary Mary Mary Mary Mary													
11 A	03001 Sloy er Station 35m 14.95m Ground Level Foliation RECORD rom: m .1m liper CM 16 0	Client: Borehole: 03001 Sloy Power Station Per Station Grid Rei 35m 14.95m Ground Level Foliation Fracture RECORD rom: m .1m liper Tilt CM 16 0 deg 4 Azimuth 0 deg 360	Client: Scottish and Sout Borehole: BH12 03001 Sloy Power Station er Station Grid Reference: 35m Date: 14.95m Recorded It Ground Level Foliation Fracture Vein RECORD rom: To: m 4.1m 35.0m liper Tilt 3D Log M 16 0 deg 4 Azimuth O deg 360	Client: Scottish and Southern Energy PLC Borehole: BH12 03001 Sloy Power Station er Station Grid Reference: Elevation: 35m Date: 05/03/2010 14.95m Recorded By: Foliation Fracture Vein RECORD rom: To: m 4.1m 35.0m liper Tilt 3D Log M 16 0 deg 4 Azimuth 0 deg 360	Client: Scottish and Southern Energy PLC Optical Televiewer L Borehole: BH12 Optical Televiewer L Optical Teleview	Client: Scottish and Southern Energy PLC Borehole: BH12 Optical Televiewer Log Optical Televitor Optical Televiewer Log Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Televitor Optical Te	Client: Scottish and Southern Energy PLC Borehole: BH12 O3001 Sloy Power Station	Client: Scottish and Southern Energy PLC Borehole: BH12 O3001 Sloy Power Station	Client: Scottish and Southern Energy PLC Borehole: BH12 03001 Sloy Power Station er Station Grid Reference: Elevation: Stem Date: 05/03/2010 Recorded By: Faccured By: Faccured Station Fracture Vein RECORD CASING RECORD Too: Type Size From A 1 In Steel 150 mm 0 m Azimuth O deg 360	Client: Scottish and Southern Energy PLC Discretoile: BH12 O3001 Sloy Power Station er Station Gird Reference: Elevation: Strim Date: 0509/2010 Recorded By: Ground Level Remarks: North reference is reagnetet, Tacpole log and tabulated data is corrected for borehole deviation Test stopped at 14 9m due to poor visibility as a result of the water condition Foliation Fracture Vein CASING RECORD To: Type Size From In 4.1 in Steel 150mm Im Imper Tilt 30 Log OBI Structure Azimuth O deg 4 O* O* 90* 180° 270* O* 0* 90* 180° 270* O* -10 Azimuth O deg 380	Client: Scottah and Southern Energy PLC Borehole: BH12 O3001 Slay Power Station of Station Grid Reference: Elevation: Stem Dete: 05032010 Recorded By: Formarks: Noth reference is magnetic. Taiglois log and tabulated data is carrected for borehald deviation Tool stopped at 14.0m due to poor visibility as a result of the water condition FERCORD CASING RECORD Too: Type Size From To To Steel 150mm 0m 4.1m Int 35.0m Talpin To deg 380	Clean: Scotisish and Southern Energy PLC Optical Toloviewor Log Sortinole: BH12 Approved: Coptical Toloviewor Log App		

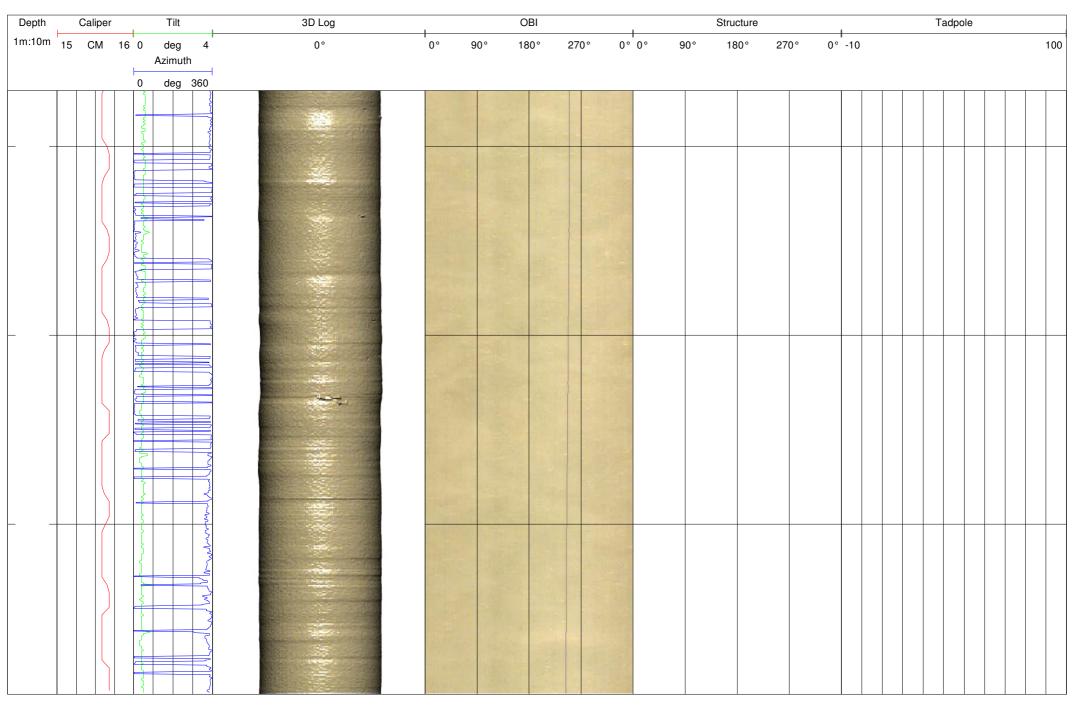




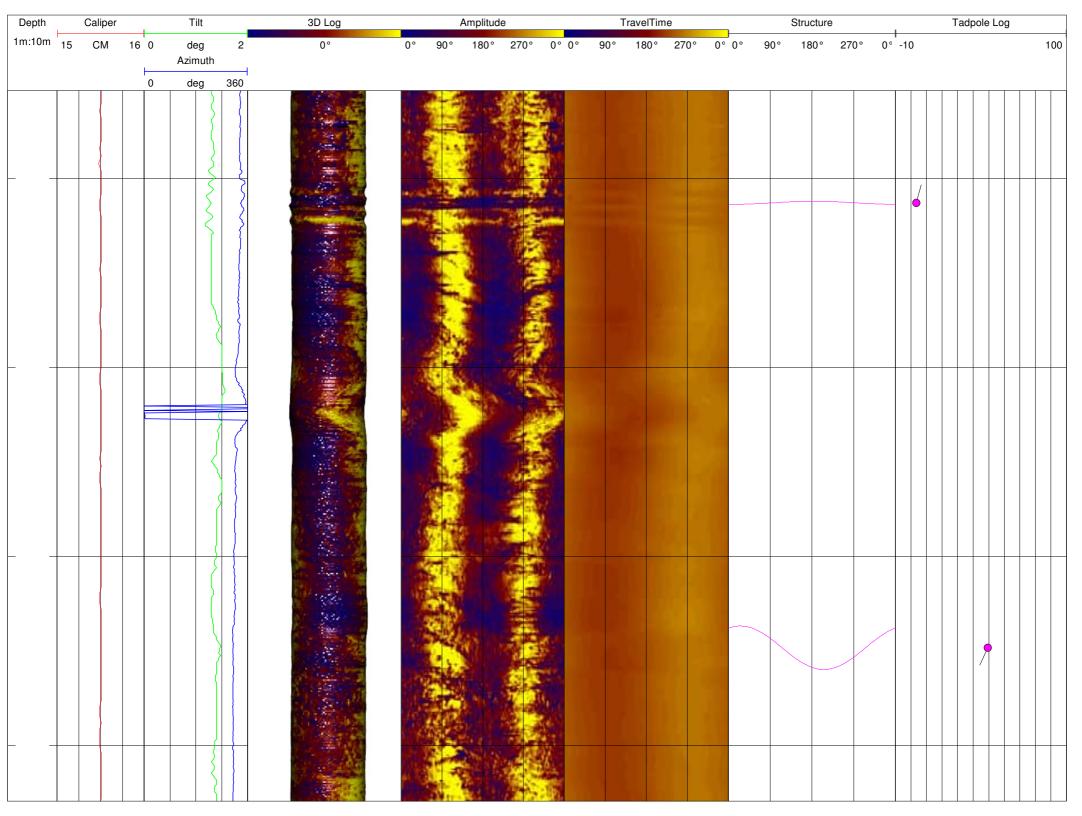


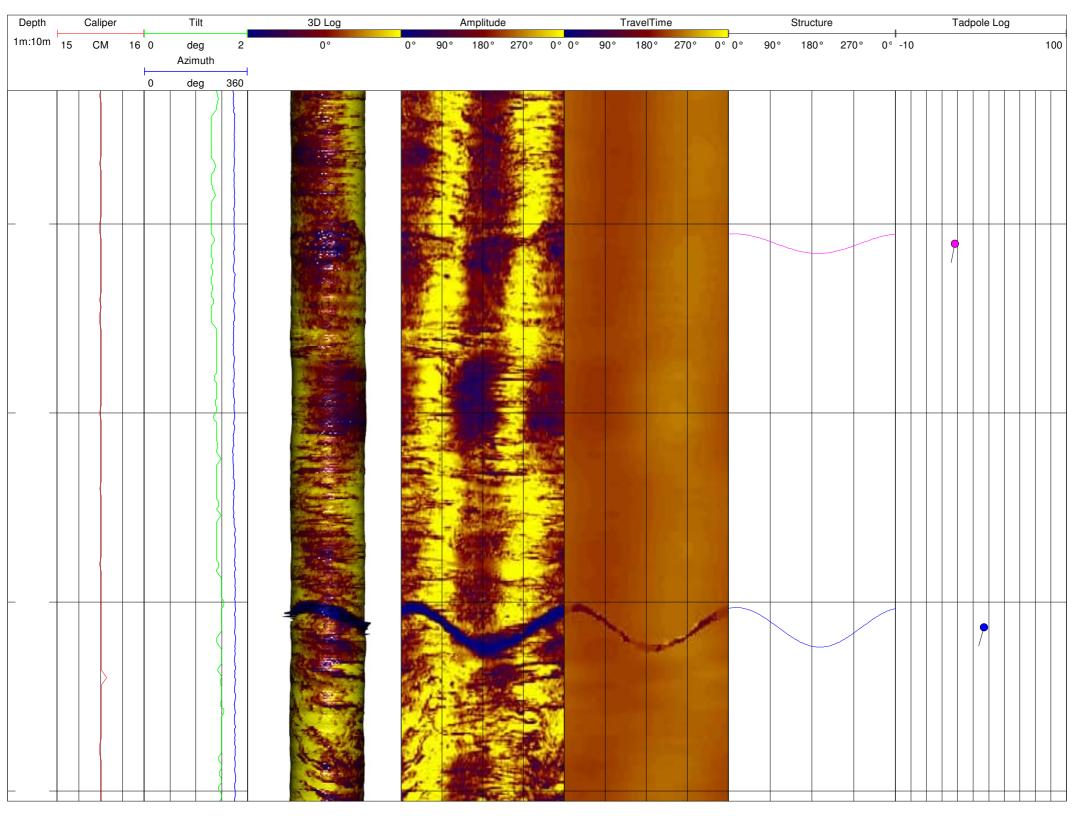


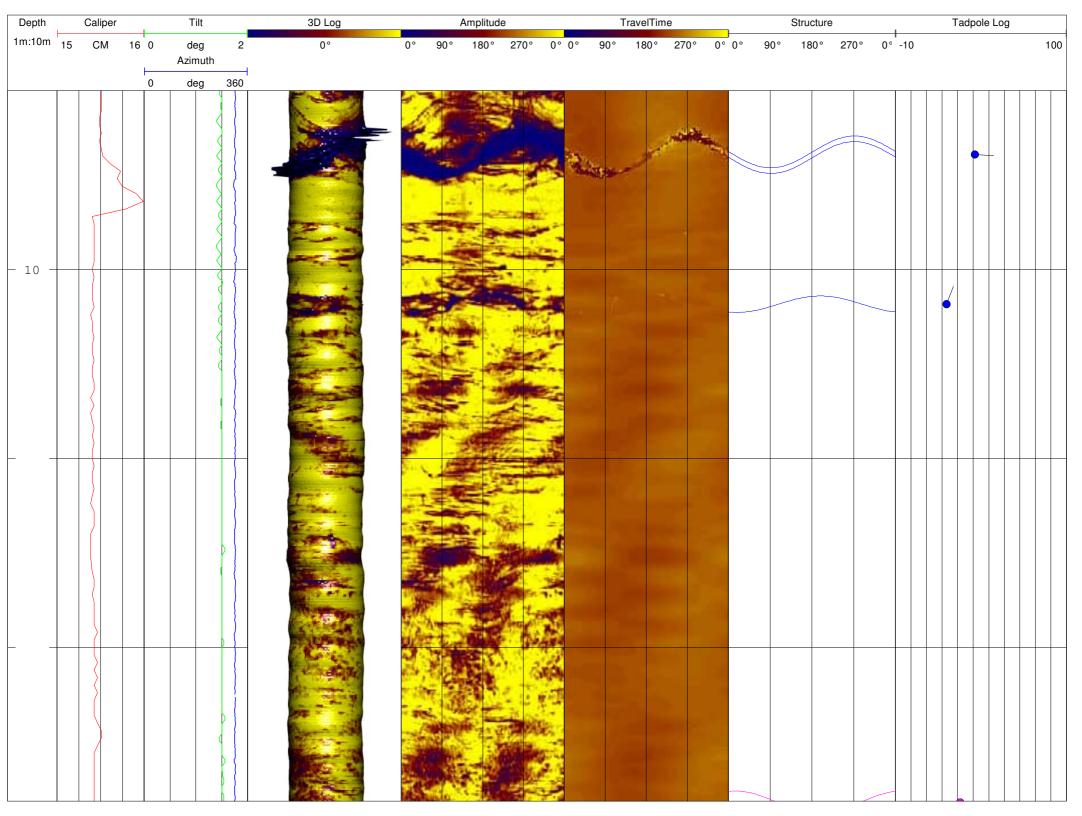


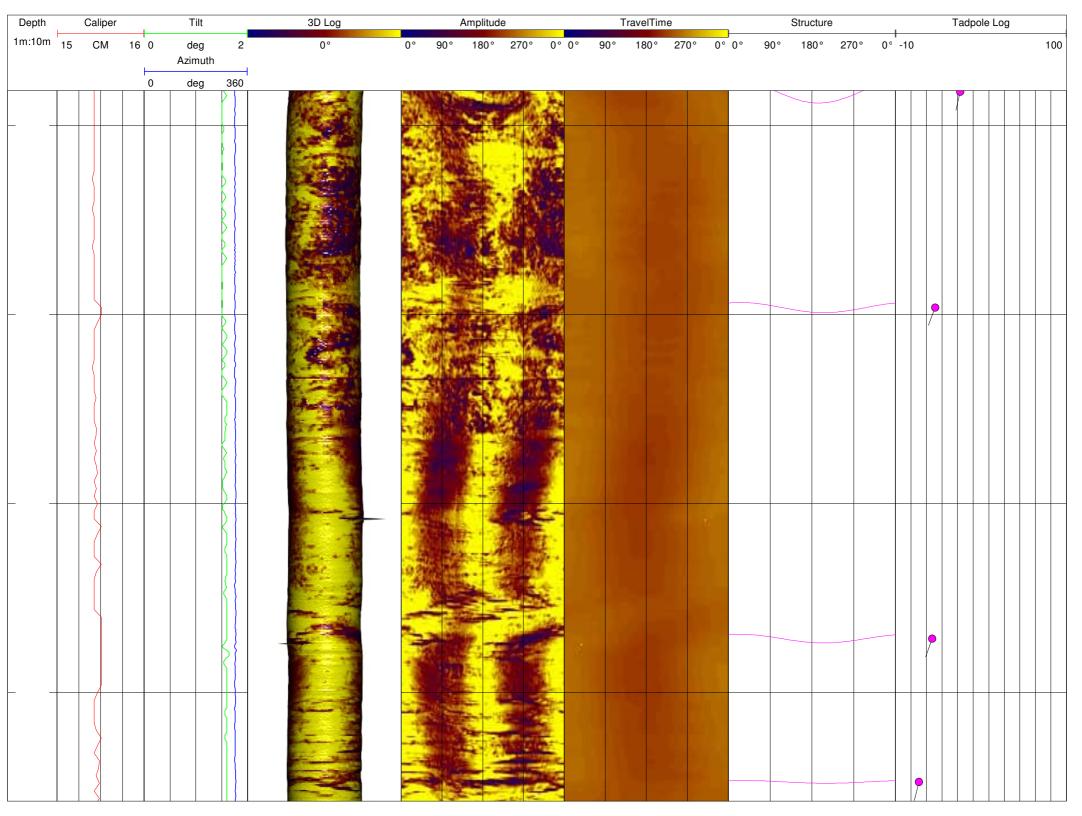


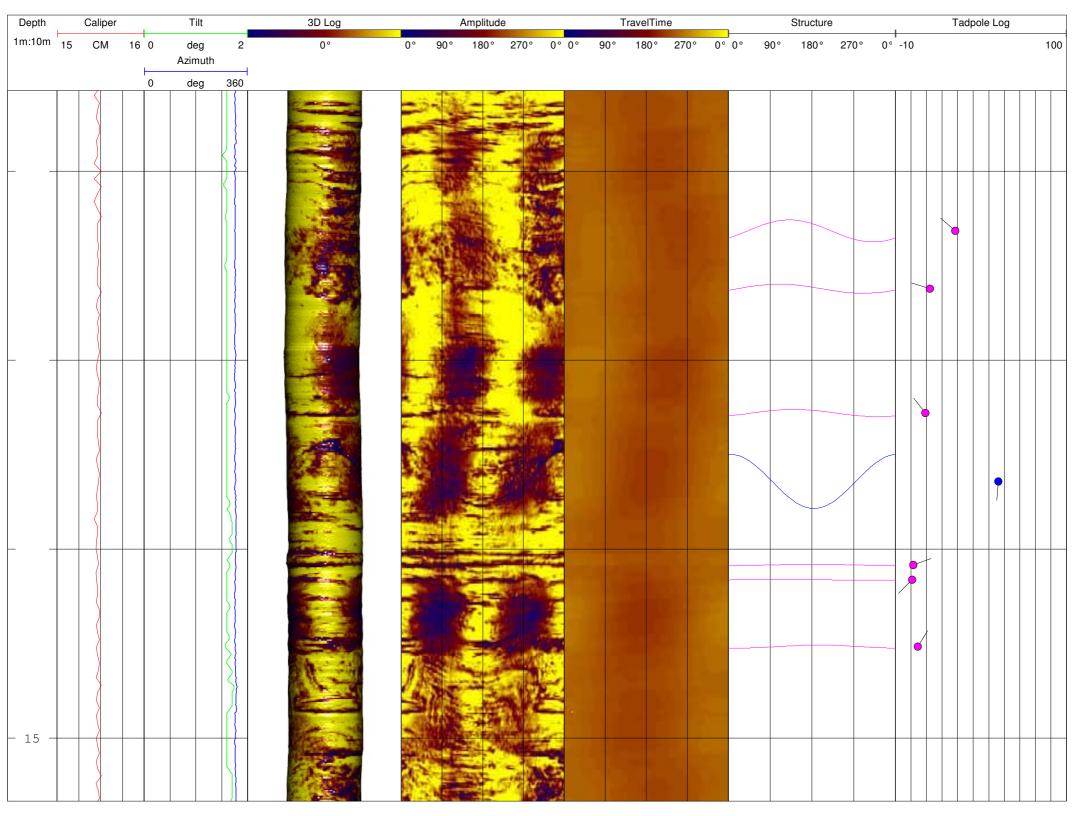
-Fuc	RO	Fugro Er	gineering Servi	ces								
	$= \sim$	Client:	Scottish and	Southern Energy PLC	Log Type:							
	\approx	Borehole:	BH1		Acoustic Televiewer Log							
Project: COI	N103001 Slo	y Power Station			Approved:							
Location: Sloy			Reference:	Elevation:								
Drilled Depth:	20.0m		Date:	04/03/2010								
Logged Depth:	19.40m		Reco	rded By:								
Logging Datum:	Ground Level				Remarks:							
Logged Interval:					North reference is magnetic, Tadpo	le log and tabulated data is correct	ed for borehole deviation					
Fluid Level:												
Structure Key:	Foliati	on — Fracture	Vein									
BOREHOLI	E RECORD				CASING RECORD							
Bit Diameter:	From:		То:		Туре	Size	From	То				
150mm	0m		5.5m		Steel	150mm	0	5.5m				
120mm	5.5m		20.0m									
Depth	Caliper	Tilt	3D Log	Amplitude	Travel	Time	Structure	Tadı	pole Log			
1m:10m 15	CM 16	-	0°	0° 90° 180° 2	70° 0° 0° 90° 180	0° 270° 0° 0° 90	0° 180° 270° 0° -	10	100			
	H	Azimuth 0 deg 360										
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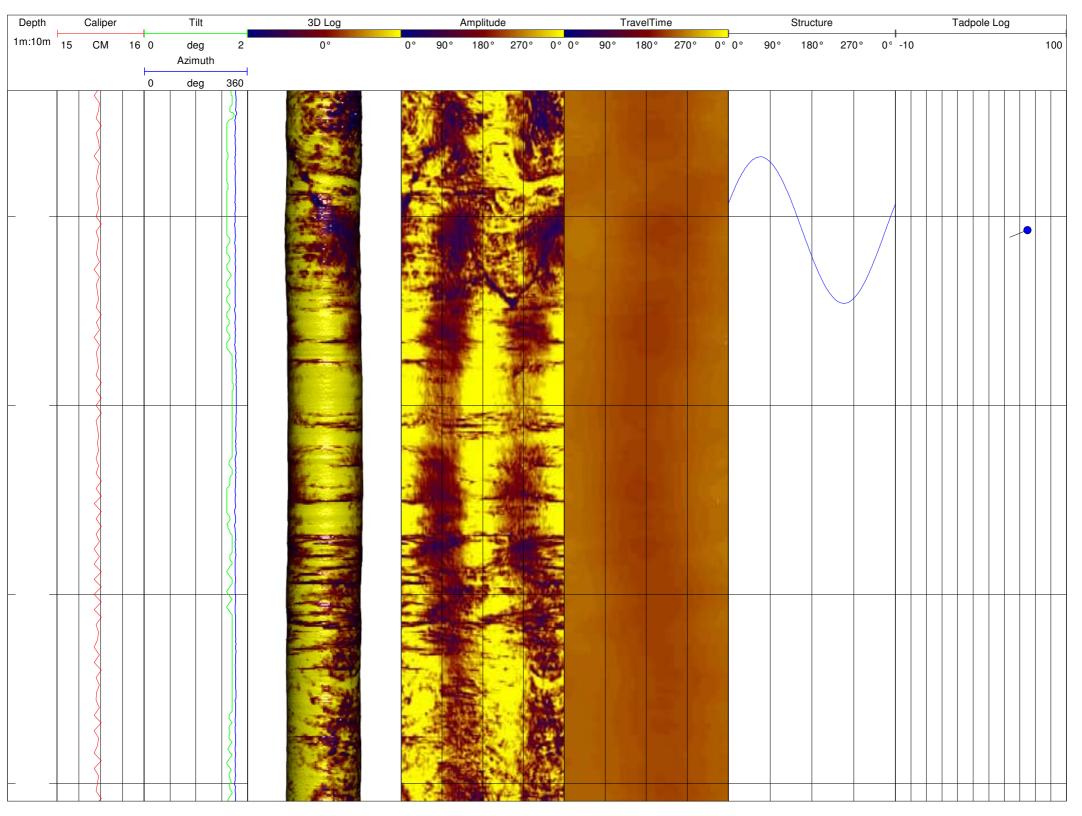


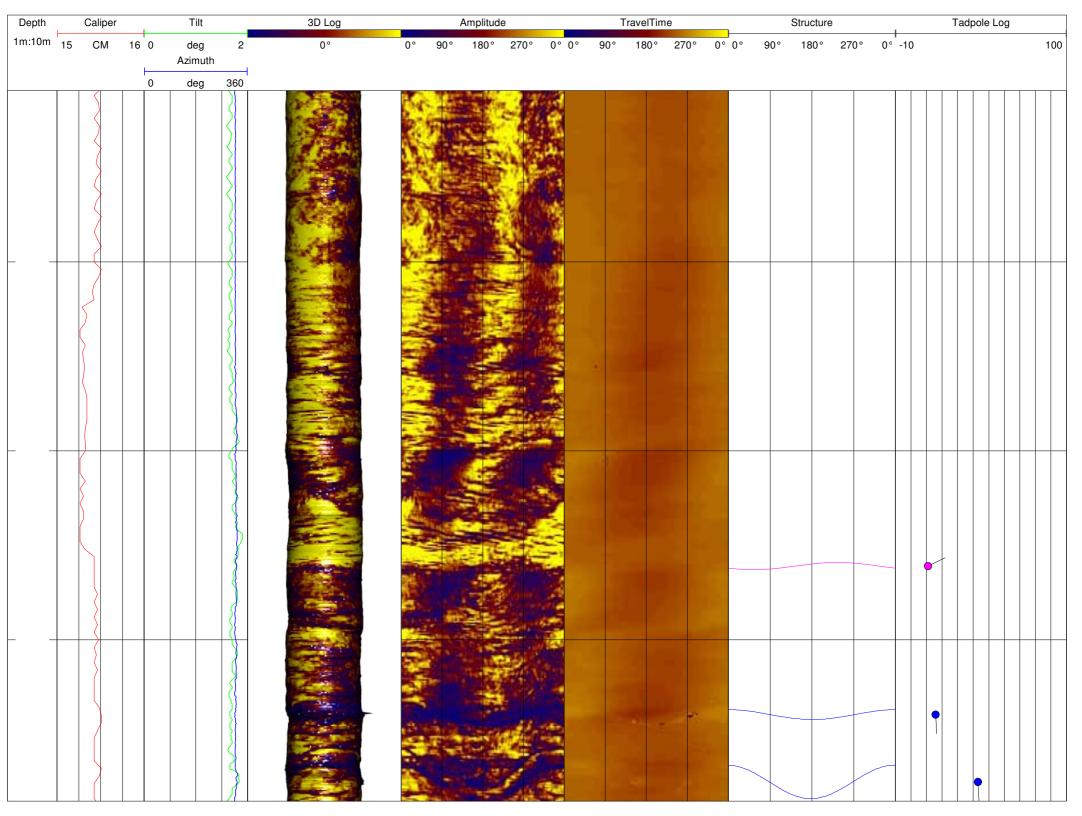


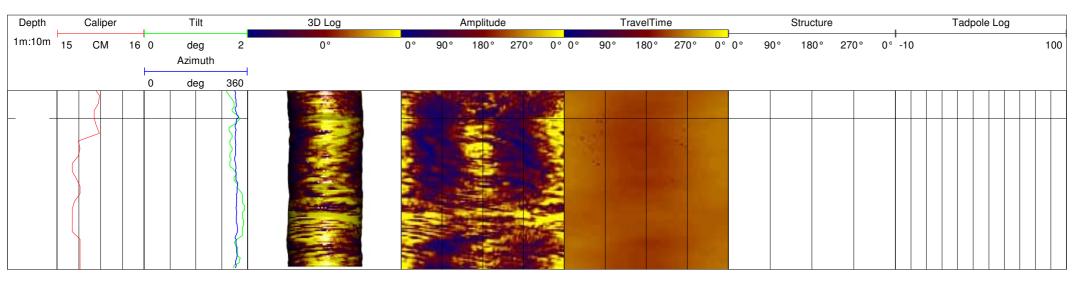




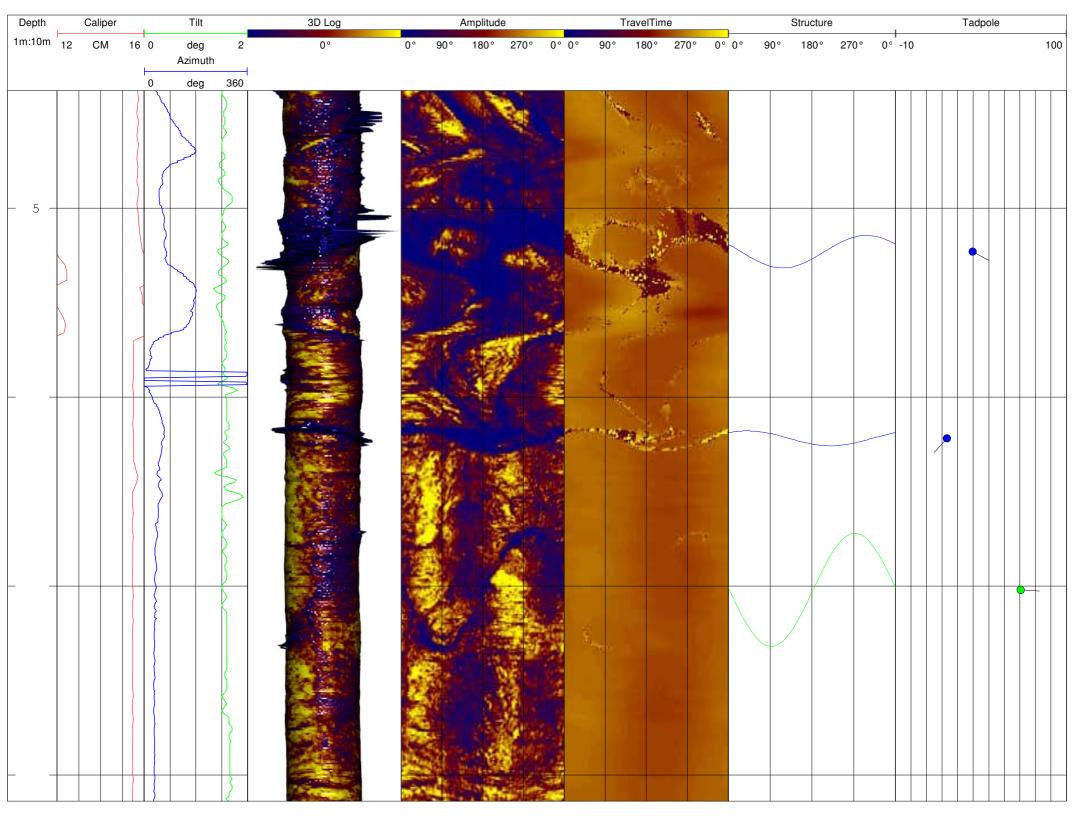


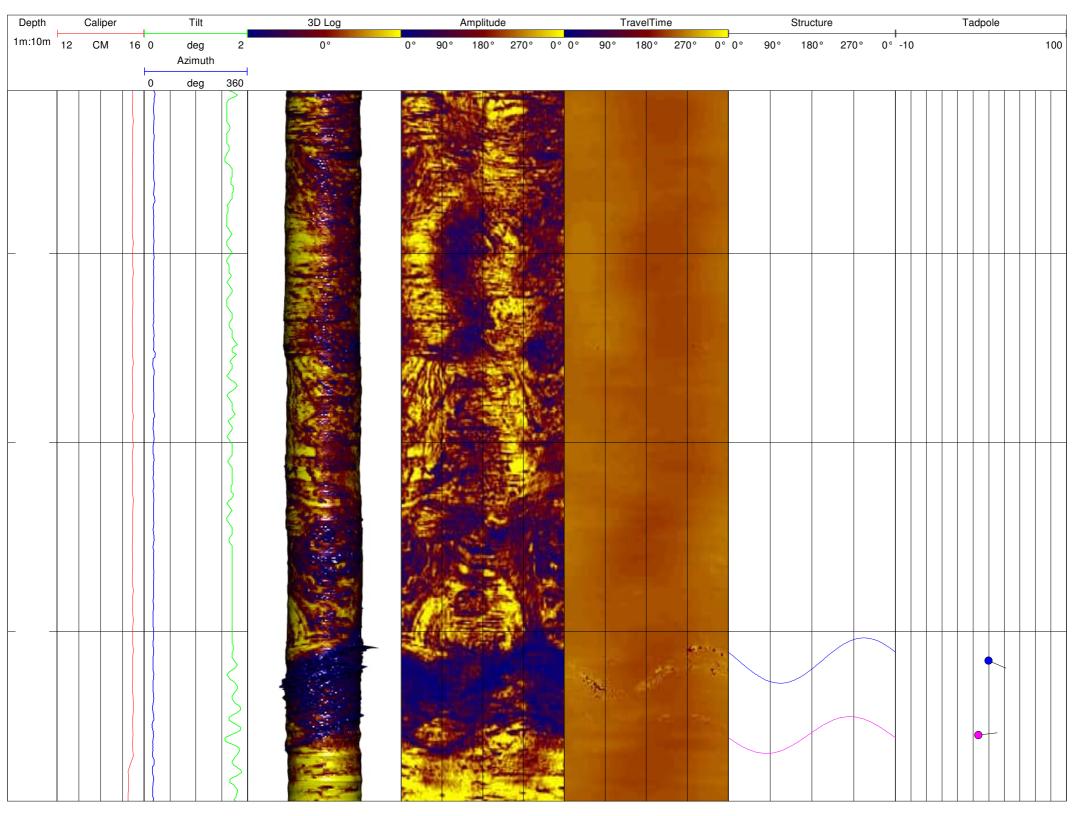


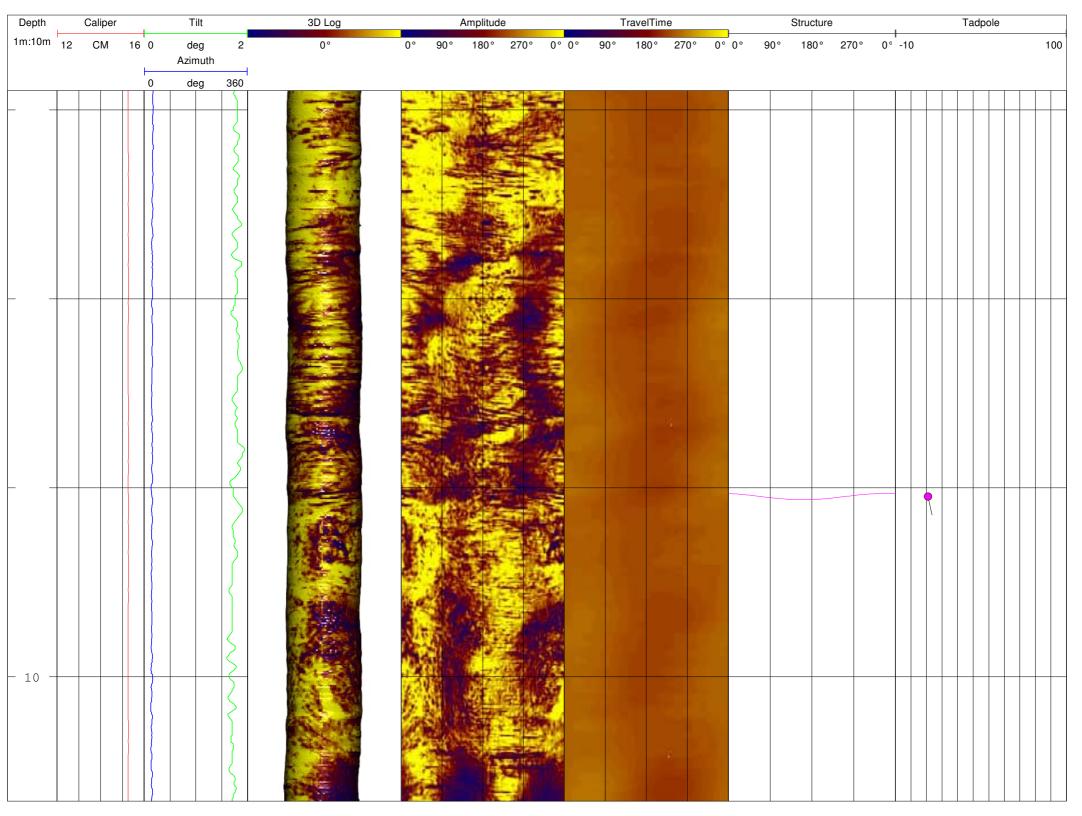


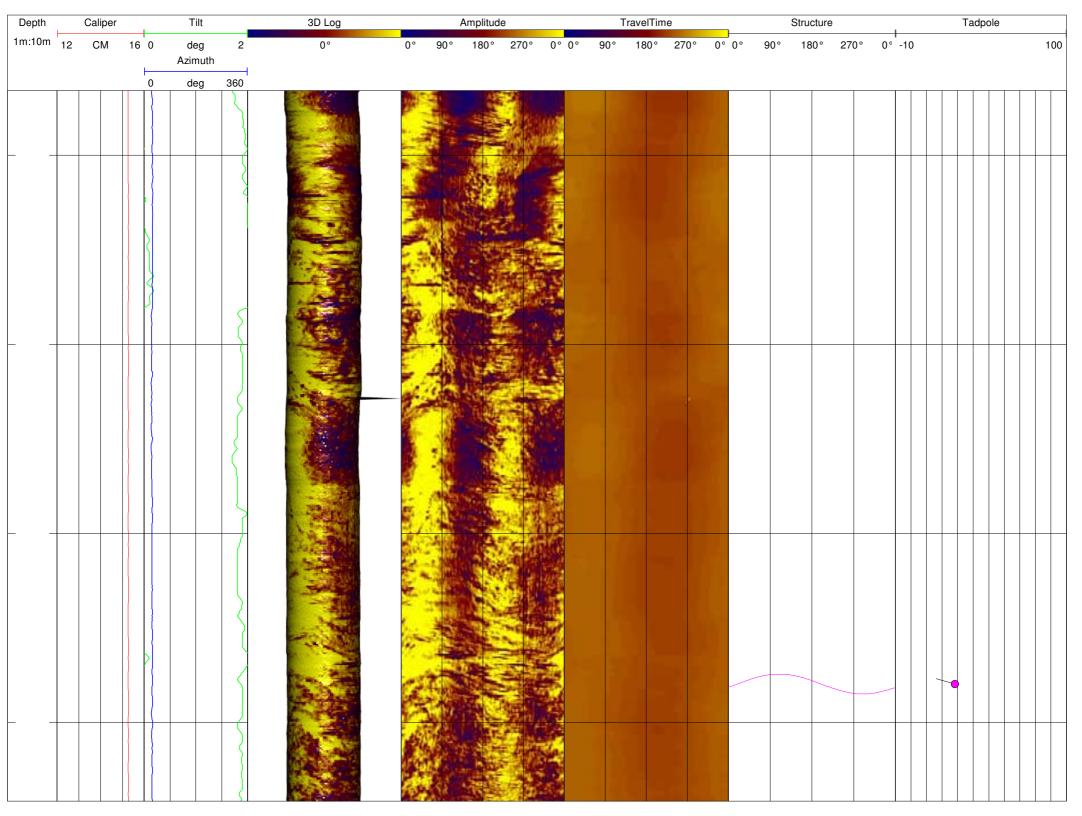


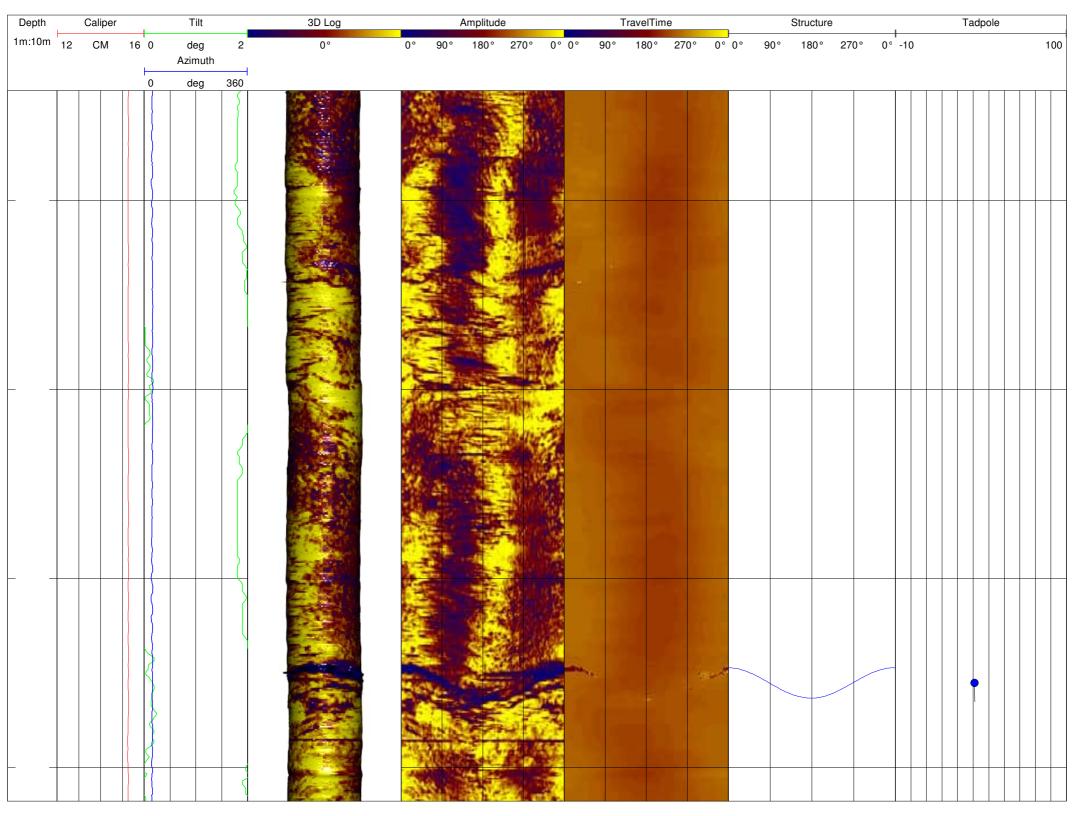
-Fu	RO	Fugro Eng	ineering Servi	ces								
	=	Client:	Scottish and	Southern Energy PLC	Log Type:							
· ·	$\widehat{}$	Borehole:	BH2		- Acoustic Televiewer Log							
Project: COI	N103001 Slo	Power Station			Approved:							
Location: Sloy			eference:	Elevation:								
Drilled Depth:	35.0m		Date:	04/03/2010								
Logged Depth:	34.02m		Record	ded By:								
Logging Datum:	Ground Level				Remarks:							
Borehole: BH2 Project: CON103001 Sloy Power Station Location: Sloy Grid Reference: Elevation: Orilled Depth: 35.0m Date: 04/03/2010 Logged Depth: 34.02m Recorded By: Logging Datum: Ground Level Logged Interval: Fluid Level: Structure Key: Foliation Fracture Vein BOREHOLE RECORD Sit Diameter: From: To: 150mm Om 4.2m Depth Caliper Tilt 3D Log Amplitude			North reference is magnetic, Tadpo	le log and tabulated data is correct	ed for borehole deviation							
Fluid Level:												
Structure Key:	Foliation	n — Fracture –	Vein									
BOREHOLI	E RECORD				CASING RECORD							
Bit Diameter:	From:		То:		Туре	Size	From	То				
150mm 0m 4.2			4.2m		Steel	150mm	0m	4.2m				
120mm	4.2m		35.0m									
Depth	Caliper	Tilt	3D Log	Amplitude	Travel	Time	Structure	Ta	adpole			
1m:10m ₁₂	CM 16 (-	0°	0° 90° 180° 2	70° 0° 0° 90° 180	0° 270° 0° 0° 90	0° 180° 270° 0°-	10	100			
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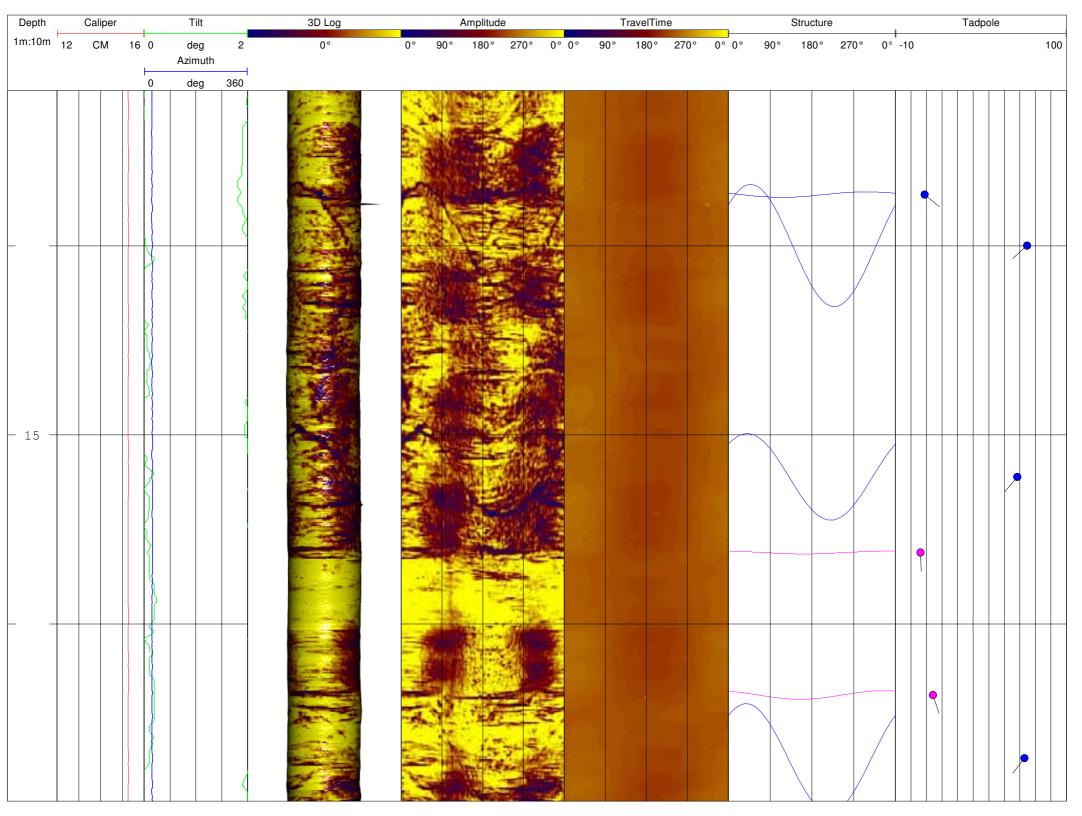


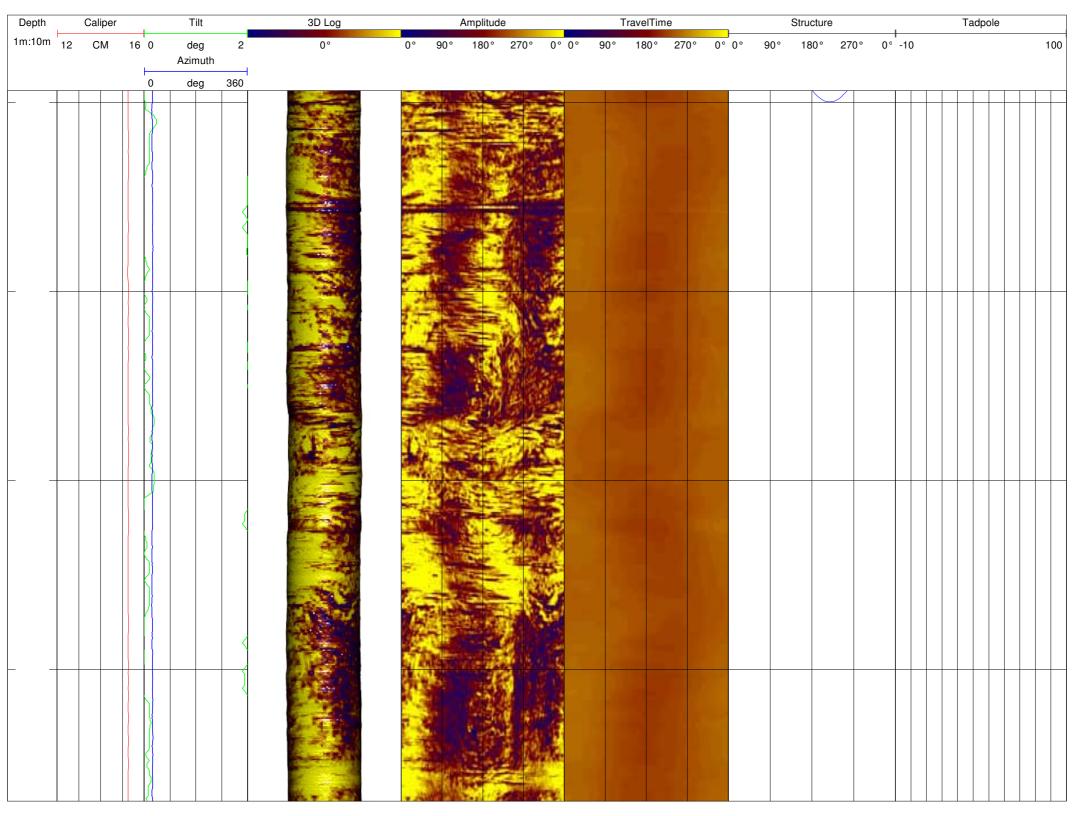


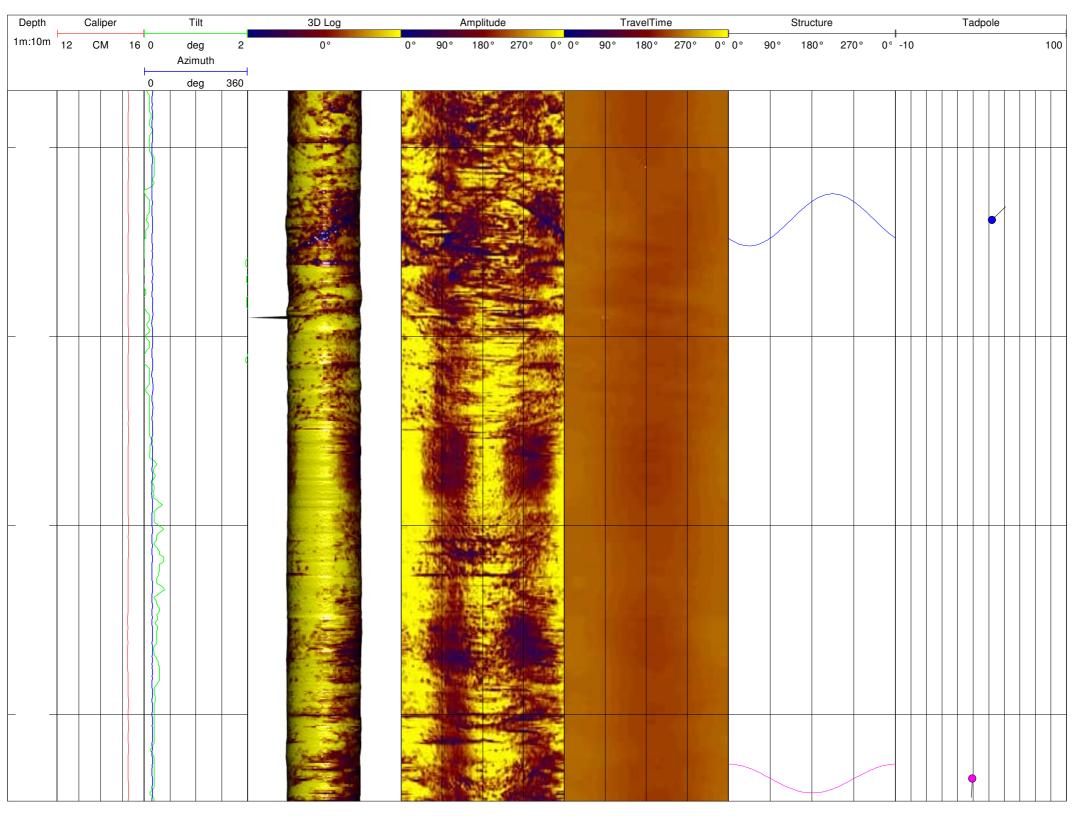


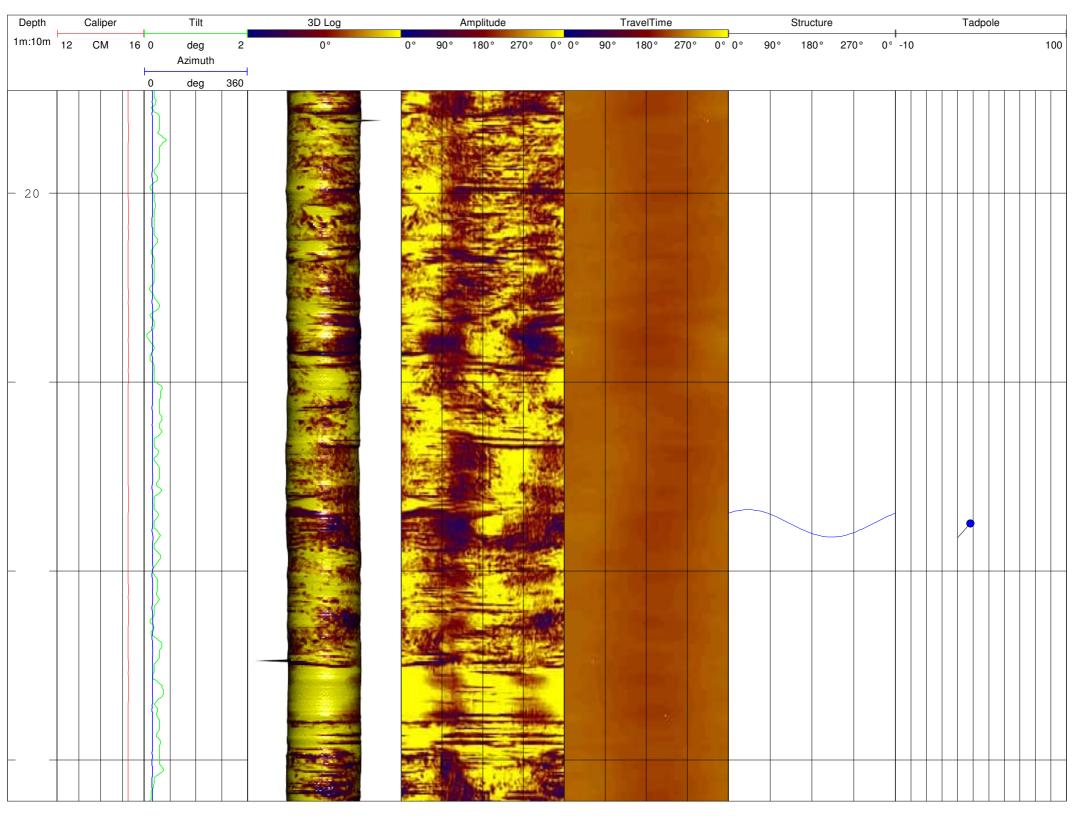


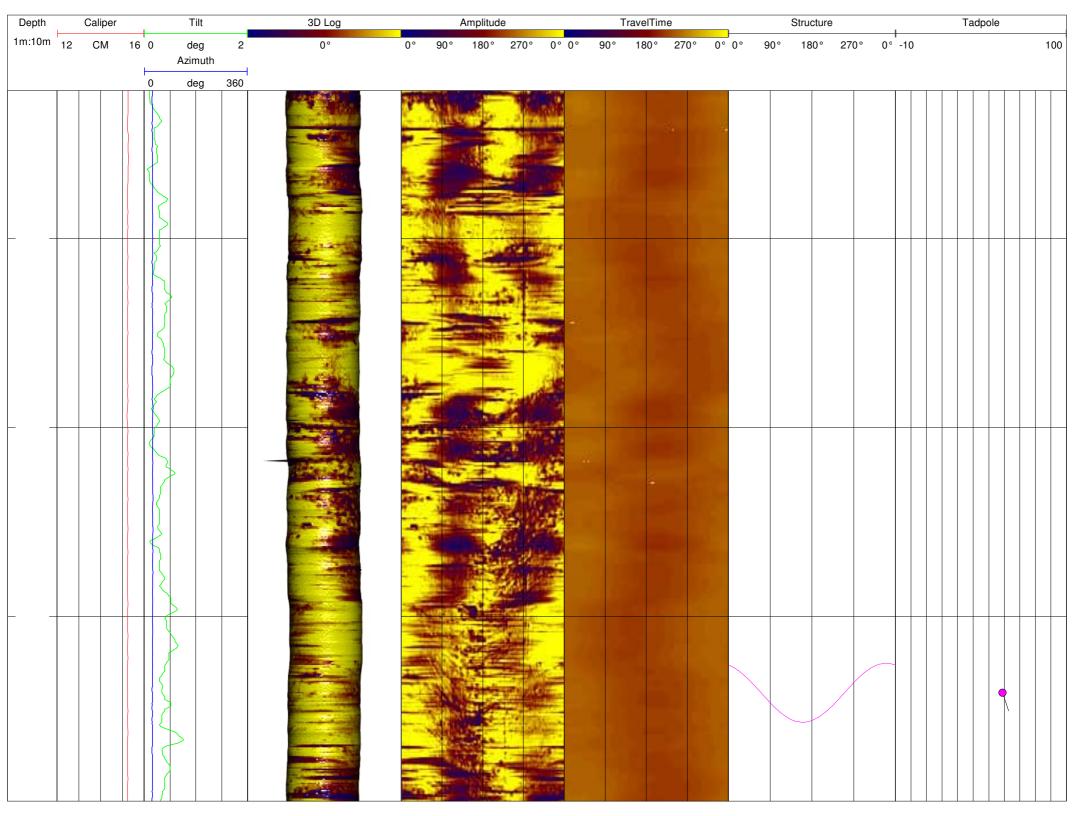


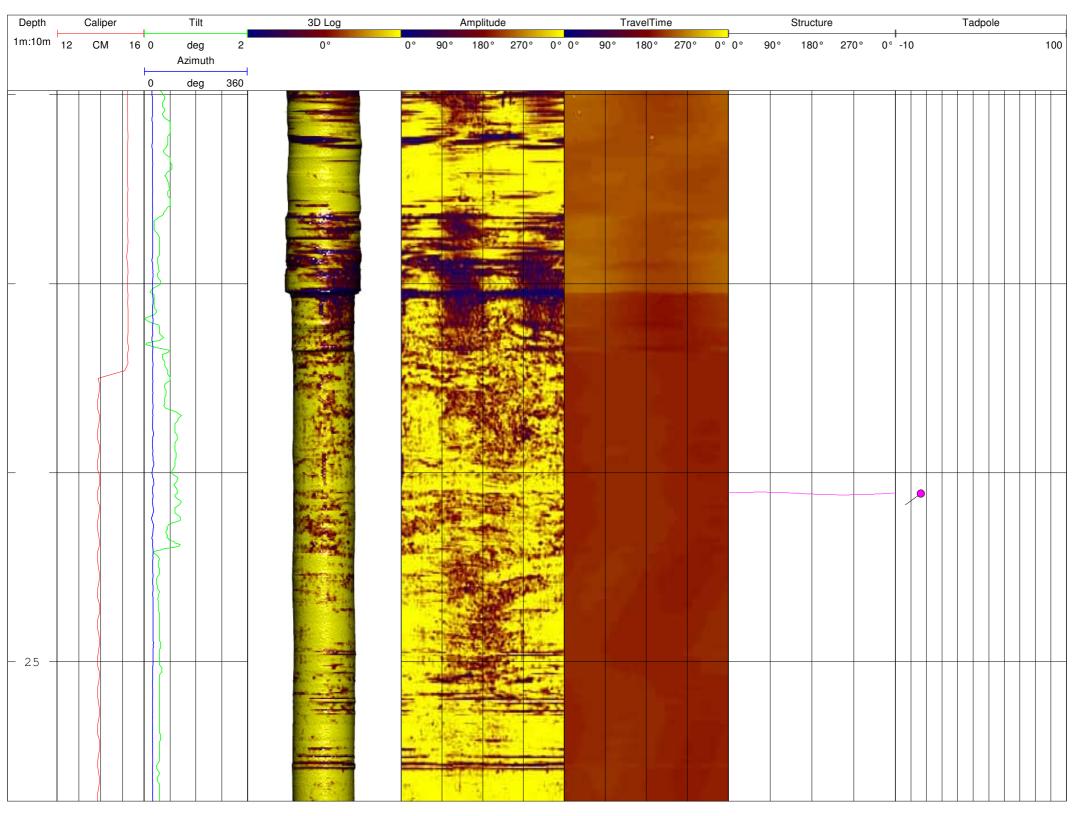


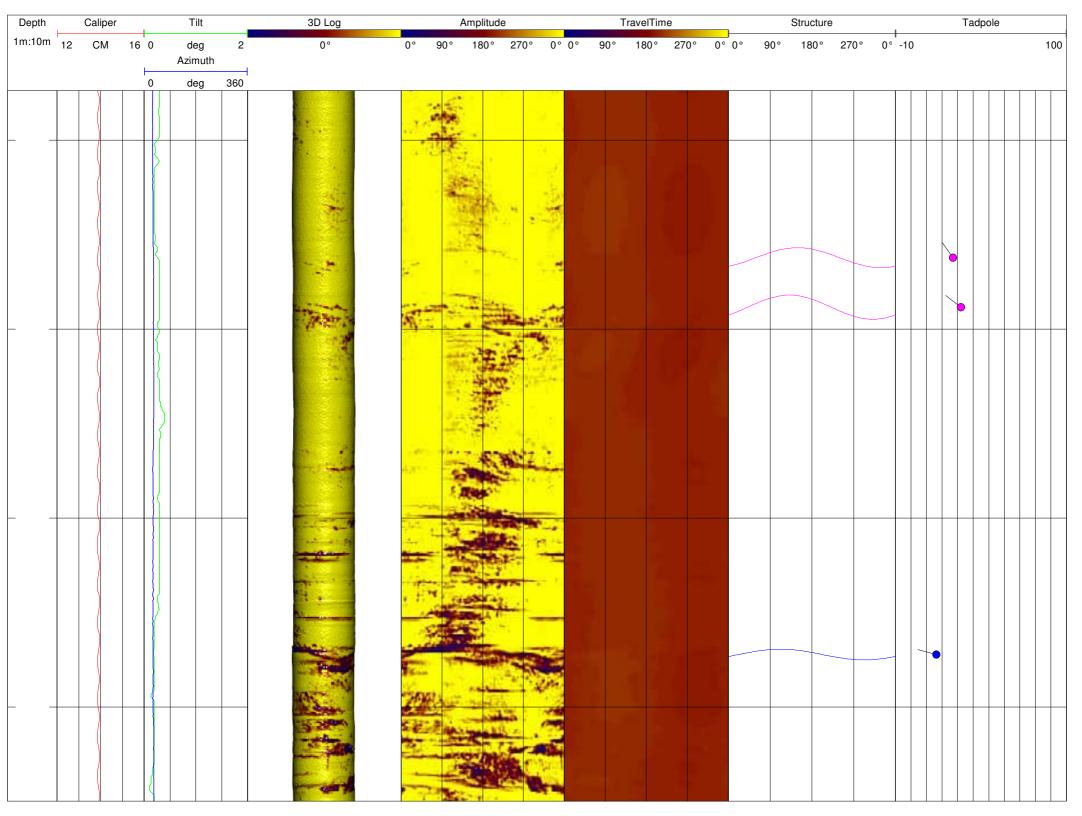


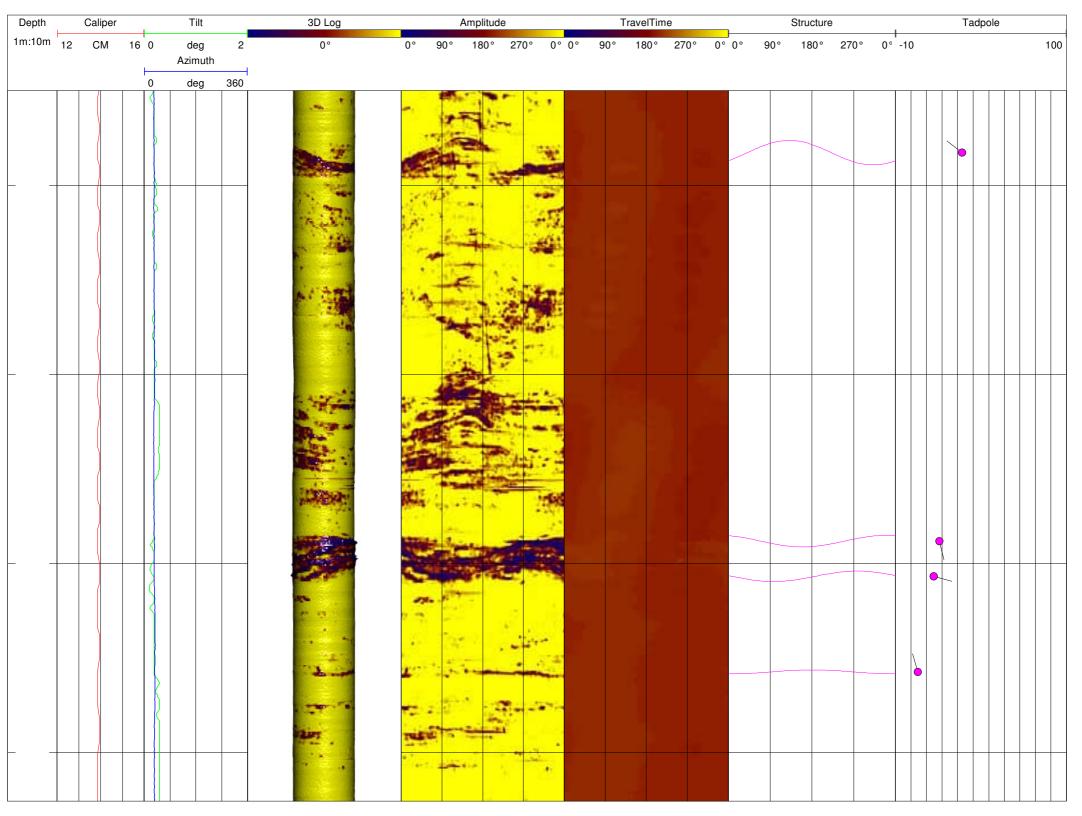


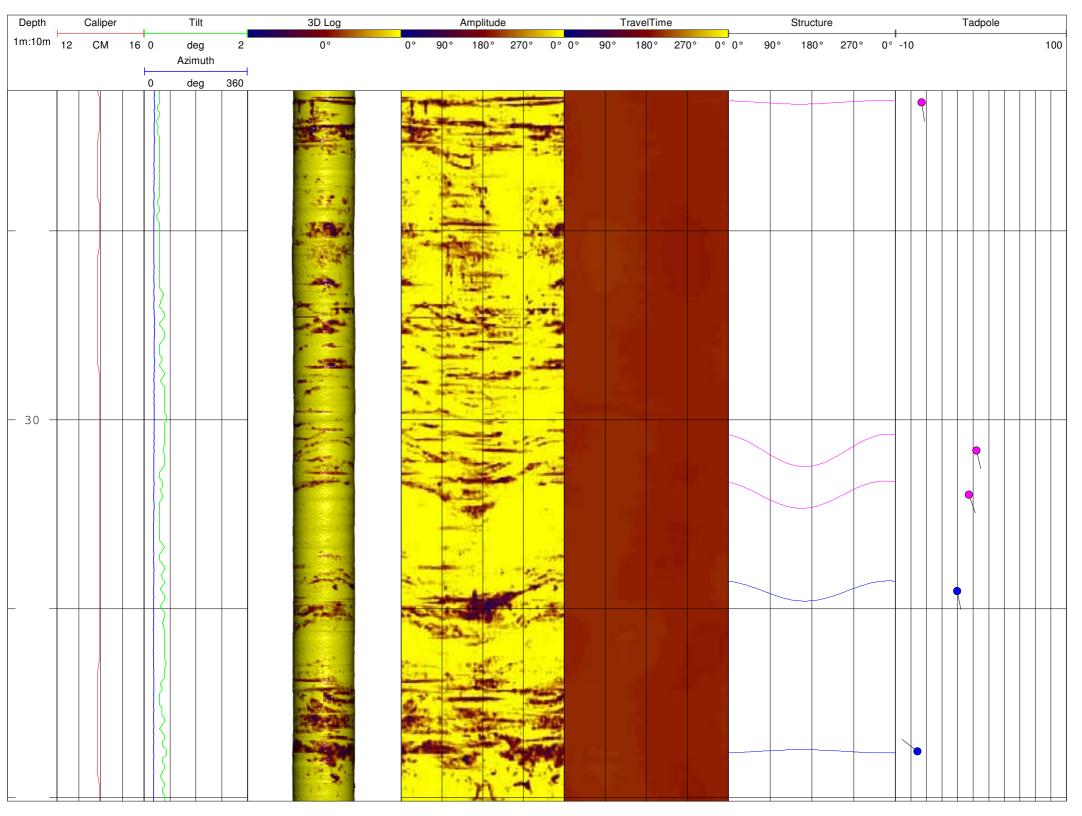


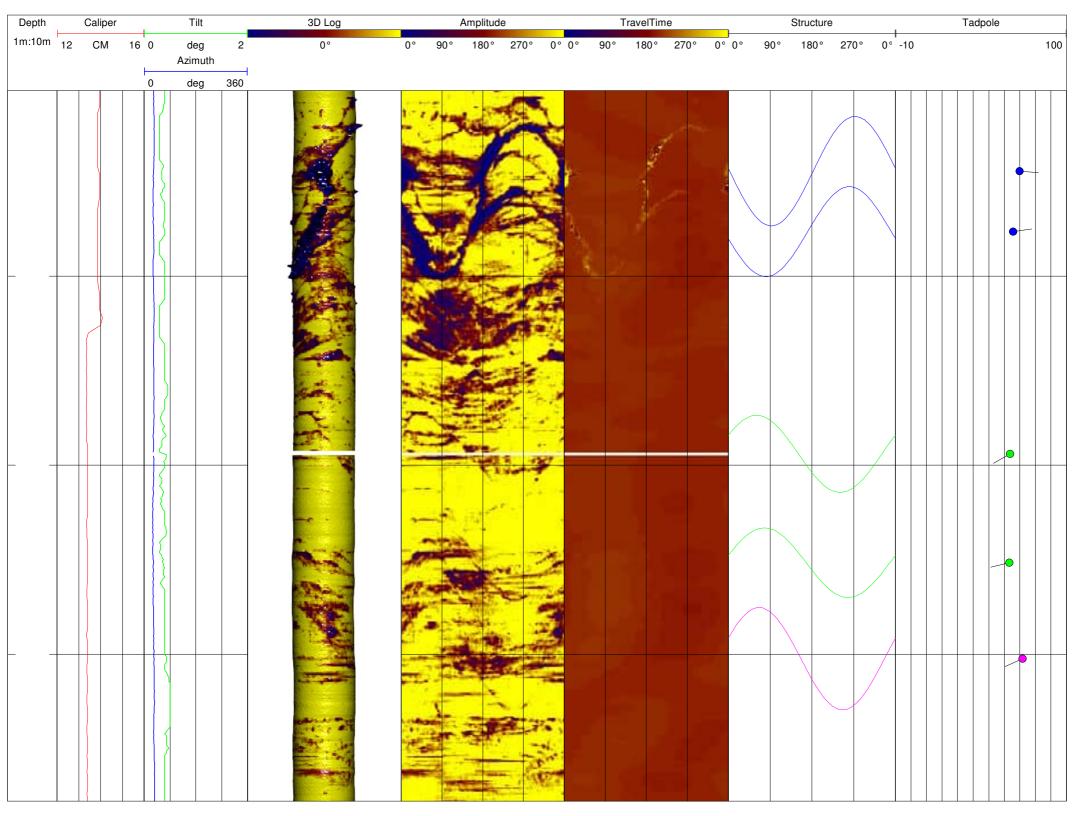


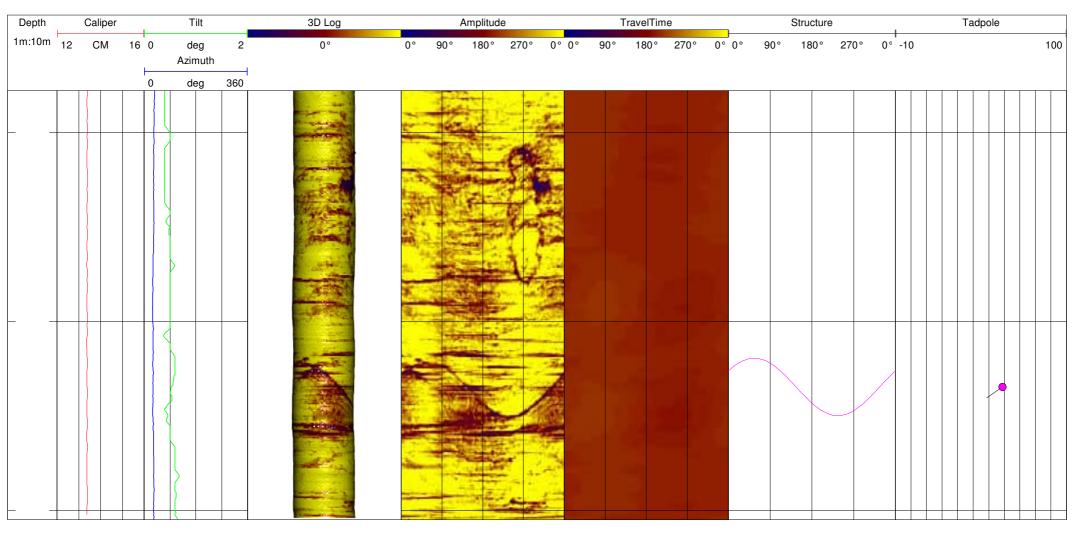




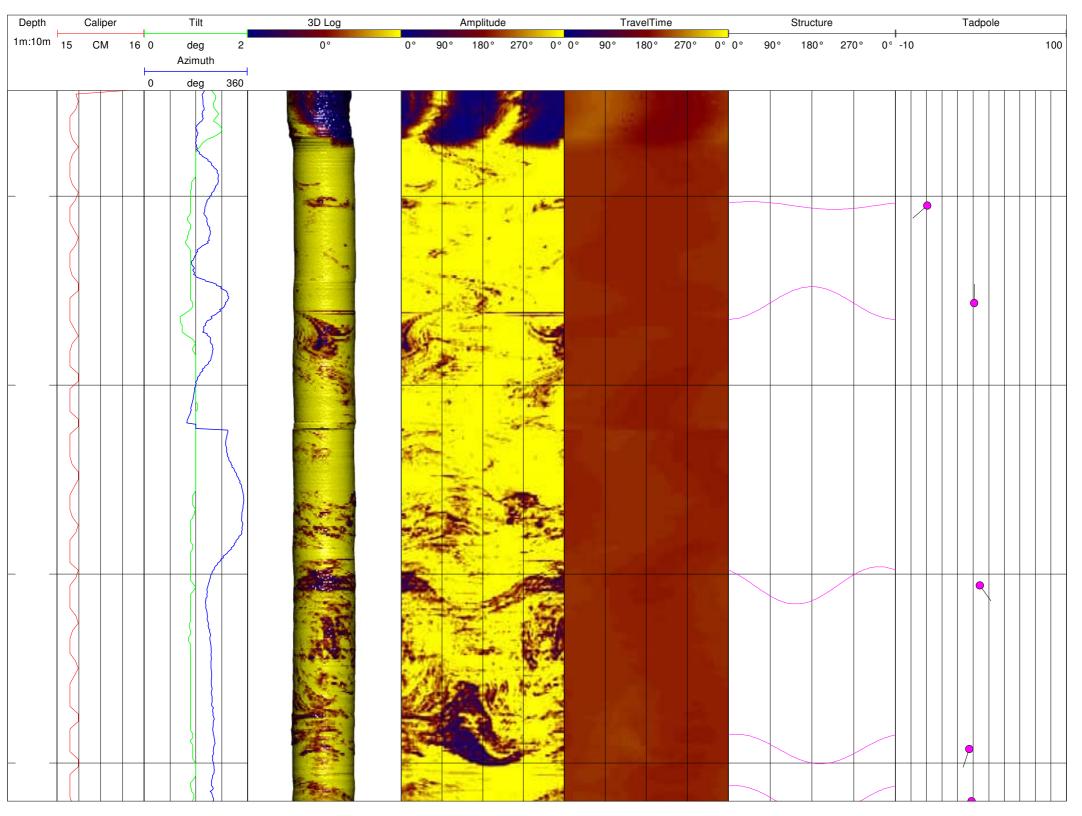


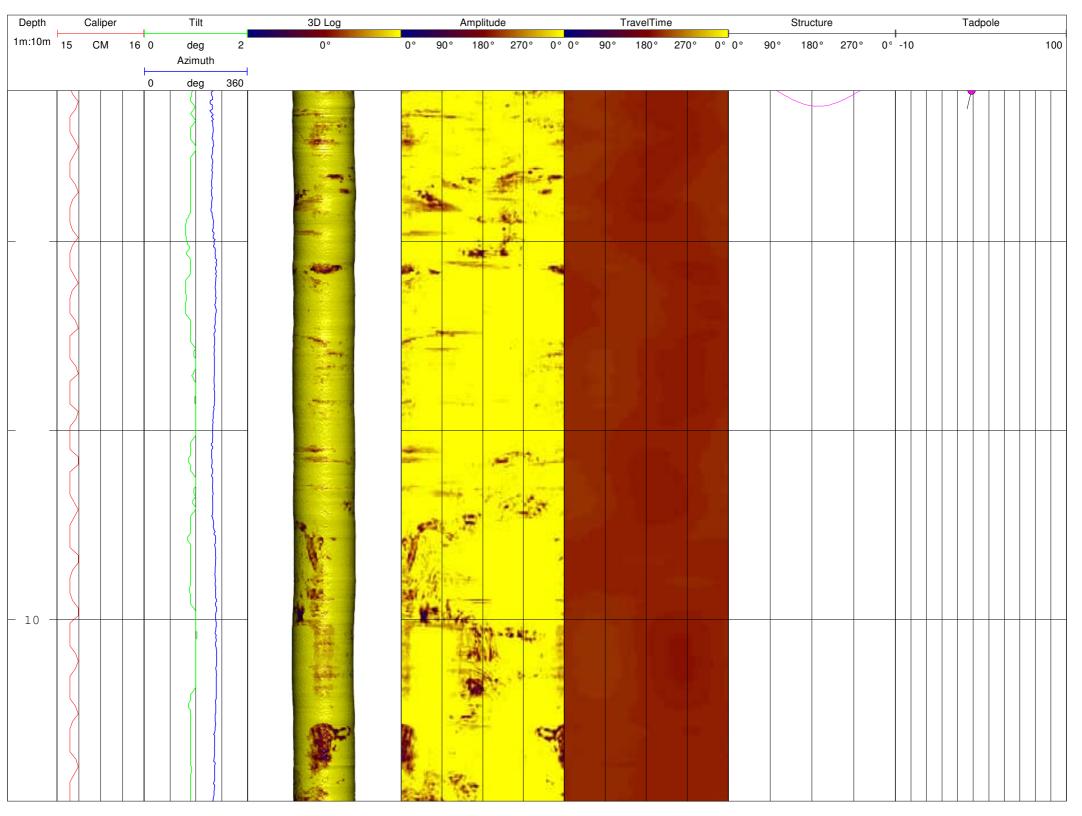


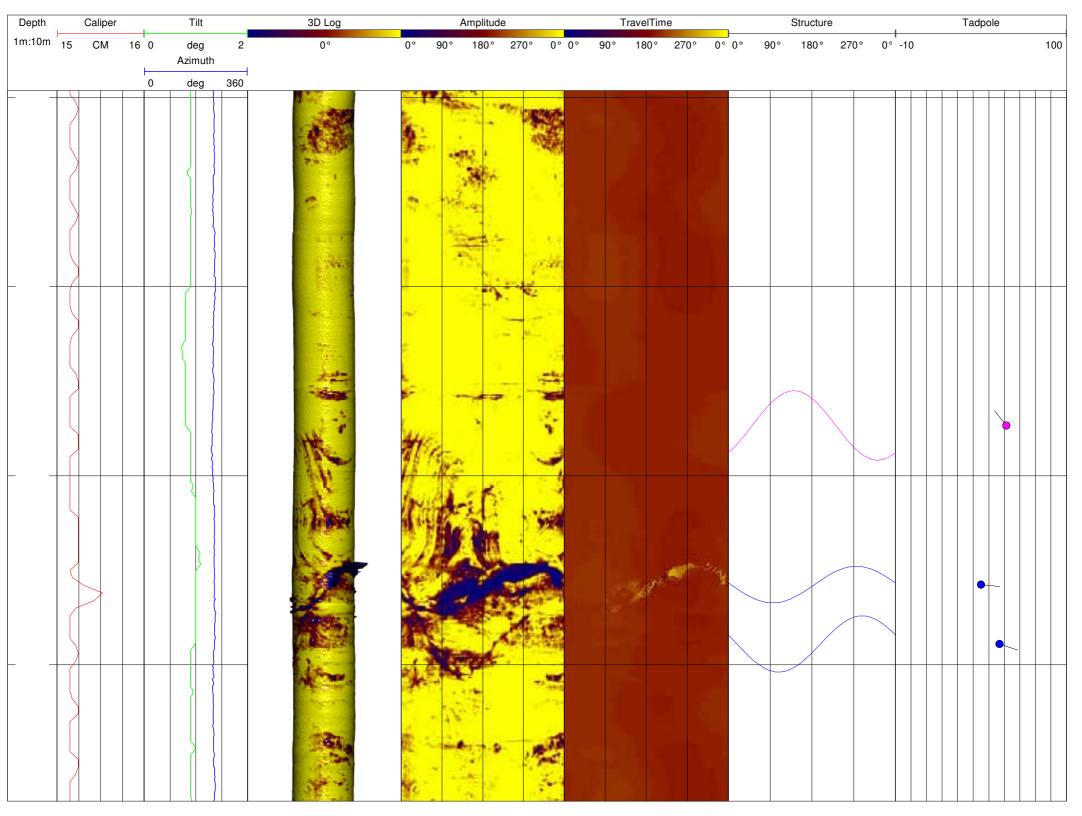


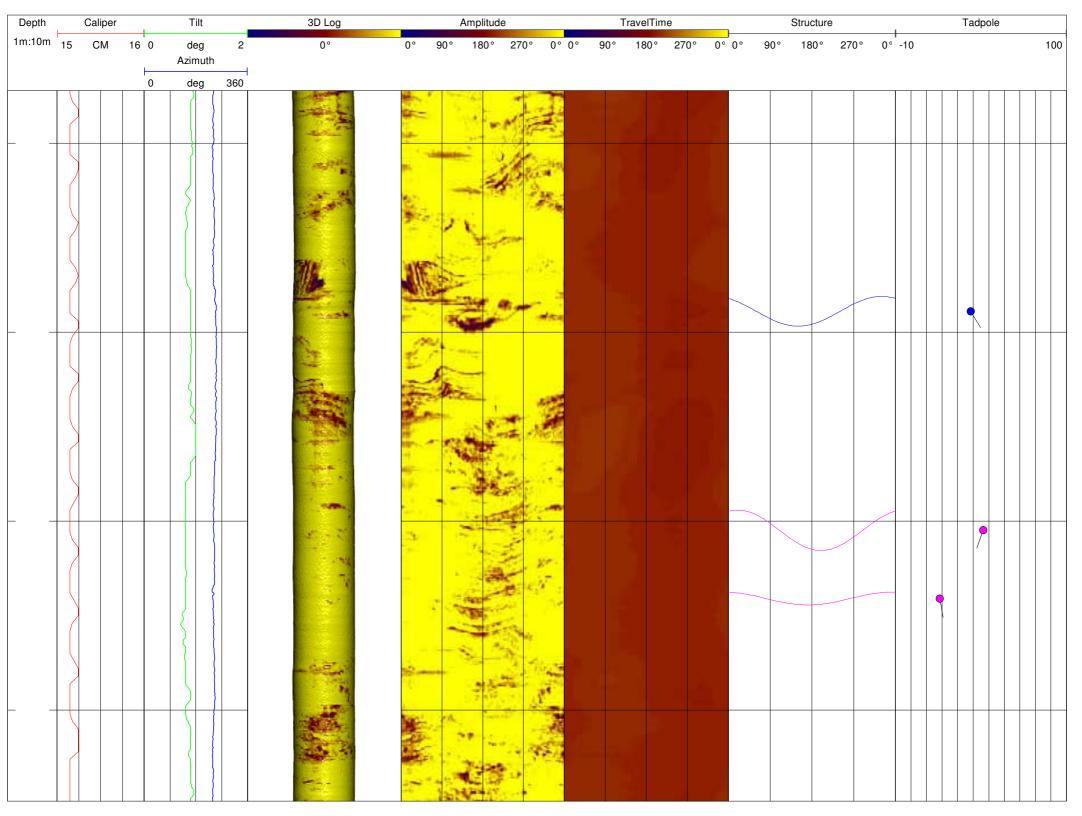


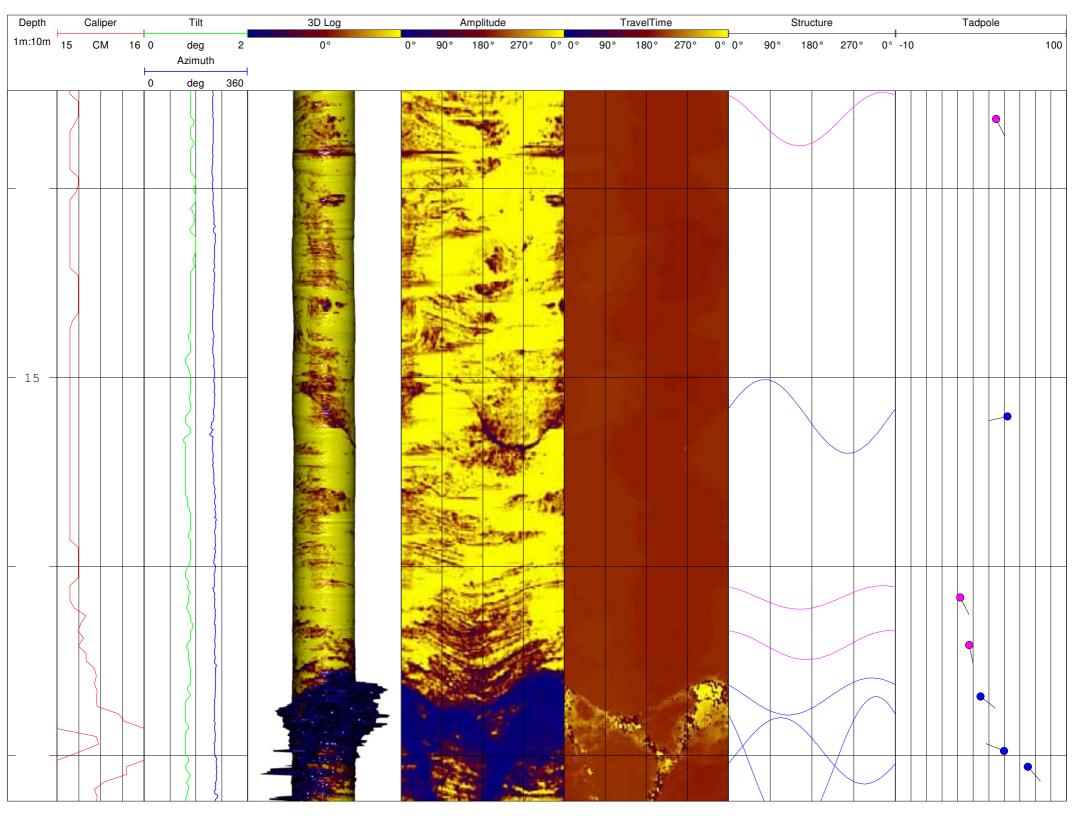
-fuc	RO	Fugro Engi	neering Servi	ces						
	Client: Scottish and Southern Energy PLC		Log Type:							
	Boi	rehole:	ВН3		Acoustic Televiewer Log					
Proiect: CO	N103001 Sloy Po	wer Station			Approved:					
Location: Sloy		Grid Re	ference:	Elevation:						
Drilled Depth: 35m Date: 04/03/2010										
Logged Depth: 33.62m Recorded By:										
Logging Datum:										
Logged Interval:					North reference is magnetic, Tadpole log and tabulated data is corrected for borehole deviation					
Fluid Level:										
Structure Key: -	Foliation -	Fracture —	Vein							
BOREHOLE	E RECORD				CASING RECORD					
Bit Diameter:				То:		Size	From	То		
150mm	0m 6.		6.8m		Steel	150mm	0m	6.8m		
120mm	6.8m 35.0m									
1	Caliper	Tilt	3D Log	Amplitude	Travel	Time	Structure	Tac	dpole	
1m:10m 15	CM 16 0	deg 2 Azimuth	0°	0° 90° 180° 2	70° 0° 0° 90° 180	0° 270° 0° 0° 9	0° 180° 270° 0°-1	10	100	
	0	deg 360								
		deg 300	20							
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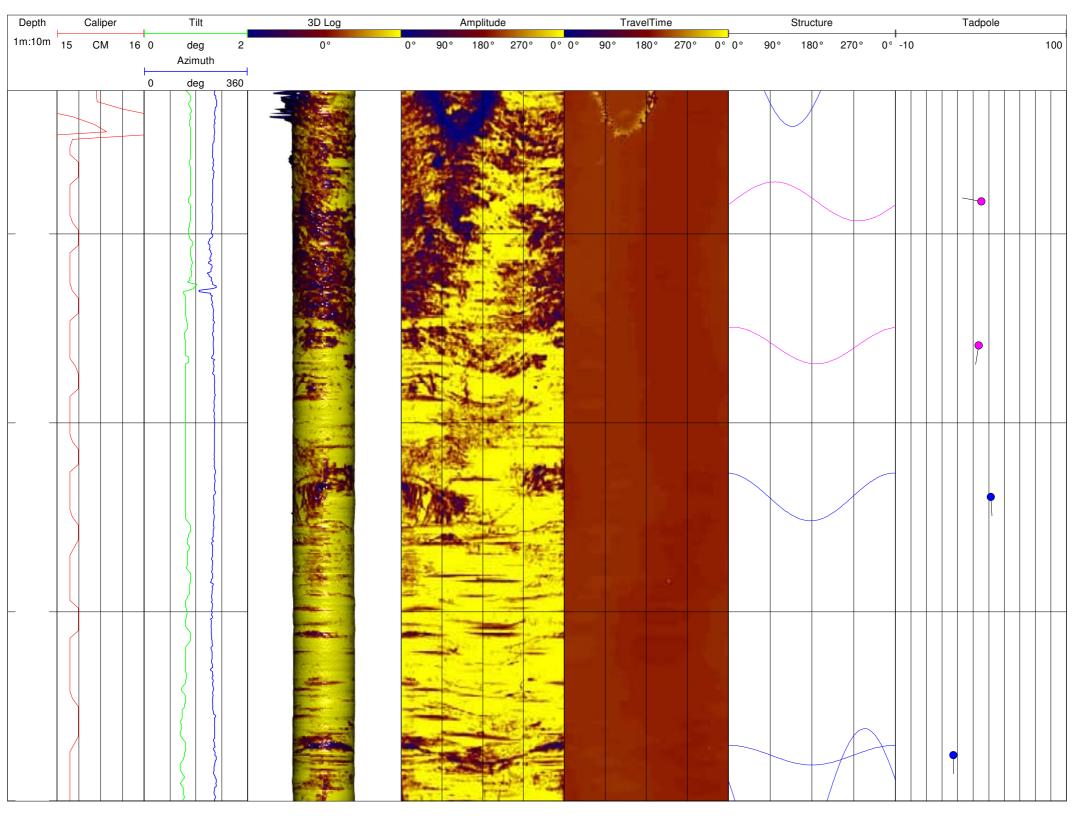


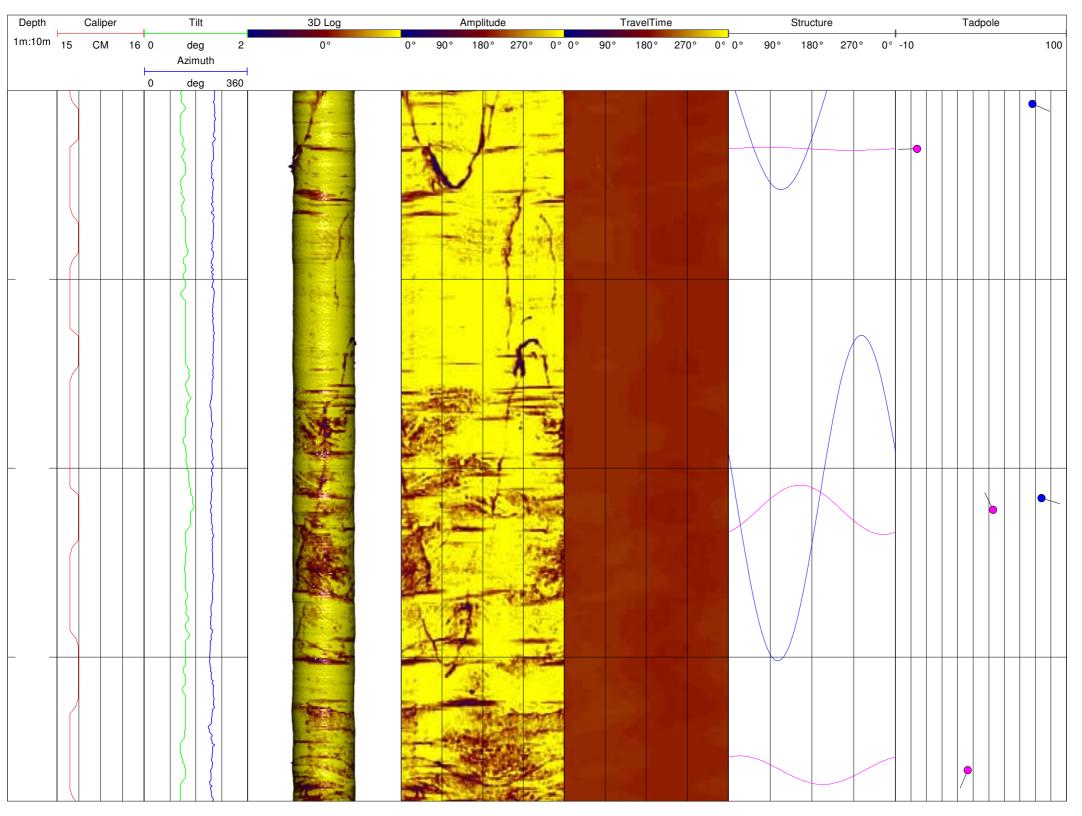


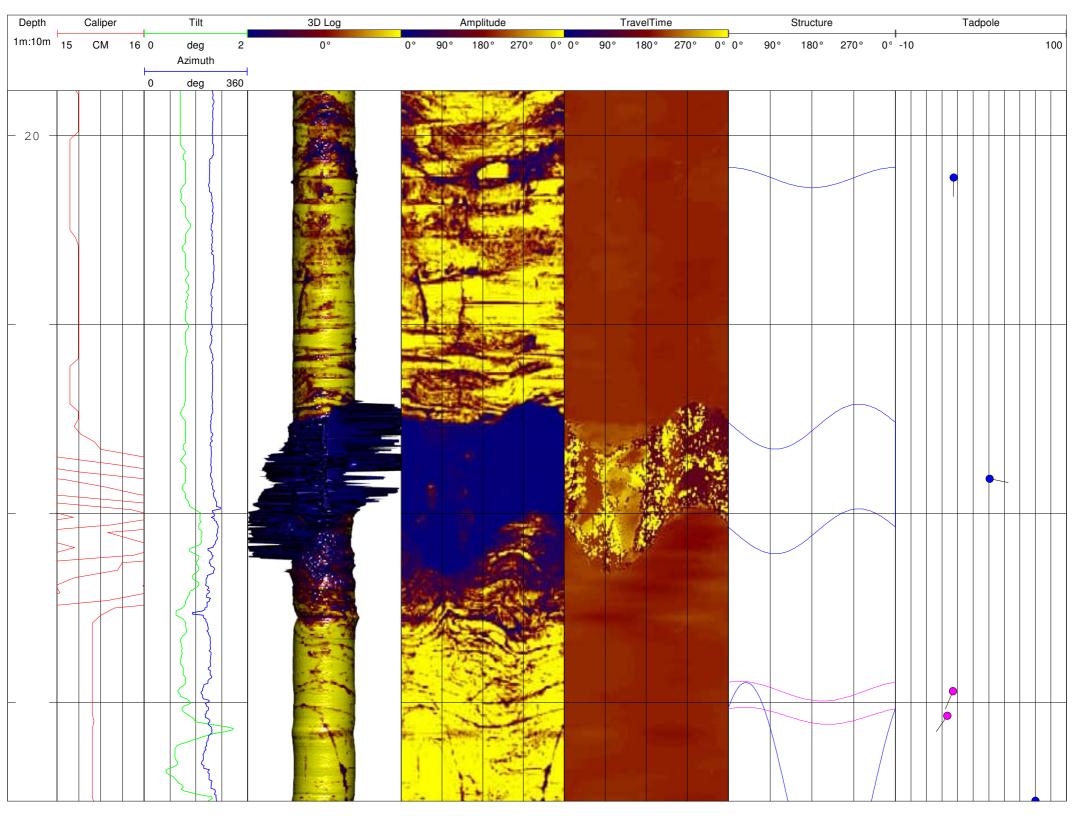


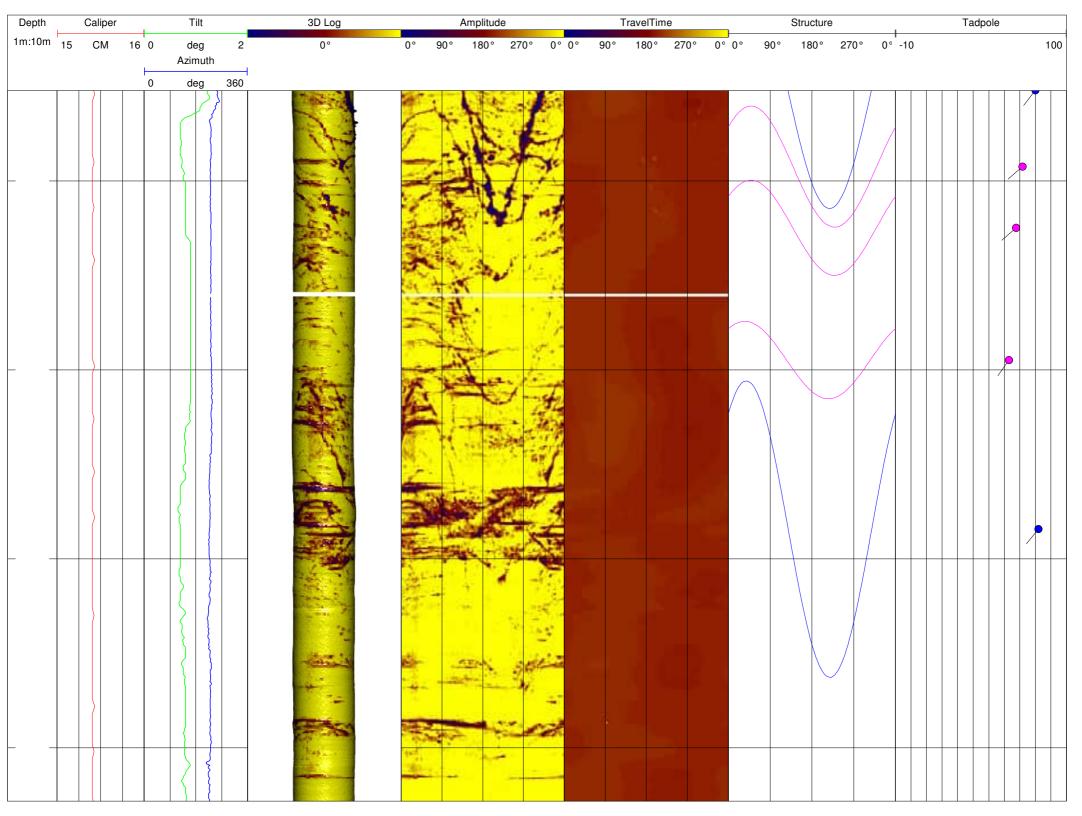


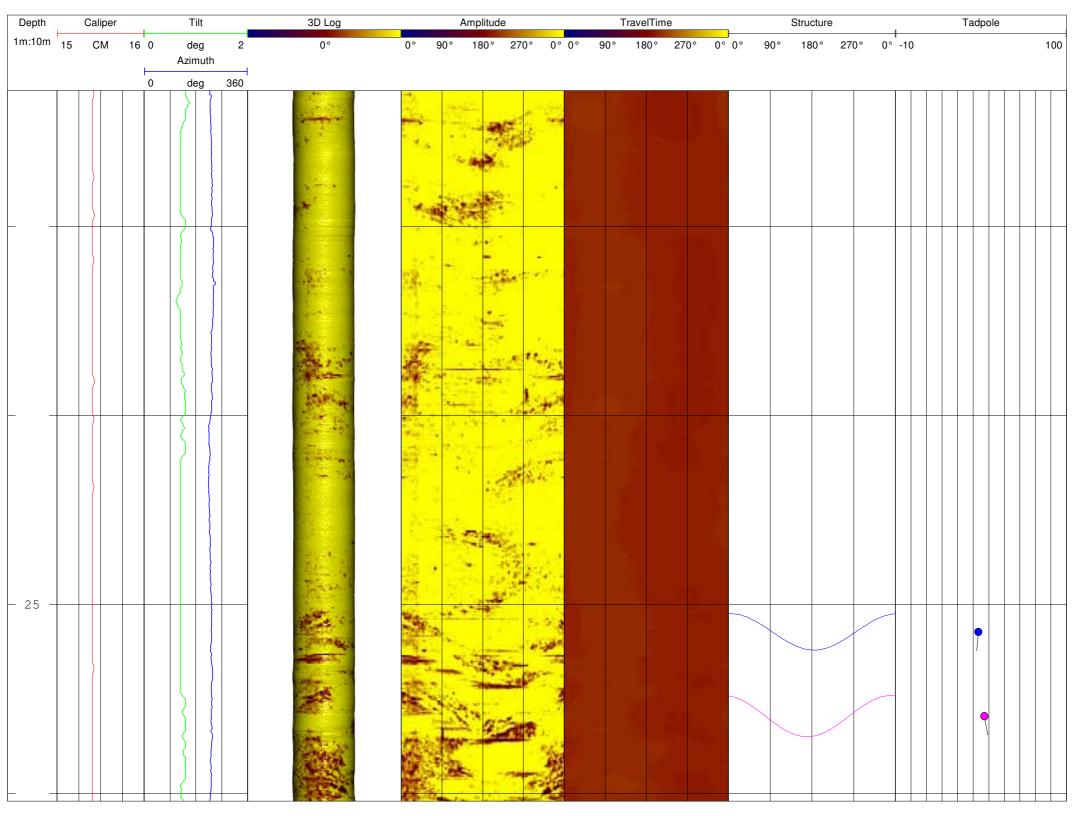


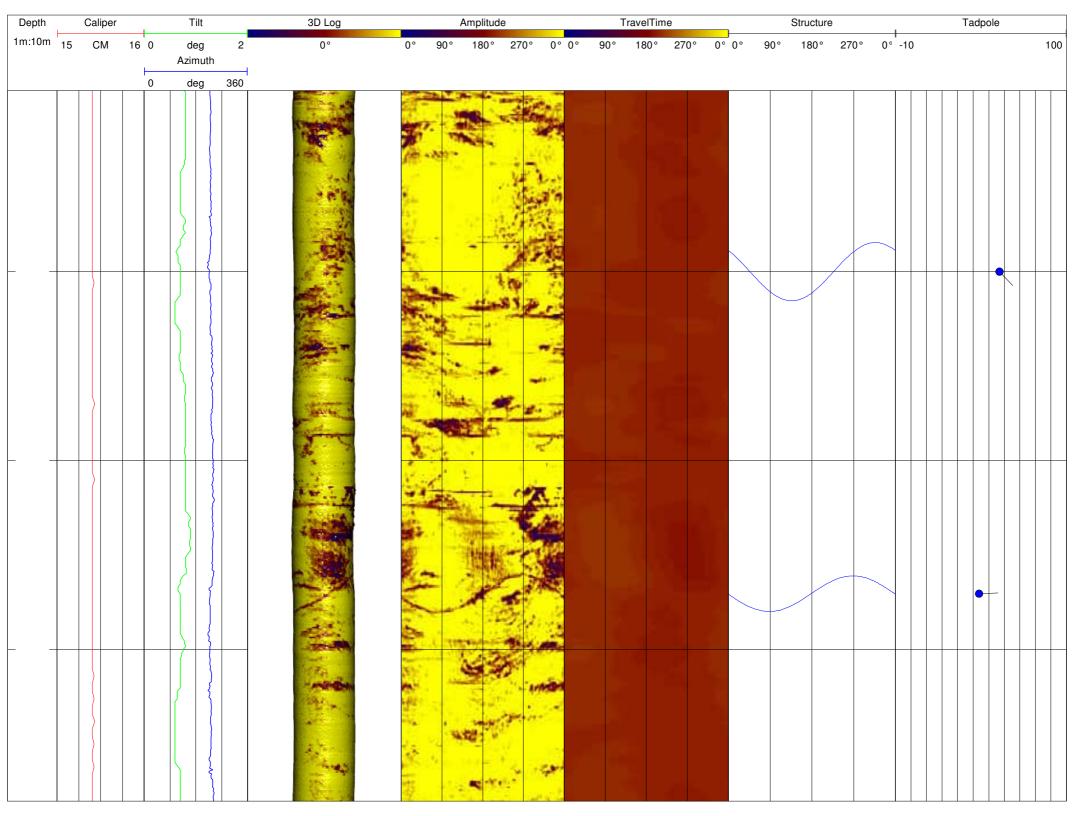


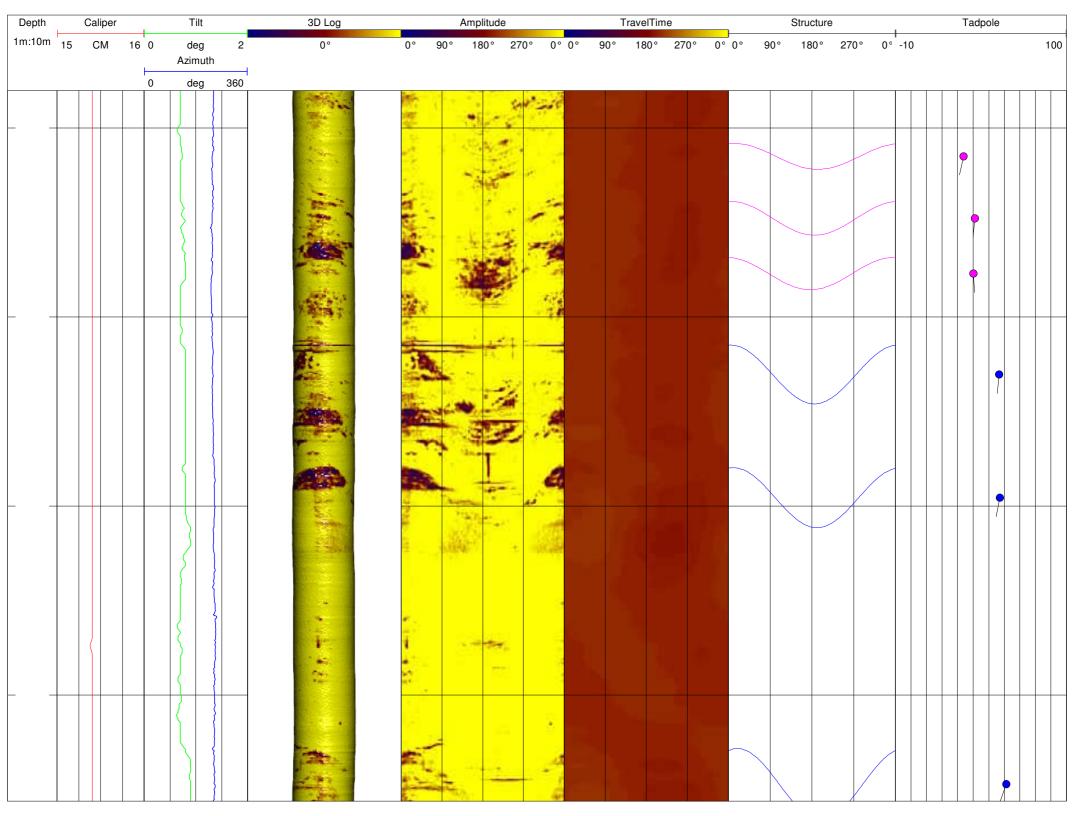


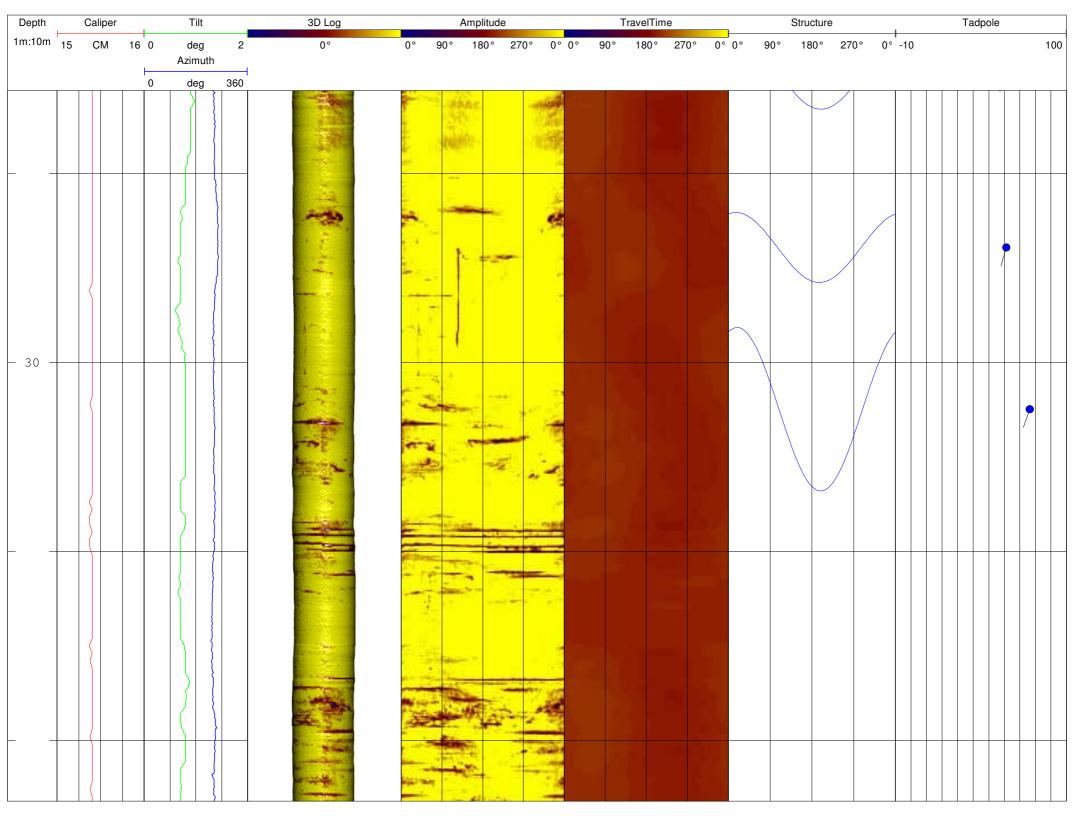


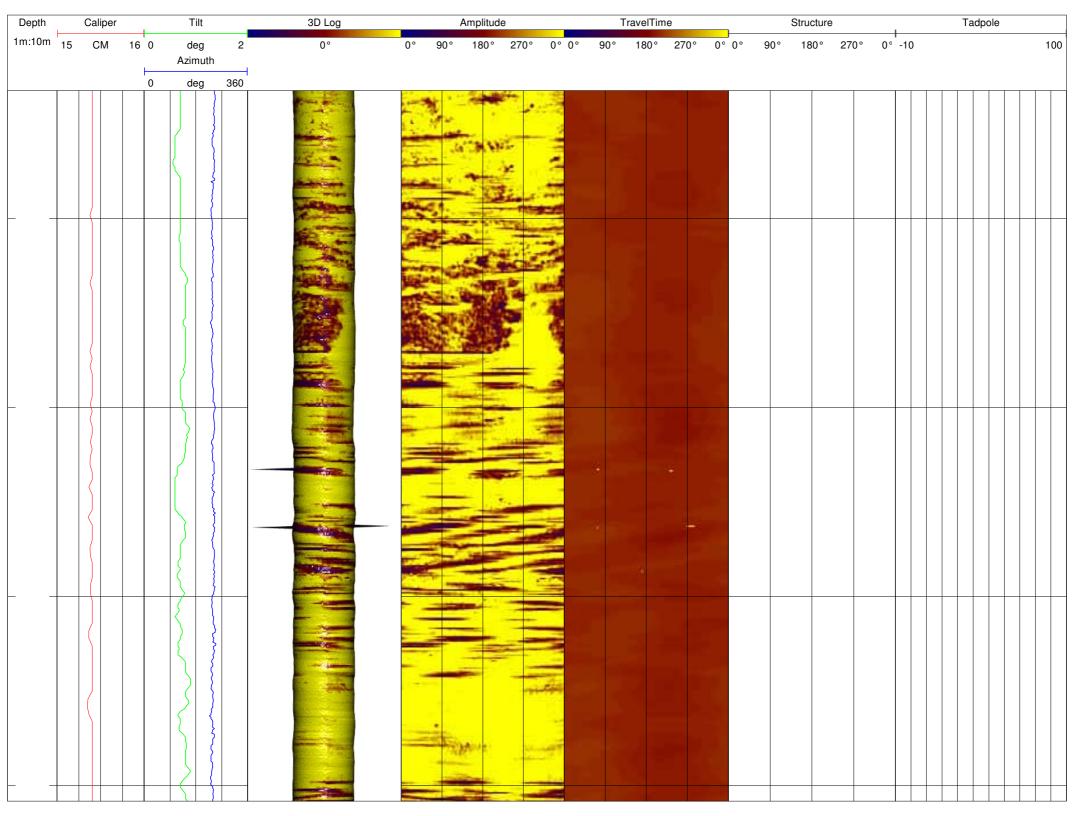


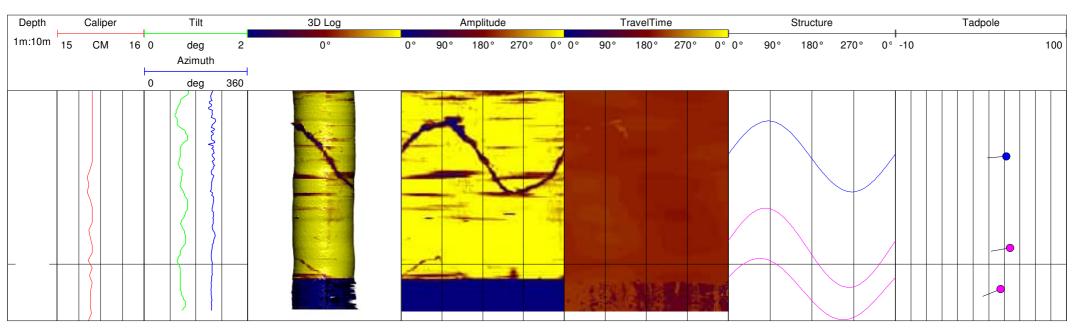












Fugro Engineering Services										
		Client:	Scottish and So	outhern PLC	Log Type:					
	$= \propto =$	Borehole:	BH4		Acoustic Televiewer Log					
Project: CON103001 Sloy Power Station					Approved:					
Location: Sloy Grid Reference: Elevation:										
Drilled Depth: 35m Date: 04/03/2010										
Logged Depth: 33.99m Recorded By:										
Logging Datum:	Ground Level				Remarks:					
Logged Interval:					North reference is magnetic, Tadpole log and tabulated data is corrected for borehole deviation					
Fluid Level:										
Structure Key: -	Foliatio	n ——— Fracture —	Vein							
BOREHOLE	E RECORD				CASING RECORD					
Bit Diameter:	it Diameter: From: To:			То:		Size	From	То		
150mm	0m		4.2m		Steel 150mm		0m	4.2m		
120mm	4.2m 35.0m									
Depth	Caliper	Tilt	3D Log	Amplitude	Travel	Time	Structure	Tadpole		
1m:10m 15	CM 16 0	deg 2 Azimuth	0°	0° 90° 180° 27	70° 0° 0° 90° 180	0° 270° 0° 0° 90)° 180° 270° 0°-1	0 100		
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